

ADDENDUM NO. 4
January 07, 2026

Town of Urbana
Champlin Beach Pier Project
SA PROJECT # 2019-021.10

This addendum, issued to bid document holders of record, indicates changes to the Bid Documents issued December 05, 2025. All changes described herein shall be incorporated into the contractor's bid proposal. This addendum is part of the Contract Documents. Acknowledgement of receipt of this Addendum is required on the respective Bid Form.

Prepared By:

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Attachments: N/A

The following notes, changes, additions, and replacements shall be incorporated into the Project Manual or drawings as applicable:

ITEM 1. Question Responses

A. Below are responses to the following previously submitted questions:

Question #1:

Can you please provide the reaction loads at each pile based on the layout provided? We calculated 22.5Kips per pile based on 300 PSF combined load. Is this correct?

Response #1:

The live load (15' wide pier & 10' extension): shall use aashto h5 vehicle or pedestrian load = 90 psf, whichever is greater.

The live load (end of main dock at octagonal end) shall have a 150 psf live load & 150 psf dead load.

The live load (wide 4' docks): shall have a pedestrian load = 90 psf.

The above loads are superimposed on top of the pier structure; dead loads vary depending on the pier structure which depend on the proposed delegated design.

Question #2:

Will the Town be providing special inspections?

Response #2:

The Town will provide any required in-place density testing, concrete testing, and E&S inspections.

Question #3:

We did not see any structural steel or miscellaneous metals specifications. Will these be provided?

Response #3:

Structural steel notes/specifications are listed on the bid documents.

Question #4:

What are the requirements for testing of field welds and weld certification?

Response #4:

Any person performing structural field welds will need to be approved by the Owner. It is the contractor's responsibility to provide third party weld testing at their cost. All results and certifications shall be submitted to Town for Final Approval.

Question #5:

Can you specify the frequency of load testing required on the piles?

Response #5:

Special inspection shall be per nys building code 1705.7 requiring continuous special inspections to verify installation details like material compliance, driving logs (for example: hammer type, blows per foot), penetration depths, and elevations for driven piles, and load tests, ensuring all work aligns with geotechnical reports and construction documents. continuous inspection and detailed record-keeping are required for deep foundations. load testing requirements may vary depending on the variation/consistency of the installed and inspected piles.

Question #6:

The plans indicate driven piles. Will helical piles be accepted?

Response #6:

Engineered helical piles are acceptable.

Question #7:

The agreement states one-year warranty and the drawings state 50 year warranty, please advise?

Response #7:

The agreement of one-year warranty is for installation and workmanship of the pier, while the 50 year warranty is to include all installed materials.

Question #8:

Will there be a construction staging area on-site? If so, where?

Response #8:

The staging area is to be located just southwest of the proposed pier landing as shown on L200.

Question #9:

Can you provide a cohesion value for the clay soils?

Response #9:

The boring log was provided from borings completed by Nothnagle Drilling near the shoreline adjacent to the proposed pier in 2020. These show a silty clay found and typical blow counts documented. All information that we have was provided in prior addendums.

Question #10:

Can you state if the Ray Teeter, PE Report is valid for this project? It appears that these borings were in significantly different locations of the property and may not be useful for this design?

Response #10:

The boring log was provided from borings completed by nothnagle drilling completed near the shoreline adjacent to the proposed pier in 2020. These show a silty clay found and typical blow counts documented. If additional soils testing is required for your delegated design, it should be included in your bid and outlined as such.

Question #11:

Can you provide details of the load testing that was completed on Work Reports dated 4/2/2020? What was the load applied to the piles for testing? Can you provide deflection of the piles during the tests?

Response #11:

All information that we have was provided in prior addendums.

Question #12:

Will a geotechnical report be provided?

Response #12:

All information that we have was provided in prior addendums.

Question #13:

Please provide the design parameters and performance specifications for the delegated design components. It is unclear if the delegated design piece is the foundation or the structural steel.

Response #13:

The delegated design is the complete structural design of the pier, decking material, and pile supports for the structure. For the removal of the existing pier, it is the means and methods of the removal that would be the delegated design.

Question #14:

Use of the timbertech decking. 50 year warranty and much more cost effective for the town to replace if damaged. This decking requires no maintenance in comparison to the IPE needing treatment every one to two years.

Response #14:

Sure, Timbertech decking can be presented as a decking alternative. It is a decent product but will have to be approved by the Owners as they are the final decision if they want this.

Question #15:

The IPE 2 by 8 is over 400 a single board. By doing this would increase the cost of the total job to way more than 2 million with all the other aspects of the project. By doing so this will increase the count of the galvanized 6 inch joists. The joists will need to be 16 on center and not 20.

Response #15:

I believe the supports under the Timbertech needs to be smaller on center spacing (ie. 12" OC). Again, with the delegated design and substituting decking the structure will increase with more supports. The cost implications have to be considered on the final decision.

Question #16:

We are also proposing to weld caps into the 10 inch pipes instead of the bolt on bracket as shown on the engineer drawings. We would weld the W10 to the caps instead of the bolt on bracket. This will increase the shear strength on the steel beams to the pilings. Please let me know your thoughts.

Response #16:

Again, design calculations would have to be provided with the connection details proposed.