ADDENDUM NO. 1
December 10, 2025

Town of Urbana
Champlin Beach Pier Project
SA PROJECT # 2019-021.10

This addendum, issued to bid document holders of record, indicates changes to the Bid Documents issued December 05, 2025. All changes described herein shall be incorporated into the contractor's bid proposal. This addendum is part of the Contract Documents. Acknowledgement of receipt of this Addendum is required on the respective Bid Form.

Prepared By:

Saratoga Associates
Landscape Architects, Architects, Engineers, and Planners, P.C.
21 Congress Street
Saratoga Springs, New York 12866

Attachments: Geotechnical Reports, Bathymetric Survey of Area, Daily Work Report at existing pier.

The following notes, changes, additions, and replacements shall be incorporated into the Project Manual or drawings as applicable:

ITEM 1. Pre-Bid Discussion Points

A. Attention called to existing site conditions: Attached is a geotechnical boring completed at the pier location and a geotechnical report completed on an adjacent project within 1,000 Feet of proposed project for reference. Attached are the findings from that report.

B. Attached is a bathymetric survey of the pier location.

C. Attached is the Daily Work Report from assessment work conducted near the pier.

ITEM 2. Pre-Bid Conference and site walk-through

A. Pre-Bid Conference and site walk-through is not mandatory for bidding. Bidders are recommended to visit the site at their own during park hours.

ITEM 3. Pre-Bid Questions

A. Questions Due – via email to Daniel Shearer by 2pm on Friday, January 16th

NOTHNAGLE *DRILLING*, INC.
1821 Scottsville-Mumford Road
Scottsville, New York 14546
Phone (585) 538-2328
Fax (585) 538-2357

Test Boring No. B-1
Page 1 of 3
ND Job # 206647

Project Proposed Pier, Champlin Beach, Town of Urbana, Steuben County, New York
Client Town of Urbana, P.O. Box 186, Hammondsport, New York 14840
Elevation _____ Start 2/12/20 Completed 2/12/20 Driller T. Mangefrida
Water Level - During Drilling 31'6" Inspector _____
Water Level - At Completion 3'3"
Seasonal and climatic changes may alter observed water levels.

	C	Blows on Sampler 0"/6"	6"/12"	12"/18"	18"/24"	Sample N	Rec.	No.	Depth	Visual Soil and Rock Information Remarks	
0		1	1							Topsoil and organics	0'2"
				1	4	2	8"	1	0'0"-2'0"	Loose gray moist fine sand, some silt, little gravel (fill)	
5											5'0"
		1	1								
				2	2	3	13"	2	5'0"-7'0"	Loose dark gray moist silt, some clay, trace gravel and organics	
10											10'0"
		1	3							Medium gray moist silty clay, trace sand and organics	
				3	3	6	15"	3	10'0"-12'0"		
15											
		WH	WH							Very soft gray moist	
				WH	2	WH	19"	4	15'0"-17'0"		
20										Very soft gray moist	20'6"
		WH	WH							Loose brown moist silt, some organics	
				WH	2	WH	20"	5	20'0"-22'0"		
25											25'0"
		WH	WH							Very soft gray moist silty clay, trace sand and organics	
				WH	WH	WH	20"	6	25'0"-27'0"		
30											
		WR	WR							Very soft gray moist	31'6"
				WR	WH	WR	20"	7	30'0"-32'0"	Loose dark gray saturated fine sand	
35											35'0"
		WR	WH							Loose dark gray saturated silt and sand, little clay	
				WH	WH	WH	20"	8	35'0"-37'0"		
40											40'0"

N=No. of Blows to Drive 2" Spoon 12" with 140 lb. Wt. 30" Ea. Blow
C=No. of Blows to Drive__ Casing___with___ lb. Wt.___ Ea. Blow
Transitional Depths are Estimated Based on Field Observations

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Water Level - At Completion 3'3"
Seasonal and climatic changes may alter observed water levels.

	C	Blows on Sampler 0"/6"	6"/12"	12"/18"	18"/24"	Sample N	Rec.	No.	Depth	Visual Soil and Rock Information Remarks
40		WH	WH							Very soft gray saturated sandy clay
				WH	WH	WH	20"	9	40'0"-42'0"	
45										
		WR	WR							Very soft gray saturated
				WR	WH	WR	20"	10	45'0"-47'0"	
50										50'0"
		WR	WR							Loose gray saturated fine sand, some silt, little clay
				WR	WH	WR	16"	11	50'0"-52'0"	
55										
		WR	WR							Loose gray wet
				WH	WH	WH	20"	12	55'0"-57'0"	
60										
		WR	WR							Loose gray wet
				WR	WH	WR	20"	13	60'0"-62'0"	
65										65'0"
		WR	WR							Very soft gray wet silty clay with thin sand lenses
				WR	WH	WR	20"	14	65'0"-67'0"	
70										
		WR	WR							Very soft gray wet
				WR	WH	WR	20"	15	70'0"-72'0"	
75										
		WR	WR							Very soft gray wet
				WR	WH	WR	20"	16	75'0"-77'0"	
80										

N=No. of Blows to Drive 2" Spoon 12" with 140 lb. Wt. 30" Ea. Blow
C=No. of Blows to Drive__ Casing___with___ lb. Wt.___ Ea. Blow
Transitional Depths are Estimated Based on Field Observations

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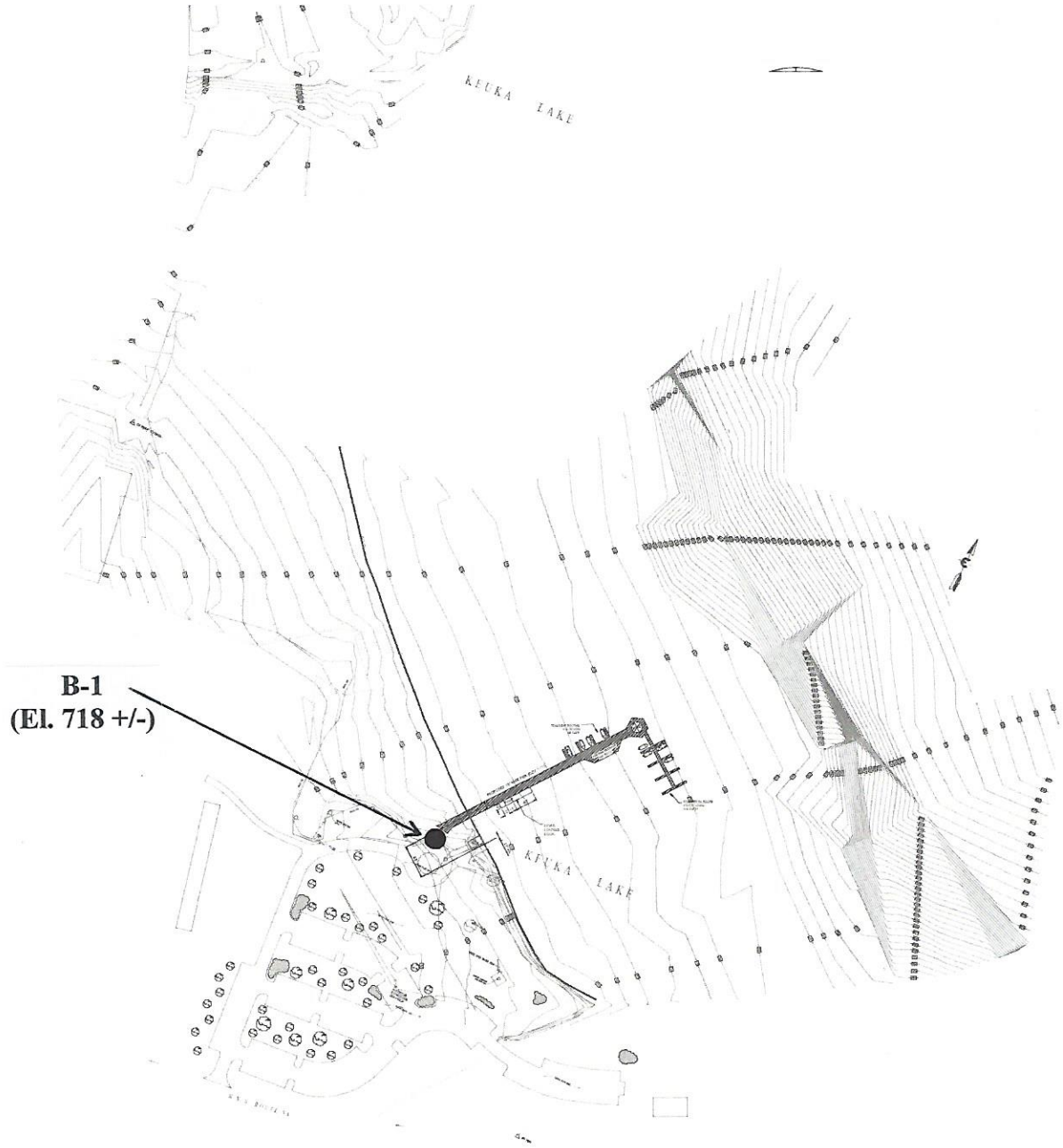
Elevation _____ Start 2/12/20 Completed 2/12/20 Driller T. Mangefrida
Water Level - During Drilling 31'6" Inspector _____
Water Level - At Completion 3'3"
Seasonal and climatic changes may alter observed water levels.

	C	Blows on Sampler 0"/6"	6"/12"	12"/18"	18"/24"	Sample N	Rec.	No.	Depth	Visual Soil and Rock Information Remarks
80		WR	WR							Very soft gray wet
				WR	WR	WR	20"	17	80'0"-82'0"	
85		WR	WR							Very soft gray wet
				WR	WR	WR	18"	18	85'0"-87'0"	
90		WR	WR							Very soft gray wet
				WR	WR	WR	20"	19	90'0"-92'0"	92'0"
95										
100										Boring terminated at 92'0" Advanced test boring with hollow stem auger casing. Boring backfilled on completion.
105										
110										
115										
120										

N=No. of Blows to Drive 2" Spoon 12" with 140 lb. Wt. 30" Ea. Blow
C=No. of Blows to Drive__ Casing___with___ lb. Wt.___ Ea. Blow
Transitional Depths are Estimated Based on Field Observations



Approximate Location and Ground Surface Elevation
Test Boring B-1
Proposed Pier
Champlin Beach
Town of Urbana, Steuben County, New York
2/12/20

Natural Water Contents
Proposed Pier
Champlin Beach
Town of Urbana, Steuben County, New York

Test Boring Number	Sample Number	Sample Depth (feet)	Natural Water Content (percent)
B-1	1	0-2	9
B-1	2	5-7	32
B-1	3	10-12	30
B-1	4	15-17	51
B-1	5	20-22	198
B-1	6	25-27	43
B-1	7	30-32	35
B-1	8	35-37	41
B-1	9	40-42	36
B-1	10	45-47	34
B-1	11	50-52	31
B-1	12	55-57	31
B-1	13	60-62	32
B-1	14	65-67	31
B-1	15	70-72	30
B-1	16	75-77	30
B-1	17	80-82	29
B-1	18	85-87	28
B-1	19	90-92	28

RAY M. TEETER, P.E.
CONSULTING GEOTECHNICAL ENGINEER

July 20, 2017

Town of Urbana
P.O. Box 186
8014 Pleasant Valley Road
Hammondsport, New York 14840

Attention: David Oliver

Subject: Geotechnical Engineering Investigation
Proposed Pedestrian Bridge and Boardwalk
Town of Urbana, Steuben County, New York
RMT-12900

Readers:

This report presents the results of a geotechnical engineering investigation for the project identified above.

UNDERSTANDING OF PROPOSED CONSTRUCTION

It is understood that the pedestrian bridge and boardwalk will be parts of a walking trail at the south end of Keuka Lake. The approximate bridge location and trail alignment are shown on the attached figure. Anticipated foundation loads are modest.

The bridge will cross Keuka Lake Inlet (also known as Cold Brook), will be approximately 90 feet long and 8 feet wide, and will have a dead weight of approximately 35,000 pounds.

The boardwalk will be approximately 500 to 600 feet long, and will cross a marsh/wetland south of Keuka Lake Inlet.

SUBSURFACE EXPLORATIONS AND LABORATORY TESTS

Subsurface Explorations

Subsurface explorations for this investigation consisted of four test borings, identified as B-1 through B-4. The approximate locations of the borings are shown on the attached figure. Borings B-1 and B-4 were respectively located at or near the north and south limits of the project.

P.O. BOX 316 PHONE 585-393-4762
CANANDAIGUA, NEW YORK 14424

The borings were performed by Nothnagle Drilling, using an ATV-mounted rotary drilling rig, between 7/10/17 and 7/12/17. Borings B-1 through B-4 were advanced to respective depths of 47, 30, 30, and 35 feet below the ground surface.

The logs of the borings, as prepared by Nothnagle Drilling, are attached.

Geotechnical Laboratory Tests

A limited program of geotechnical laboratory testing was performed on samples obtained from the borings, to assist in characterizing the subsurface conditions. The tests consisted of 42 natural water content determinations. The results are attached.

It should be noted that the natural water contents of soil are customarily performed on a dry-weight basis, and that water contents greater than 100 percent are therefore possible.

COMMENTS ON SUBSURFACE CONDITIONS

The encountered soils included varying amounts of silt, sand, and clay, with varying minor amounts of gravel. The soils are generally loose or soft.

Zones of cohesive soil (such as clay and clayey silt) and cohesionless soil (such as sand and non-plastic silt) were both encountered.

Many of the samples, at varying locations and depths, also contained varying amounts of organic matter.

Observations of down-hole water and sample moisture were made during the field program, and are noted on the logs.

It should be noted that short-term observations may not be representative of actual groundwater levels, and that groundwater levels will vary with factors including location, time, precipitation, season, and site activities. For purposes of analysis and design, it should be assumed that water will be at or above the ground surface.

An artesian condition was encountered by Boring B-1, and is described on the log.

More detailed descriptions of the subsurface conditions, as encountered by the borings, are provided on the attached logs.

DESIGN AND CONSTRUCTION

General

All design and construction should satisfy the requirements of all applicable codes.

Deep Foundations

It is recommended that the bridge and boardwalk be supported by deep foundations. Two feasible deep foundation types are drilled-in helical piles and driven timber piles. Additional deep foundation types may be feasible as well, and may be proposed by designers and/or contractors.

Any deep foundation should derive its vertical load capacity primarily by side resistance. Little or no tip resistance should be assumed. This recommendation reflects the presence of weak and compressible organic zones. In the case of a helical pile, no fewer than three helices should be used.

Overall, the ultimate vertical capacity of any deep foundation should be at least twice the allowable load. In addition, the foundation should be capable of withstanding the installation stresses.

For purposes of analysis and design, the moist or total unit weight of soil should be taken as 100 pounds per cubic foot. The buoyant or submerged unit weight should be taken as 40 pounds per cubic foot. As previously noted, it should be assumed for purposes of analysis and design that water will be at or above the ground surface.

The soils are neither purely frictional nor purely cohesive. For purposes of analysis and design, a duel approach is therefore recommended. The first approach would be based on a soil internal friction angle of 28 degrees and a cohesion of zero. The second approach would be based on a soil internal friction angle of zero and a cohesion of 300 pounds per square foot. The approach yielding the more conservative result (the lower load capacity) should be used.

Load capacities should in some way be verified during construction. In the case of a drilled-in helical pile, there are relationships involving load capacity and installation torque. In the case of a driven pile, the use of a pile driving analyzer (PDA) could be considered. Designers and/or contractors may suggest additional methods of load capacity verification.

The minimum center-to-center spacing of deep foundations should be 3 feet or 3 diameters, whichever is greater. The load-transfer connections between foundations and the overlying structural elements should be coordinated between the structural engineer and the foundation provider.

Deep foundations should be designed by an experienced engineer, and should be installed by an experienced specialty contractor. In many cases, the engineer and specialty contractor are within the same organization.

The recommendations above are intended to provide a reasonable level of confidence that widespread and noticeable settlements will not occur. The foundations required to achieve a significantly greater level of confidence would be impractically expensive. The possibility of noticeable settlement (due to the weak and compressible soil, and in particular the varying zones of organic matter), must be acknowledged. It is therefore recommended that the load-transfer connections between foundation and superstructure be designed and constructed in such a way that they can be adjusted, if necessary, in the event of settlement.

It is finally recommended that grades not be raised. That is, no fill should be placed above the level of the existing ground surface. Grade increases are likely to create excessive settlement.

CLOSING COMMENTS AND RECOMMENDATIONS

Professional services for this investigation were performed in accordance with generally accepted geotechnical engineering practices, exclusively for the subject project. No warranty, expressed or implied, is made.

Subsurface conditions are inferred from the logs of subsurface explorations. Conditions between, beyond, and below these explorations are likely to vary. It should also be noted that subsurface conditions are often described on the basis of visual examinations of recovered samples, and that these visual descriptions may not always agree well with descriptions made on the basis of laboratory tests. If subsurface conditions are subsequently revealed that appear to be significantly different or less favorable than those described, I should be given the opportunity to revise the statements in this report.

This report was prepared primarily for design purposes, and may not contain sufficient information for bidding. Contractors should visit the site, review this report and its attachments, and evaluate construction difficulties on the basis of their own knowledge and experience.

It is recommended that qualified personnel be retained to review the geotechnical portions of the contract drawings and specifications, and to provide monitoring services during construction.

If you have questions or comments regarding this report, please contact me.

Yours truly,

R M. Teeter

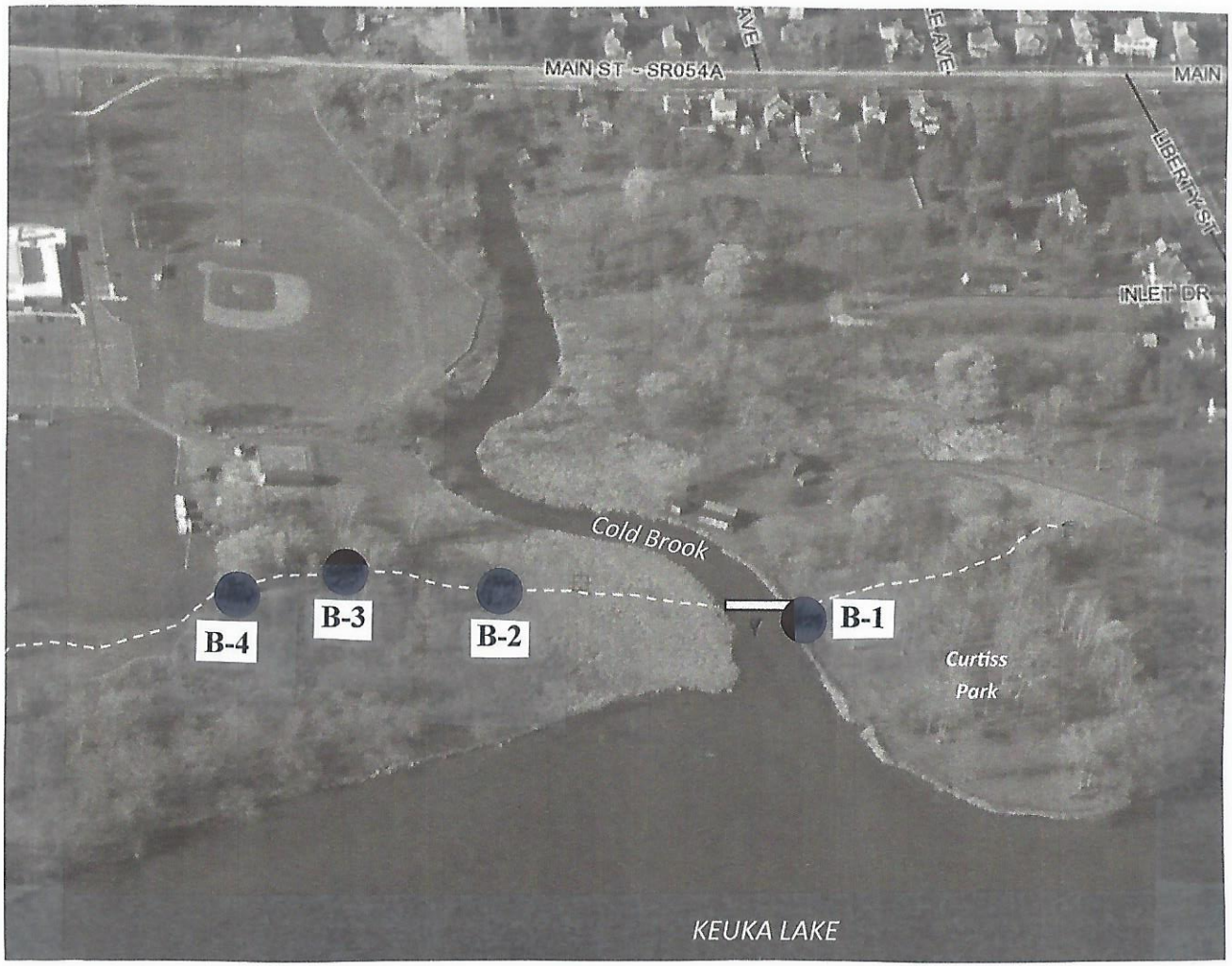
RAY M. TEETER, P.E.

Attachments: Figure Showing Approximate Boring Locations

Logs of Borings B-1 through B-4, by Nothnagle Drilling (5 pages)

Table of Natural Water Contents (3 pages)

APPROXIMATE BORING LOCATIONS



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Test Boring No. B-1
Page 1 of 2
ND Job # 174974

Project Bridge and Boardwalk Soil Study, Town of Urbana, New York
Client Town of Urbana, P.O. Box 186, Hammondsport, New York 14840
Elevation _____ Start 7/10/17 Completed 7/10/17 Driller T. Mangefrida
Water Level - During Drilling 1'8" Inspector _____
Water Level - At Completion _____
Seasonal and climatic changes may alter observed water levels.

	C	Blows on Sampler 0"/6"	6"/12"	12"/18"	18"/24"	Sample N	Rec.	No.	Depth	Visual Soil and Rock Information Remarks	
0		1	1							Topsoil and organics	0'5"
				WH	WH	1	5"	1	0'0"-2'0"	Loose black-brown moist organics, some silt, little gravel (wet @ 1'8")	
		1	WH								
				WH	WH	WH	6"	2	2'0"-4'0"	Loose black-brown wet	
5		WH	WH							Loose black-brown wet	4'3"
				2	2	2	11"	3	4'0"-6'0"	Very soft blue-gray moist silty clay, trace fine gravel and organics	
		WH	1								
				2	3	3	15"	4	6'0"-8'0"	Soft blue-gray moist	6'8"
		2	3							Loose blue-gray moist sand and silt, little fine gravel, trace organics (wood)	
10				4	5	7	3"	5	8'0"-10'0"		
		2	2							Loose blue-gray moist (some coarse to fine gravel)	
				1	1	3	8"	6	10'0"-12'0"	Loose blue-gray moist	11'0"
										Loose brown moist silt, some fine sand, trace fine gravel and organics	15'0"
15											
		2	1							Loose gray saturated fine sand, trace silt and organics	
				2	2	3	13"	7	15'0"-17'0"		
20											20'0"
		WH	WH							Loose gray saturated fine sand and silt	
				1	1	1	10"	8	20'0"-22'0"		
25											25'0"
		WR	WR							Loose gray saturated medium sand, trace fine gravel	
				WH	WH	WH	16"	9	25'0"-27'0"		
										(@ 30' formation blew to 20', switched to rotary wash to obtain samples below 30')	
30											
		2	4							Loose gray saturated	
				2	4	6	12"	10	30'0"-32'0"		
35											35'0"
		2	8							Firm gray saturated fine sand, little silt	
				11	13	19	14"	11	35'0"-37'0"		
40											40'0"

N=No. of Blows to Drive 2" Spoon 12" with 140 lb. Wt. 30" Ea. Blow
C=No. of Blows to Drive__ Casing___with___ lb. Wt.___ Ea. Blow
Transitional Depths are Estimated Based on Field Observations

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1821 Scottsville-Mumford Road
Scottsville. New York 14546
Phone (585) 538-2328
Fax (585) 538-2357

Test Boring No. B-1
Page 2 of 2
ND Job # 174974

Project Bridge and Boardwalk Soil Study, Town of Urbana, New York
Client Town of Urbana, P.O. Box 186, Hammondsport, New York 14840
Elevation _____ Start 7/10/17 Completed 7/10/17 Driller T. Mangefrida
Water Level - During Drilling 1'8" Inspector _____
Water Level - At Completion _____
Seasonal and climatic changes may alter observed water levels.

	C	Blows on Sampler 0"/6"	6"/12"	12"/18"	18"/24"	Sample N	Rec.	No.	Depth	Visual Soil and Rock Information Remarks
40		2	3							Loose gray saturated medium sand, trace silt and fine gravel
				4	6	7	12"	12	40'0"-42'0"	
45										
		3	3							Loose gray saturated
				6	8	9	13"	13	45'0"-47'0"	(@ 47' formation blew to 30' with artesian flow. Static water @ +4'6") 47'0"
50										
55										
60										
65										
70										
75										
80										

Boring terminated at 47'0"
Advanced test boring with hollow stem auger casing to 30'0" and 3 7/8" rotary 30'0" - 45'0"
Boring grout abandoned on completion.

Location checked on 7/12/17
No artesian flow noticed.

N=No. of Blows to Drive 2" Spoon 12" with 140 lb. Wt. 30" Ea. Blow
C=No. of Blows to Drive__ Casing___with___ lb. Wt.___ Ea. Blow
Transitional Depths are Estimated Based on Field Observations

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1821 Scottsville-Mumford Road
Scottsville. New York 14546
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Test Boring No. B-2
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Project Bridge and Boardwalk Soil Study, Town of Urbana, New York
Client Town of Urbana, P.O. Box 186, Hammondsport, New York 14840
Elevation _____ Start 7/11/17 Completed 7/11/17 Driller T. Mangefrida
Water Level - During Drilling 15'0" Inspector _____
Water Level - At Completion 12'0"
Seasonal and climatic changes may alter observed water levels.

	C	Blows on Sampler 0"/6"	6"/12"	12"/18"	18"/24"	Sample N	Rec.	No.	Depth	Visual Soil and Rock Information Remarks
0		WH	WH							Loose brown moist silt, little organics,
				WH	1	WH	10"	1	0'0"-2'0"	trace clay 1'8"
		1	3							Very soft brown-gray moist silty clay, trace
				4	5	7	15"	2	2'0"-4'0"	organics
5		1	2							Medium brown-gray moist
				3	3	5	14"	3	4'0"-6'0"	Soft brown-gray damp
		5	7							Medium brown-gray damp
				6	6	13	16"	4	6'0"-8'0"	
		WH	WH							Very soft brown-gray damp
10				WH	4	WH	20"	5	8'0"-10'0"	
		WH	1							Soft brown-gray damp
				2	3	3	20"	6	10'0"-12'0"	
15										15'0"
		1	1							Loose gray saturated medium to fine sand,
				WH	1	1	18"	7	15'0"-17'0"	some silt
20										20'0"
		WH	WH							Loose brown-gray moist silt, little organics,
				3	2	3	12"	8	20'0"-22'0"	trace clay
25										
		WH	WH							Loose brown-gray moist
				3	7	3	16"	9	25'0"-27'0"	
										(@ 30' formation blew to 15')
30										30'0"
										Boring terminated at 30'0"
35										Advanced test boring with hollow
										stem auger casing.
										Boring backfilled on completion.
40										

N=No. of Blows to Drive 2" Spoon 12" with 140 lb. Wt. 30" Ea. Blow
C=No. of Blows to Drive__ Casing___with___ lb. Wt.___ Ea. Blow
Transitional Depths are Estimated Based on Field Observations

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Test Boring No. B-3
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ND Job # 174974

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Client Town of Urbana, P.O. Box 186, Hammondsport, New York 14840
Elevation ____ Start 7/11/17 Completed 7/11/17 Driller T. Mangefrida
Water Level - During Drilling 20'0" Inspector ____
Water Level - At Completion 6'4"
Seasonal and climatic changes may alter observed water levels.

	C	Blows on Sampler 0"/6"	6"/12"	12"/18"	18"/24"	Sample N	Rec.	No.	Depth	Visual Soil and Rock Information Remarks	
0		WH	WH							Topsoil and organics	0'6"
				2	2	2	15"	1	0'0"-2'0"	Loose brown-gray damp silt, trace clay and organics	
		2	3								
				2	3	5	14"	2	2'0"-4'0"	Loose brown-gray damp	2'9"
5		1	2							Loose brown-gray damp silt, little clay, trace coarse gravel and organics	
				2	3	4	16"	3	4'0"-6'0"		
		4	4							Loose brown-gray damp	
				4	3	8	18"	4	6'0"-8'0"	Loose brown-gray damp	6'3"
		1	2							Medium gray-brown damp silty clay, trace organics	
10				3	2	5	12"	5	8'0"-10'0"		
		WH	1							Soft gray-brown damp	8'9"
				2	2	3	19"	6	10'0"-12'0"	Loose gray-brown damp silt, some fine sand, little coarse to fine gravel	10'0"
										Loose gray damp silt, trace clay and organics	
15											15'0"
		WH	WH							Loose brown damp silt, little organics	
				2	2	2	20"	7	15'0"-17'0"		
20											20'0"
		WH	WH							Loose gray saturated sand, some silt	
				WH	2	WH	18"	8	20'0"-22'0"		
25											25'0"
		1	2							Loose gray saturated coarse to fine sand, trace fine gravel	
				2	1	4	16"	9	25'0"-27'0"		
30										(@ 30' formation blew to 10'	30'0"
										Boring terminated at 30'0" Advanced test boring with hollow stem auger casing. Boring backfilled on completion.	
35											
40											

N=No. of Blows to Drive 2" Spoon 12" with 140 lb. Wt. 30" Ea. Blow
C=No. of Blows to Drive__ Casing___with___ lb. Wt.___ Ea. Blow
Transitional Depths are Estimated Based on Field Observations

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Test Boring No. B-4
Page 1 of 1
ND Job # 174974

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Client Town of Urbana, P.O. Box 186, Hammondsport, New York 14840
Elevation ____ Start 7/12/17 Completed 7/12/17 Driller T. Mangefrida
Water Level - During Drilling 31'0" Inspector ____
Water Level - At Completion 8'6"
Seasonal and climatic changes may alter observed water levels.

	C	Blows on Sampler 0"/6"	6"/12"	12"/18"	18"/24"	Sample N	Rec.	No.	Depth	Visual Soil and Rock Information Remarks	
0		1	1							Topsoil and organics	0'7"
				2	3	3	13"	1	0'0"-2'0"	Loose brown-gray damp silt, trace fine gravel and organics	
		2	2								
				3	2	5	17"	2	2'0"-4'0"	Loose brown-gray damp	2'3"
5		2	2							Loose brown-gray damp silt, trace clay and organics	
				1	1	3	7"	3	4'0"-6'0"		
		3	3							Loose brown-gray damp	4'2"
				4	3	7	14"	4	6'0"-8'0"	Loose blue-gray damp silt, some fine gravel, trace sand and clay	
		WH	WH								
10				WH	2	WH	18"	5	8'0"-10'0"	Loose blue-gray damp (organics noted below 7'6")	
		WH	1							Loose blue-gray damp	8'6"
				2	3	3	16"	6	10'0"-12'0"	Loose blue-gray moist silt, little clay, trace fine gravel and organics	
										Loose blue-gray moist	
15											15'0"
		WH	WH							Very soft blue-gray damp silty clay, trace organics	
				2	3	2	18"	7	15'0"-17'0"		
20											20'0"
		WH	WH							Loose brown damp silt, little organics	
				WH	1	WH	16"	8	20'0"-22'0"		
25										Loose brown damp	26'4"
		1	1							Loose gray moist silt and fine gravel, trace organics	
				1	1	2	18"	9	25'0"-27'0"		
30										Loose gray moist	31'1"
		1	2							Loose gray saturated fine sand and silt	
				3	3	5	19"	10	30'0"-32'0"		
35										(@ 35' formation blew to 25')	35'0"
										Boring terminated at 35'0" Advanced test boring with hollow stem auger casing. Boring backfilled on completion.	
40											

N=No. of Blows to Drive 2" Spoon 12" with 140 lb. Wt. 30" Ea. Blow
C=No. of Blows to Drive__ Casing___with___ lb. Wt.___ Ea. Blow
Transitional Depths are Estimated Based on Field Observations

Natural Water Contents
Proposed Pedestrian Bridge and Boardwalk
Town of Urbana, Steuben County, New York
(Page 1 of 3)

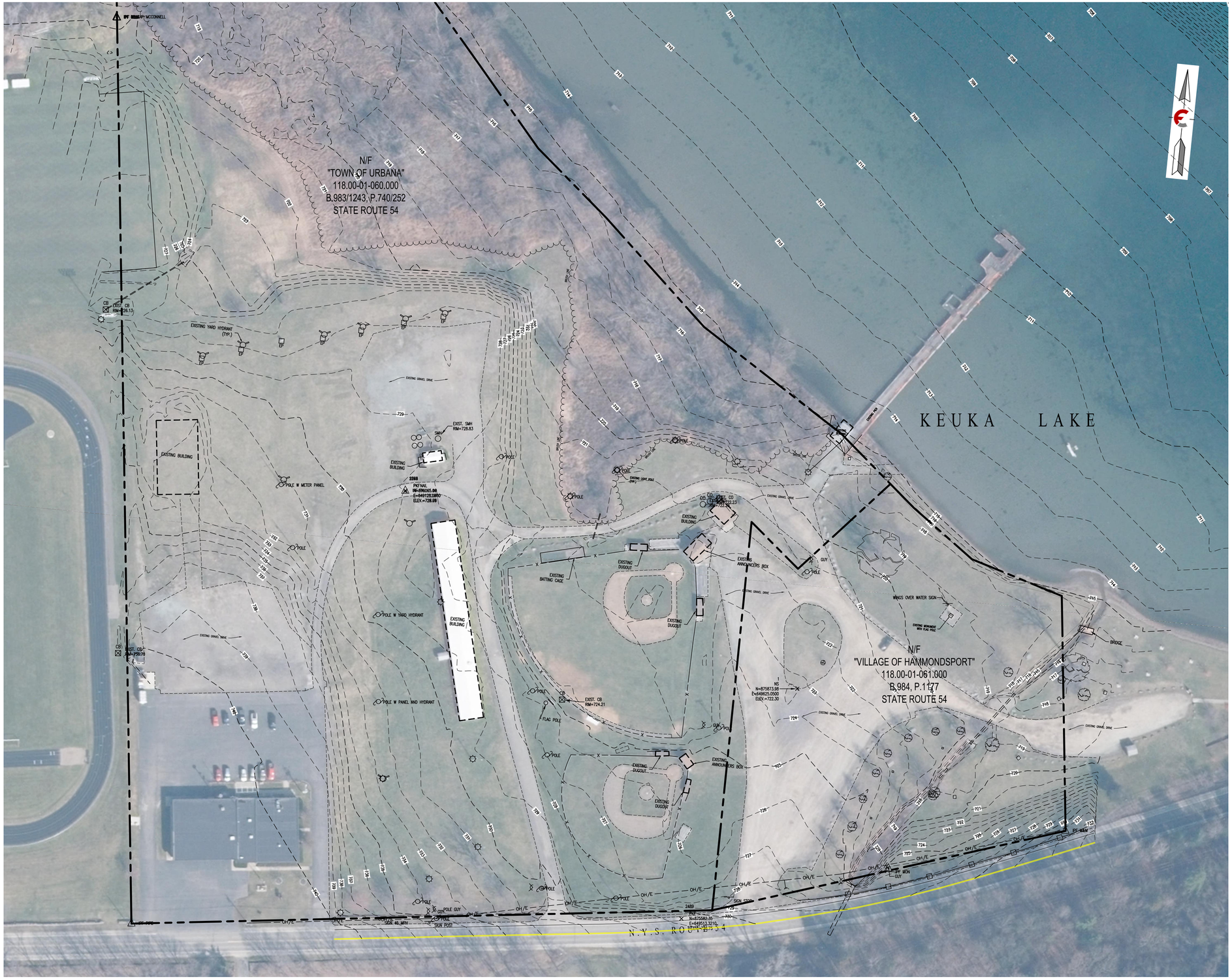
Test Boring Number	Sample Number	Sample Depth (feet)	Natural Water Content (percent)
B-1	1	0-2	49
B-1	2	2-4	111
B-1	3	4-6	28
B-1	4	6-8	62
B-1	5	8-10	22
B-1	6	10-12	67
B-1	7	15-17	45
B-1	8	20-22	47
B-1	9	25-27	27
B-1	10	30-32	18
B-1	11	35-37	23
B-1	12	40-42	21
B-1	13	45-47	26
B-2	1	0-2	42
B-2	2	2-4	26
B-2	3	4-6	27
B-2	4	6-8	30

Natural Water Contents
Proposed Pedestrian Bridge and Boardwalk
Town of Urbana, Steuben County, New York
(Page 2 of 3)

Test Boring Number	Sample Number	Sample Depth (feet)	Natural Water Content (percent)
B-2	5	8-10	36
B-2	6	10-12	42
B-2	7	15-17	28
B-2	8	20-22	44
B-2	9	25-27	34
B-2	Augers	27-30	26
B-3	1	0-2	33
B-3	2	2-4	27
B-3	3	4-6	29
B-3	4	6-8	29
B-3	5	8-10	26
B-3	6	10-12	32
B-3	7	15-17	136
B-3	8	20-22	32
B-3	9	25-27	21
B-4	1	0-2	23
B-4	2	2-4	29

Natural Water Contents
Proposed Pedestrian Bridge and Boardwalk
Town of Urbana, Steuben County, New York
(Page 3 of 3)

Test Boring Number	Sample Number	Sample Depth (feet)	Natural Water Content (percent)
B-4	3	4-6	24
B-4	4	6-8	16
B-4	5	8-10	31
B-4	6	10-12	33
B-4	7	15-17	31
B-4	8	20-22	59
B-4	9	25-27	39
B-4	10	30-32	29



LEGEND

- PROPERTY LINE
- EXISTING EASEMENT
- EXISTING EDGE OF ROADWAY
- EXISTING CURB LINE
- SAN — EXISTING SANITARY SEWER
- G — EXISTING GAS MAIN
- UG/T/T/C — EXISTING UTILITY LINE (UG–UNDERGROUND, OH–OVERHEAD, T–TELEPHONE, E–ELECTRIC, C–CABLE)
- x — EXISTING FENCE LINE
- W — EXISTING WATER LINE
- 932 — EXISTING CONTOUR LINE
- LOD — PROPOSED LIMIT OF DISTURBANCE
- 99 — PROPOSED CONTOUR LINE
- PROPOSED EASEMENT
- ST — PROPOSED STORM SEWER
- PROPOSED EDGE OF ROADWAY
- PROPOSED CURB LINE
- SAN — PROPOSED SANITARY SEWER
- G — PROPOSED GAS LINE
- UG/T/T/C — PROPOSED UTILITY LINE (UG–UNDERGROUND, OH–OVERHEAD, T–TELEPHONE, E–ELECTRIC, C–CABLE)
- W — PROPOSED WATER LINE
- SF — PROPOSED SILT FENCE
- CS — PROPOSED COMPOST SOCK
- SMH — EXISTING SANITARY MANHOLE
- EXISTING FIRE HYDRANT ASSEMBLY
- CO — EXISTING CLEANOUT
- 99.50 x — EXISTING SPOT ELEVATION
- EP — EXISTING ELECTRIC PANEL
- SMH — PROPOSED SANITARY MANHOLE
- WV — PROPOSED WATER VALVE
- TB — PROPOSED THRUST BLOCK
- PROPOSED FIRE HYDRANT ASSEMBLY
- CO — PROPOSED CLEANOUT
- x — PROPOSED LIGHTING FIXTURE
- 99.42 — PROPOSED SPOT ELEVATION
- PROPOSED DRYWELL
- PROPOSED CATCH BASIN
- PROPOSED INLET PROTECTION

CONCEPT PLAN

Rev.	Date	Revision Description
1.		

SEAL

Hammondsport-Urbana Waterfront Project

Urbana (T), Steuben (Co.), New York

FAGAN ENGINEERS
& LAND SURVEYORS PC
113 East Chemung Place
Elmira N.Y. 14904
Phone (607) 734-2165
Fax (607) 734-2169
www.FaganEngineers.com

Scale:	######## 11x17 Prints are 1/2 Size
Date:	4/11/2019
Design By:	BMG
Drawn By:	BMG
Checked By:	JBG
Project No.:	2019.016-001
Drawing Name:	19016-a.dwg

EXISTING CONDITIONS

C1

MURK 1
(06/08)

DAILY WORK REPORT

JOB STAMP
Town of Urbana
Steuben County, New York
Urbana Pile Driving Inspection
Contractor - R and R Docks
Fagan Engineers Project Number: 2019.016.003

Date: April 02, 2020

Day of Week: S | M | T | W | X | F | S

Sheet No. 1 of 4 DWR No. 1

	AM		PM	
Weather	Clear		Cloudy	
Temperature	35	° F	51	° F

CONTRACTOR WORK HOURS: 8:00 am TO 3:00 pm

DESCRIPTION OF WORK AND MATERIAL USED FOR EACH OPERATION, INCLUDING CONTRACTOR/SUB NAME, ITEM NO. AND LOCATION

Contractor R and R Docks (R&R) was onsite today to perform work at the location of the existing pier at Champlin Beach Park in the Town of Urbana. Work consisted of performing the following tasks under the direction/supervision of Wendy Meagher, PE of Meagher Engineering.
Task #1 - Placement of two (2) - 7" diameter x 40' long steel pipe piles.
Task #2 - Removal of existing wood piles from the existing pier to investigate embedment length.
Task #3 - Test the driven test piles for "Pipe Freeze"
Task #4 - Pull test piles based on satisfied results.

Task #1 - R&R had their equipment mobilized and began required work at approximately 8:00 am by installing the two (2) - 7" diameter x 40' long steel pipe piles at the previously determined Primary and Secondary locations. The initial pile was installed at the Primary location which was in line with the outermost structure line of the existing pier, 42.3' away from the Northwest corner of the pier. The actual time expended to vibrate the test pile to a depth that allowed a small section to remain above the waterline was recorded at 59 seconds. The barge was relocated to the Secondary location which was 67.9' away from the existing pier reference, 100' away from the Primary Test location. The actual time expended to vibrate this test pile to a depth that also allowed a small section to remain above the waterline was recorded at 125 seconds. This Task was completed at approximately 9:05 am. Refer to attached Page 2 for photographs of the above described work.

CONTRACT PAY ITEM NO	FS	ES	INTERIM QUANTITY	UNITS	FINAL QUANTITY	QTY CHK	LOCATION OF WORK	CEES ENT	CEES CHK

I certify that the work described in this report was incorporated into this contract on the date of this DWR, unless otherwise noted.

Inspector's Signature: [signature]

Date Prepared: 4/02/20

Reviewed by: _____

☐ Engineer-in-Charge
☐ Resident Engineer

Date Reviewed: _____

☐ MURK 1-1 CONTINUATION attached for additional narrative. ☐ MURK 1-2 CONTINUATION attached for additional pay items & quantities.

Task #2 - R&R repositioned the barge adjacent to the existing pier to remove existing piles to investigate embedment length. A total of two (2) existing piers where removed, the first was an old mooring pile located along the western side of the existing pier and the second pile removed was located under the Northeast corner of the existing pier. The first pile removed was measured at 22' and the second was measured at 27' 6". The field measured embedment length for both removed piles was recorded at 15'. This Task was completed at approximately 9:50 am. Refer to attached Page 3 for photographs of the above described work.

Task #3 & #4 - The installed Test Piles were allowed to sit until 12:00 pm (Noon) at which time the "Pipe Freeze" tests were performed. The barge was repositioned adjacent to Test Pile #1 and the excavator loaded the pile. The loading duration was recorded at approximately 10 minutes with satisfactory results. It was decided to pull this test pile prior to relocating to Test Pile #2.Following removal, the barge was repositioned adjacent to Test Pile #2 and the excavator loaded the pile. The loading duration was also recorded at approximately 10 minutes with satisfactory results. R&R proceeded to pull this final test pile. Task #3 & #4 were completed at 3:00 pm (estimated). Refer to attached Page 4 for photographs of the above described work.

It should be noted that following the above described work, the requested photographic survey of Champlin Beach from the locations referenced in an email was performed and the results will be forwarded via email & Sharefile. Arrangements were unable to be made to gain access to the interior of the building on photos 3-7 so those requested photos will need to be completed at a later date.

The following were on site today during the above described work:
Wendy Meagher, PE - Meagher Engineering
Gene Cafolla - Fagan Engineers
Chad Robbins - R and R Docks
John Miller - R and R Docks
Dave Oliver, Town of Urbana
Dave Shaw, Town of Urbana

LABOR					EQUIPMENT					
CLASSIFICATION	OP 1	OP 2	OP 3	OP 4	TYPE	ID #	OP 1	OP 2	OP 3	OP 4
FOREPERSON	1				Self Propelled Barge		1			
LABORER					Excavator (Doosan DX85R)		1			
EQUIP. OPERATOR	1									
TRUCK DRIVERS										
IRONWORKERS										
CARPENTERS										
MASONS										
OWN./OPERATORS										

OP 1: R and R Docks	OP 2:	OP 3:	OP 4:

URBANA PILE DRIVING INSPECTION
TEST PILES FOR CHAMPLIN PIER

1.) Looking North. A view of the barge moving into position to install the Test Pile at the Primary Location off the Northwest corner of the existing Pier.

2.) Looking Northwest. A view of the Test Pile at the Primary Location being positioned at the proposed location approximately 42.3' away from the Northwest corner of the existing Pier.

3.) Looking West. A view of the Test Pile at the Secondary Location being positioned at the proposed location approximately 67.9' away from the existing Pier reference.

4.) Looking West. Another view of the Test Pile at the Secondary Location being driven by the hydraulic vibratory attachment on the DX85R Doosan Excavator w/thumb.

URBANA PILE DRIVING INSPECTION
TEST PILES FOR CHAMPLIN PIER

5.) Looking Northeast. A view of the barge in position off of the Northeast corner of the existing Pier in the process of removing an existing wood pile to determine embedment of pile.

6.) Looking Northwest. A closeup view of the existing wood pile removal. Note the previously removed wood pile lying on the barge.

7.) Looking Northwest. The existing wood pile located at the Northeast corner of the existing Pier almost removed.

8.) Looking Northwest. The existing wood pile removed from the existing Pier being placed on the barge.

URBANA PILE DRIVING INSPECTION
TEST PILES FOR CHAMPLIN PIER

9.) Looking Northwest. A view of the barge approaching the previously installed Test Pile at the Primary Location to perform a test to evaluate "Pipe Freeze".

10.) Looking Northwest. A closeup view of the barge adjacent to the previously installed Test Pile at the Primary Location performing the test to evaluate "Pipe Freeze".

11.) Looking West. The excavator removing the Test Pile at the Primary Location following satisfactory results from the "Pipe Freeze" test.

12.) Looking West. A closeup view of the barge adjacent to the previously installed Test Pile at the Secondary Location performing the test to evaluate "Pipe Freeze".