

BID ADDENDUM NO. (4)

3/13/2026

Prattsburgh Central School District
2025 Capital Improvements Project
2716-043

(SED #57-23-01-04-0-001-034 – Main Building)
(SED #57-23-01-04-0-008-012 – Agricultural Building)
(SED #57-23-01-04-0-002-012 – Auxiliary Building)
(SED #57-23-01-04-7012-001 – Toilet Room Facility)
(SED #57-23-01-04-2-005-001 – Home Dugout)

The following Addendum items shall be considered a part of the contract documents prepared by HUNT ENGINEERS, ARCHITECTS, LAND SURVEYORS & LANDSCAPE ARCHITECT, DPC. Bid Document date of (12/18/2025).

Clarifications issued by this Addendum:

1. N/A

Project Manual Sections issued by this Addendum:

00 31 32A – Geotechnical Report
22 10 05 – Plumbing Piping and Specialties
28 20 00 – Video Surveillance
33 14 16 – Water Utility Distribution Piping

Drawings issued by this Addendum:

AG-A6.1 – ROOF PLAN
AD4-H1 – MODIFICATIONS TO 2ND FLOOR - AREA C
AD4-H2 – MODIFICATIONS TO 2ND FLOOR - AREA C
AD4-P1 – REVISED FIRST FLOOR SANITARY PLAN – AREA C
AD4-P2 – REVISED FIRST FLOOR DOMESTIC PLAN – AREA C
AD4-P3 – UV SCHEMATIC
AD4-L1 – ELJEN PROFILE DETAIL
AD4-L2 – BASEBALL FENCE PERIMETER
AD4-L3 – DISTRIBUTION BOX DETAILS

AD4-T1 – TR OSP DISTRIBUTION DIAGRAM

AD4-T2 – TR ACCESS CONTROL PLAN

Revisions to Project Manual issued by this Addendum:

ITEM AD4-1 Refer to Section 00 31 32A – Geotechnical Report

ADD Specification Section 00 31 32A- Geotechnical Report, issued by this addendum.

ITEM AD4-2 Refer to Section 01 10 00 - Summary

AMEND Section 1.12., A., 12. to read as: “Provide the complete work of Division 28 – Electronic Safety and Security with the following exceptions:”

AMEND Section 1.12., A., 12., a. to read as: “Specification Section 28 10 00 – Access Control by Access/Security Co-Op Contractor.

ADD Section 1.12., A., 12., b. to read as: “Specification Section 28 20 00 – Video Surveillance:”

ADD Section 1.12., A., 12., b., 1) to read as: “EC to provide conduit, pathways and cabling for video surveillance equipment.”

ADD Section 1.12., A., 12., b., 2) to read as: “Access/Security Co-Op Contractor to Furnish and install all equipment.”

ITEM AD4-3 Refer to Section 01 10 00 - Summary

AMEND Section 1.14, A., 1., b. to read as: “Provide the complete work of Specification Section 28 20 00 – Video Surveillance, with the following exceptions:”

ADD Section 1.14, A, 1., b., 1) to read as: “Conduit, pathways and cable provided by Electrical Contractor.”

ITEM AD4-4 Refer to Section 22 10 05 – Plumbing Piping and Specialties

DELETE Specification 22 10 05 – Plumbing Piping and Specialties

ADD Specification 22 10 05 – Plumbing Piping and Specialties, as issued by this addendum.

ITEM AD4-4 Refer to Section 27 05 28 – Pathways For Communications Systems

DELETE Specification 27 05 28 – Pathways For Communications Systems

ITEM AD4-5 Refer to Section 28 20 00 – Video Surveillance

DELETE Specification 22 10 05 – Video Surveillance

ADD Specification 22 10 05 – Video Surveillance, as issued by this addendum.

ITEM AD4-3 Refer to Section 33 14 16 – Water Utility Distribution Piping

DELETE Specification 33 14 16 – Water Utility Distribution Piping

ADD Specification 33 14 16 – Water Utility Distribution Piping, as issued by this addendum.

Revisions to Drawings issued by this Addendum:

ITEM AD4-6 Refer to AG-A6.1 – ROOF PLAN

DELETE Drawing AG-A6.1 – ROOF PLAN in its entirety.

ADD Drawing AG-A6.1 – ROOF PLAN as issued by this addendum.

ITEM AD4-7 Refer to AX-A1.1 – AUXILIARY ROOF PLANS

AMEND Demolition note D3 to read:

“D3 MECHANICAL CONTRACTOR TO REMOVE MECHANICAL EQUIPMENT. DISCONNECT DUCTWORK, GAS, AND CONTROLS. ROOF CURB TO REMAIN. ELECTRICAL CONTRACTOR TO DISCONNECT AND RECONNECT AS REQUIRED FOR ROOFING SCOPE COMPLETION. GENERAL TRADES CONTRACTOR SHALL PREPARE FOR ROOF MEMBRANE TIE IN. REFER TO NEW WORK PLANS FOR ROOFING SYSTEM INFORMATION.”

ITEM AD4-8 Refer to AX-A1.1 – AUXILIARY ROOF PLANS

AMEND Demolition note D4 to read:

“D4 MECHANICAL CONTRACTOR TO RECLAIM REFRIGERANT, DISCONNECT, REMOVE AND RETAIN REFRIGERANT LINES FOR REINSTALLATION. REMOVE BLOCKING AND PIPE SUPPORTS.”

ITEM AD4-9 Refer to AX-A1.1 – AUXILIARY ROOF PLANS

AMEND Roof drawing note 2 to read:

“2 COMPLETE RE-ROOF. REFER TO ROOF LEGEND FOR ROOFING SYSTEM DETAILS. VERIFY EXISTING ROOF SLOPE IN FIELD. PROVIDE HIGH SIDE TAPERED INSULATION AT ¼” MINIMUM SLOPE AT ALL EXISTING CURB AND EQUIPMENT LOCATIONS.”

ITEM AD4-10 Refer to AX-A1.1 – AUXILIARY ROOF PLANS

AMEND Roof drawing note 3 to read:

“3 MECHANICAL CONTRACTOR TO REINSTALL MECHANICAL EQUIPMENT, EXTEND AND RECONNECT DUCTWORK, GAS AND CONTROLS. ELECTRICAL CONTRACTOR TO DISCONNECT AND RECONNECT AS REQUIRED FOR ROOFING SCOPE COMPLETION. V.I.F. FOR EXACT LOCATION.”

ITEM AD4-11 Refer to AX-A1.1 – AUXILIARY ROOF PLANS

AMEND Roof drawing note 6 to read:

“6 VENT LOCATION. RECONNECT AND EXTEND TO MINIMUM 18” ABOVE FINISHED MEMBRANE TYP.”

ITEM AD4-12 Refer to AX-A1.1 – AUXILIARY ROOF PLANS

AMEND Roof Legend new roofing description to read: “FULLY ADHERED EPDM MEMBRANE ON 1/4” COVERBOARD OVER THREE (3) LAYERS OF 2” RIGID INSULATION. INSULATION TO BE FULLY ADHERED TO ONE LAYER OF A 40 MIL RUBBERIZED ASPHALTIC VAPOR RETARDER OVER 1/2” ROOF UNDERLAYMENT MECHANICALLY FASTENED TO METAL DECK. SYSTEM TO ACHIEVE AN AVERAGE R VALUE OF 36; NO LESS THAN R30 MIN PER IECC. SLOPED STRUCTURE BELOW TO ACHIEVE POSITIVE FLOW TO ROOF DRAINS AT A MINIMUM SLOPE OF 1/8” PER FOOT.”

ITEM AD4-13 Refer to AX-A1.1 – AUXILIARY ROOF PLANS

AMEND Roof drawing note 5 to read:

“5 MECHANICAL CONTRACTOR TO REINSTALL REFRIGERANT PIPING AFTER COMPLETION OF ROOF WORK, EXTEND AND RECONNECT LINES. PROVIDE NEW PIPE SUPPORTS AND RECHARGE REFRIGERANT.”

ITEM AD4-14 Refer to MB-H0.6 – SECOND FLOOR DEMOLITION PLAN – AREA B & C

ADD Scope of work shown on AD4-H1 – MODIFICATIONS TO 2ND FLOOR - AREA C issued in this addendum.

ITEM AD4-15 Refer to MB-H1.4 – SECOND FLOOR NEW WORK PLAN – AREA B & C

ADD Scope of work shown on 1/AD4-H2 – MODIFICATIONS TO 2ND FLOOR - AREA C issued in this addendum.

ITEM AD4-16 Refer to MB-H3.1 – SCHEDULE

ADD Fin tube radiation schedule shown on AD4-H2 – MODIFICATIONS TO 2ND FLOOR - AREA C issued in this addendum.

ITEM AD4-17 Refer to Drawing MB-P1.1 – FIRST FLOOR SANITARY PLANS

AMEND Detail 1 as shown on drawing AD4-P1 – REVISED FIRST FLOOR SANITARY PLAN – AREA C, as issued by this addendum.

ITEM AD4-18 Refer to Drawing MB-P2.1 – FIRST FLOOR DOMESTIC PLANS

AMEND Detail 1 as shown on drawing AD4-P2 – REVISED FIRST FLOOR DOMESTIC PLAN – AREA C, as issued by this addendum.

ITEM AD4-19 Refer to Drawing L6.4 – SITE DETAILS

DELETE Detail 8 – ABSORPTION TRENCH SYSTEM DETAIL in its entirety.

ITEM AD4-20 Refer to Drawing L2.1 – SITE IMPROVEMENT PLAN

AMEND Detail 1 as shown on drawing AD4-L2 – BASEBALL FENCE PERIMETER, as issued by this addendum.

ITEM AD4-21 Refer to Drawing TR-P1.1 – FIRST FLOOR PLANS

DELETE Drawing AD2-P4 – UV SCHEMATIC in its entirety.

ADD Drawing AD4-P3 – UV SCHEMATIC as issued by this addendum.

ITEM AD4-22 Refer to Drawing L6.2 – SITE DETAILS

AMEND Detail 11 with AD4-L1 – ELJEN PROFILE DETAIL, as issued by this addendum.

ITEM AD4-23 Refer to Drawing L6.4 – SITE DETAILS

AMEND Detail 7- 3 HOLE DISTRIBUTION BOX DETAIL and Detail 9- 5 HOLE DISTRIBUTION BOX DETAIL with AD- L3 – DISTRIBUTION BOX DETAILS, as issued by this addendum.

ITEM AD4-24 Refer to Drawing AG-P1.1 – FIRST FLOOR AND ROOF PLANS

AMEND CONSTRUCTION NOTE 5 to read as: “CONNECT SINK TO EXISTING COLD WATER AND HOT WATER INSIDE THE BUILDING WITH ISOLATION VALVES. CONNECT TO EXISTING SANITARY AND VENT PIPING. EXTEND PIPING AS NEEDED”, as issued by this addendum.

ITEM AD4-25 Refer to Drawing TR-T1.1 – TECHNOLOGY PLAN

ADD Diagram AD4-T1 –TR OSP DISTRIBUTION DIAGRAM, as issued by this addendum.

ITEM AD4-26 Refer to Drawing TR-T1.1 – TECHNOLOGY PLAN

ADD Add Construction Note 5 to read:

“5 ACCESS CONTROL BY CONTROLS CONTRACT.”

End of Addendum (4)



Buffalo
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atlantictesting.com

January 22, 2026

Prattsburgh Central School District

c/o Hunt Engineers, Architects, Land Surveyors, and Landscape Architect, DPC
100 Hunt Center
Horseheads, New York, 14845

Attn: Sarah Morrisey, RLA
Landscape Architect

Re: Subsurface Investigation and Geotechnical Evaluation
Proposed Baseball Field Improvements
7274 Prattsburgh Pulteney Road (County Route 74)
Town of Prattsburgh, Steuben County, New York
ATL Report No. BD298E-02-07-25

Sarah Morrisey:

Enclosed is one (1) electronic copy of the referenced report. ATL appreciates the opportunity to provide geotechnical services for this project.

Please note that upon completion of the subsurface investigation, the borings and infiltration test holes were backfilled with on-site soils and the surface was patched as appropriate. It is important that the backfilled borings and infiltration test holes be monitored for settlement or subsidence. This will be the responsibility of Prattsburgh Central School District. ATL assumes no liability for loss or damage resulting from borehole settlement.

The soil samples obtained during this investigation will be retained for a period of six months and subsequently discarded, unless otherwise instructed.

Please contact our office should you have any questions or comments on this information, or if we may be of further service. We look forward to our continued association to obtain a successful completion of this project.

Sincerely,
ATLANTIC TESTING LABORATORIES, Limited

Thomas R. Seider, PE
Senior Engineer

TRS/sb

Enclosures

**SUBSURFACE INVESTIGATION
AND
GEOTECHNICAL EVALUATION**

**PROPOSED BASEBALL FIELD IMPROVEMENTS
PRATTSBURGH CENTRAL SCHOOL DISTRICT
7274 PRATTSBURGH PULTENEY ROAD (COUNTY ROUTE 74)
TOWN OF PRATTSBURGH, STEUBEN COUNTY, NEW YORK**

PRATTSBURGH CENTRAL SCHOOL DISTRICT

**PREPARED FOR: Prattsburgh Central School District
c/o Hunt Engineers, Architects, Land Surveyors, and
Landscape Architect, DPC
100 Hunt Center
Horseheads, New York, 14845**

**PREPARED BY: Atlantic Testing Laboratories, Limited
5167 South Park Avenue
Hamburg, New York, 14075**

ATL Report No. BD298E-02-07-25

January 22, 2026

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SUBSURFACE INVESTIGATION AND GEOTECHNICAL EVALUATION

PROPOSED BASEBALL FIELD IMPROVEMENTS PRATTSBURGH CENTRAL SCHOOL DISTRICT 7274 PRATTSBURGH PULTENEY ROAD (COUNTY ROUTE 74) TOWN OF PRATTSBURGH, STEUBEN COUNTY, NEW YORK

PRATTSBURGH CENTRAL SCHOOL DISTRICT

1.0 INTRODUCTION

At the request of Sarah Morrissey, representing Hunt Engineers, Architects, Land Surveyors, and Landscape Architect, DPC (Hunt), on behalf of Prattsburgh Central School District (CSD), and in accordance with our proposal (ATL File No. BD998-1670-06-25, dated June 13, 2025), Atlantic Testing Laboratories, Limited (ATL) performed a subsurface investigation and geotechnical evaluation for the referenced project. The purpose of the investigation was to ascertain the general subsurface soil and groundwater conditions at the site; to evaluate the engineering significance of these findings; and to provide recommendations related to the design and construction of the proposed improvements.

2.0 SITE AND PROJECT DESCRIPTION

As shown on the **Site Location Plan**, included in **Appendix A**, the existing baseball / softball field is located along the north side of Prattsburgh Pulteney Road, about 1,000 feet east of its intersection with Flynn Road, (street address 7274 County Route 74), within the Town of Prattsburgh, Steuben County, New York. The existing complex consists of a relatively level, natural grass, baseball / softball field, and a soccer field. Remaining areas of the developed portion of the site include agriculture plots, and a gravel / cinder driveway and parking area. The eastern portion of the site is wooded. Additional details regarding the existing site conditions are shown on the **Boring Location Plan**, also included in **Appendix A**.

The proposed improvements to the site are expected to include:

- A concessions building that will be centrally located within the site;
- A natural turf baseball field within the eastern portion of the site;
- Asphalt pavement parking lots along Prattsburgh Pulteney Road, and within the west end of the site;
- Below grade stormwater management areas; and
- A subsurface sewage disposal system.

The concessions building is planned as a single story masonry structure with an approximate 1,000 square feet footprint. The new baseball field will include backstop tension netting supported on drilled pier foundations. Two entrance / exit drives will extend from the public roadway to the parking lot. Traffic is expected to consist predominantly of passenger vehicles. The stormwater management areas will include subsurface infrastructure to allow for the infiltration of stormwater into the soils. A conventional septic system will be located near the existing baseball / softball field. Additional details about the planned upgrades are shown on the **Proposed Improvements Plan**, included in **Appendix A**.

3.0 SUBSURFACE INVESTIGATION & SAMPLING METHODOLOGY

Hunt provided ATL with a site plan, showing the proposed location of the soil borings and infiltration test pipes. This included:

- Borings SB-1 and SB-2 for the proposed concessions building, drilled to depths of 15 feet and 45 feet, respectively;
- Borings SB-3 and SB-4 for the backstop tension netting, drilled to depths of 25 feet each;
- Borings SB-6, SB-7, and SB-8 for the parking areas, drilled to depths of 10 feet each;
- Borings IT-1 through IT-7 for the stormwater management areas drilled to depths of 10 feet each; and
- Borings SD-1 and SD-2, with temporary observation wells, for the proposed subsurface sewage disposal system, drilled to depths of 10 feet each.

Boring SB-5 was completed for a separate project and is not referenced in this report. ATL used Google Earth to establish the GPS northing and easting coordinates for each boring location. A Trimble Model R8 GPS / GNSS Receiver was then used to locate the boring locations in the field and to determine the approximate ground surface elevation at each location. The recorded coordinates, along with the approximate ground surface elevations at the marked locations, are summarized in **Table 1**, within **Appendix B**.

The soil borings were completed by ATL between October 22nd and 29th, 2025, using a Central Mine Equipment (CME) model 55LC, rubber track mounted drill rig. The borings were advanced into the overburden soils using hollow stem auger and split spoon soil sampling techniques. Soil sampling and standard penetration testing (SPT) was performed continuously throughout the full depth of the 10 feet deep borings, and to a depth of 12 feet within the deeper borings. Beyond a depth of 12 feet within the deeper borings, the soil sampling and SPT was performed in intervals of five feet or less for the remaining depth of the borings. The split spoon sampling and SPT was completed utilizing a 2-inch outside diameter split spoon sampler and automatic drop hammer, in accordance with ASTM D 1586.

Following the completion of borings SD-1 and SD-2, temporary groundwater observation wells were installed within the bore holes. Each observation well consisted of 1-inch diameter PVC machine slot well screen and riser pipe, with filter sand installed around the well screen. The sand pack extended from the bottom of the borings to a depth of 4-feet below the surface. A bentonite chip seal was installed above the sand packs, and soil cuttings were used to backfill the remainder of the bore holes to the surface.

Infiltration test holes were drilled near borings IT-1 through IT-7 to a depth of about 6 feet. The drillers then installed 4-inch diameter PVC casing pipes. Additional details regarding the infiltration testing procedures and results are provided in Section 6.0 of this report.

The boreholes and infiltration test holes were backfilled with on-site soils upon completion of the subsurface investigation, and the surface was patched as appropriate. It is important that the backfilled borings be monitored for settlement or subsidence. This will be the responsibility of Hunt, or their client. ATL assumes no liability for loss or damage resulting from borehole settlement.

The collected soil samples were visually classified in ATL's laboratory by a soils technician, using the Burmister Soil Classification System. The split spoon sampler does not recover particles larger than 1 $\frac{3}{8}$ -inch in nominal dimension, therefore, the soil classifications may not be representative of the entire soil matrix. The visual classifications and the standard penetration test results are presented on the **Subsurface Investigation Logs**, included in **Appendix C**.

4.0 LABORATORY ANALYSES

Several samples of the existing fill soils and indigenous soils were tested in ATL's geotechnical laboratory for the following physical properties.

- Laboratory Determination of Moisture Content of Soils (ASTM D 2216).
- Particle Size Distribution, without Hydrometer Analyses (ASTM D 422).

The **Laboratory Test Results** are included in **Appendix D**, and are summarized in the following sections of this report.

5.0 SUBSURFACE CONDITIONS

The following description of the subsurface conditions is based on the conditions encountered during the subsurface investigation performed between October 22nd and 29th, 2025. Actual conditions may vary across the site in both the horizontal and vertical dimensions. Bedrock was not encountered at the depths and locations explored. More detailed subsurface descriptions are provided on the subsurface investigation logs in Appendix C.

5.1 Surface Conditions and Existing Fill Soils

Boring SB-6 was completed within the existing parking area, and encountered a mixture of gravel and asphalt millings at the surface, that was about 5 inches thick. Topsoil was encountered at the surface of the remaining boring locations. The topsoil thickness ranged from about 4 to 6 inches, as measured within split spoon samples and/or boring holes. The topsoil thickness noted by the driller were made at widely spaced locations. In addition, properly measuring these thicknesses within the split spoon samples and boring holes is difficult and can vary depending on the driller's interpretation of topsoil, and therefore such measurements can be subject to interpretive disagreements and quantity disputes. The topsoil thickness can also be greater where trees and brush are present.

We recommend the Contractor, and/or others, make their own detailed observations and measurements, prior to bidding and construction, to determine the quantities, costs, and efforts that will be required for removal of the topsoil and organic soils, and their associated replacement with appropriate suitable fill materials.

Beneath the surface conditions, apparent man-placed fill soils were encountered within all of the soil borings, which extended to depths ranging from about 2 feet to 6 feet. It should be expected that the fill thickness will vary between and away from the boring locations, and will be dependent on the original site topography prior to site development and filling. The fill soils will also extend to the bottom of the excavations made for existing structure foundations and underground utilities.

The fill soils consisted mostly of a reworked mixture of sand and gravel, with little to some amounts of silt. Trace amounts of organics were also observed within the shallower samples of fill soils. Trace amounts of brick fragments were observed within the 4 to 6 feet deep sample from boring SB-6. Three samples of the existing fill soils were tested for moisture content and particle size distribution. The Laboratory Test Results are included in Appendix D, are summarized in the following table, and generally confirm the visual soil classification.

Geotechnical Laboratory Testing Results – Existing Fill Soils					
Boring	Sample Depth (ft. bgs)	Moisture Content (%)	Particle Size Analysis		
			Gravel (%)	Sand (%)	Fines (Silt and Clay) (%)
SB-7	0.5 to 4	8.2	41	36	23
SB-8	0.5 to 2	9.1	46	39	15
SD-1	2 to 4	4.2	53	36	11

Note: ft. bgs = feet below ground surface.

The Standard Penetration Test (SPT) “N” values obtained within existing fill soils ranged from 8 to 43, with an average of about 20, indicating the fill soils generally have a “firm” relative density.

5.2 Indigenous Soils

Beneath the approximate 2 to 6 feet of fill soils, the indigenous soils consisted mostly of a gravel and sand mixture with trace to some amounts of intermixed silt. One exception includes the soils from about 33 feet to 50 feet, within deeper boring SB-2, which consisted of a clayey silt with little to some amounts of intermixed sand and gravel. Several samples of the indigenous soils were tested for moisture content and particle size distribution. The Laboratory Test Results are included in Appendix D, are summarized in the following table, and generally confirm the visual soil classification.

Geotechnical Laboratory Testing Results – Indigenous Soils					
Boring	Sample Depth (ft. bgs)	Moisture Content (%)	Particle Size Analysis		
			Gravel (%)	Sand (%)	Fines (Silt and Clay) (%)
SB-2	4 to 6	5.9	44	36	20
SB-3	4 to 6	5.1	62	27	11
SB-3	20 to 22	13.1	44	31	25
SB-4	8 to 10	11.5	60	27	13
SB-4	15 to 17	18.2	73	20	7
IT-1	4 to 8	3.7	77	16	7
IT-3	6 to 8	6.6	49	36	15
IT-6	6 to 8	6.1	56	31	13
IT-7	6 to 8	6.1	65	25	10

Note: ft. bgs = feet below ground surface.

At depths shallower than about 10 to 12 feet, the Standard Penetration Test (SPT) “N” values obtained within the predominately gravel and sand indigenous soils mostly ranged from 15 to 30, indicating the shallower indigenous soils generally have a “firm” relative density. Within the deeper borings SB-1 through SB-4, the SPT “N” values from about 12 feet to 33 feet, ranged from 5 to 18, with an average of about 11, indicating the deeper gravel and sand soils have a “loose” to “firm” relative density.

The SPT “N” values obtained within the last stratum of soils within deeper boring SB-2, from about 33 feet to 50 feet, ranged from 14 to 67, indicating the clayey silt soils have a “stiff” to “hard” consistency.

5.3 Groundwater

Based on the following information, a general groundwater condition appears to exist near a depth of about 6 feet. It should be expected that groundwater conditions will vary with location and with changes in soil conditions, precipitation and seasonal conditions.

At the completion of drilling and soil sampling, water level measurements were made within the boreholes, and are reported on the boring logs. Water was present within seven of the borings, typically at depths ranging from 8 feet to 11 feet below the surface. No water was present following the completion of the remaining nine borings. Based on the subsequent water level readings obtained at the temporary observation wells, and the soil moisture descriptions, it appears the groundwater did not have adequate time to accumulate and stabilize in the bore holes, during the relatively short time period that had elapsed from the completion of overburden drilling operations and the time of the measurement.

The collected soil samples beneath a depth of 8 feet were often described as “wet” or “saturated”, which are indications of a groundwater condition. As noted, temporary groundwater observation wells were installed within completed borings SD-1 and SD-2, with the well screen and sand pack extending from a depth of 4 feet to the bottom of the borings at 10 feet. ATL measured the water level on November 21, 2025, and determined the depth to water was about 6.1 feet below the surface at both locations. Following the water level measurements, ATL removed water from the wells with a bailer. After removing about 2 to 3 gallons of water from each well, the water levels remained unchanged.

6.0 INFILTRATION TESTING AND RESULTS

The following section describes the infiltration testing procedures and results. The infiltration testing field data are presented on the **Infiltration Test Data Summaries** included in **Appendix E**, and are summarized below. Infiltration test holes were drilled near borings IT-1 through IT-7 to a depth of about 6-feet. Four-inch diameter, PVC casing pipes, were then placed in the test holes and the annulus space between the casing pipes and the test holes were backfilled with soil cuttings. Water was added into the test pipes on October 27th, 2025 to pre-soak the soils. ATL returned to the site on October 28th, 2025 to complete the infiltration testing, in general conformance with the infiltration test procedure presented in the *NYSDEC Publication “Stormwater Management Design Manual – July 31, 2024” – Appendix D: Infiltration Testing Requirements*.

On day of the infiltration testing, no pre-soak water remained within any of the test pipes. ATL added additional water into the test pipes, raising the total water levels to a height of 24 inches above the bottom of the test pipes. The water level drop was then measured in 60 minute intervals, for four consecutive test runs. At several locations, the 24 inches of water infiltrated within less than 60 minutes. Following each test run, the water level was adjusted to the same level as the first run. The results are summarized in the following table, with more detailed information provided on the Infiltration Test Data Summaries included in Appendix E.

Infiltration Testing Results Summary			
Infiltration Test Pipe	Ground Surface Elev. (feet)	Depth / Elevation at Bottom of Infiltration Test Pipes (feet)	Approximate Infiltration Rate (in./hour)
IT-1	1323.2	6.1 / 1317.1	380
IT-2	1323.8	5.8 / 1318.0	80
IT-3	1323.3	5.7 / 1317.6	14
IT-4	1323.3	5.9 / 1317.4	22
IT-5	1323.1	5.8 / 1317.3	57
IT-6	1323.2	5.7 / 1317.5	22
IT-7	1323.7	5.7 / 1318.0	6

Note: in. /hour = inches per hour.

Soil samples from borings IT-1, IT-3, IT-6, and IT-7, collected from near the bottom of the test pipes, were tested for particle size distribution. The soils from boring IT-1 contained about 77 percent gravel size particles, and only 7 percent fines (i.e. silt and clay size particles), which appears to have resulted in the relatively fast infiltration rate. The soils tested from the remaining borings were somewhat similar.

7.0 GEOTECHNICAL ENGINEERING DISCUSSION & RECOMMENDATIONS

7.1 General

The geotechnical engineering discussion and recommendations are based on the information provided by Hunt, and the subsurface conditions outlined in this report. The following section provides generalized recommendations for the proposed improvements, with more detailed recommendations presented in the subsequent sections of this report.

1. Conventional spread foundations can be used to support the proposed concessions building, as currently planned. The geotechnical issue that should be addressed includes the removal of all existing fill soils and any looser indigenous soils from beneath the proposed foundations.
2. The concessions building floor can be constructed as a slab-on-grade over the existing soils, following: removal of the topsoil and any near surface soft / very loose soils, or other unsuitable soils; preparation of the exposed subgrades (i.e. drying, compaction and proof-rolling); installation of subgrade fill (i.e. Suitable Granular Fill), to raise site grades; and placement of a geotextile and subbase layer.
3. Shallow excavations above a depth of 6 feet, made to construct the spread foundations, are not expected to encounter a general groundwater condition. However, some isolated zones of perched or trapped groundwater could be encountered, depending on the time of year construction is completed. When encountered, appropriate dewatering measures should be implemented to allow for construction to proceed in the dry.
4. The subsurface conditions encountered at boring locations SB-3 and SB-4 are considered to be suitable for supporting the proposed backstop tension netting poles using cast-in-place concrete drilled pier foundations (i.e. caissons). It will be difficult to maintain stable excavation sidewalls and bottom stability when advancing the drilled pier excavations within the non-cohesive gravel and sand soils, especially below the groundwater level. Accordingly, the use of temporary casing and / or drilling slurry will likely be necessary to maintain the excavation sidewall and bottom stability.
5. Based on the subsurface conditions encountered, seismic design parameters should be evaluated for three seismic site classes, including “DE”, “D” and “CD”, in accordance with the criteria in the 2025 New York State Building Code.

7.2 Spread Foundations

Spread foundations can be used to support the proposed concessions building, as currently planned. The spread foundations should be designed to bear on suitable, relatively undisturbed, indigenous soils, or they can bear on Engineered Fill (i.e. compacted Structural Fill or Flowable Backfill) placed over suitable indigenous soils. Suitable indigenous soil bearing grades should consist of the relatively “firm” silty gravel and sand soils. The bearing grades must be free of existing fill soils, organics, loose, or otherwise deleterious material. The suitable bearing grade depths for spread foundations were encountered about 5 feet below the existing site grades at boring SB-1 and about 4 feet below the existing site grades at boring SB-2.

Subsurface conditions could vary between and away from the exploration locations, and therefore could require adjustments in the suitable subgrade elevation, based on actual conditions encountered at the time of construction. Accordingly, full time inspection of the foundation bearing subgrades, by qualified geotechnical personnel, is recommended as the excavations are made at the time of construction.

Structural Fill, if used as an Engineered Fill layer beneath the spread foundations, should extend out horizontally a distance equal to at least 0.5 times the thickness of the Structural Fill layer beneath the foundations. Excavations, therefore, will need to be planned and sized accordingly. Recommendations for Structural Fill material and its placement and compaction are provided in Section 9.1 of this report.

Flowable backfill material, if used as the Engineered Fill layer, should be a non-swelling type of material and should have a minimum 28-day compressive strength (f'_c) of 250 pounds per square inch (psi). The flowable backfill should extend at least 12 inches horizontally beyond the foundation limits for its entire depth.

Continuous wall footings should be at least 2.0 feet in width and column/individual footings should be at least 3.0 feet in width. All foundations should be embedded a minimum of 4.0 feet below surrounding site grades or floor slab for frost protection. All foundations, however, should bear on suitable indigenous soils or Engineered Fill installed over suitable indigenous soils, in accordance with the recommendations above.

Spread foundations, constructed in accordance with the above recommendations, can be sized based on a maximum net allowable bearing pressure of 3,000 pounds per square foot. It is estimated that 2 feet wide wall footings and 3 feet wide isolated square footings, sized and properly constructed in accordance with our recommendations will undergo a total settlement of less than $\frac{1}{2}$ inch.

7.3 Slab-on-Grade Floor

The slab-on-grade floor can be constructed following: removal of the topsoil and any organic soils; removal of the near surface looser soils, or other unsuitable soils; preparation of the exposed subgrades (i.e. drying, compaction and proof-rolling); installation of subgrade fill (i.e. Suitable Granular Fill), to raise site grades; and placement of a geotextile and subbase layer.

Following the completion of the subgrade preparation, Suitable Granular Fill or additional Structural Fill / Subbase, as described in Section 9.0, should be used to raise site grades. A minimum 6 inch thick Subbase layer is recommended beneath the floor slab. The Subbase should be separated from the existing fill soil subgrades with a suitable stabilization/separation geotextile, such as Mirafi 500X.

The slab-on-grade can be designed in accordance with procedures recommended by the Portland Cement Association or the American Concrete Institute, using a modulus of subgrade reaction of 150 pounds per cubic inch at the top of the subbase layer.

The floor slab will be constructed above the finished site grades. Therefore, the use of a moisture barrier does not appear warranted, unless otherwise recommended by the finished flooring manufacturer, or as required to meet interior air moisture requirements. It is recommended that the slab-on-grade floor be constructed such that it floats on the Subbase and is not structurally connected to wall or column footings, to limit potential differential settlement effects, unless the slab column interface is designed with sufficient reinforcement to bridge potential differential settlement effects at these interfaces.

7.4 Drilled Pier Foundations

7.4.1 End Bearing Capacity

The allowable end bearing capacities for the drilled pier foundations will vary based on the actual embedment depth and the soil conditions at the embedment depth. Several allowable end bearing capacities are provided below, based on a minimum drilled pier diameter of 30 inches.

- A bearing grade at depth of 6 feet to 10 feet, will result in an allowable end bearing capacity of 4,000 pounds per square foot.
- A bearing grade depth of 12 feet to 18 feet, will result in an allowable end bearing capacity of 4,500 pounds per square foot.
- A bearing grade deeper than 18 feet, will result in an allowable end bearing capacity of 5,000 pounds per square foot.

Settlement beneath the drilled piers will depend on the actual embedment depth and the underlying soil conditions, the diameter drilled pier, and the actual compressive load. We can evaluate the estimated settlement if this information is provided. However, it is our understanding that the lateral resistance of the soils often controls the required embedment depths and diameters.

7.4.2 Soil Properties for Foundation Design

The soil properties presented in Table 2, within Appendix B, can be used in designing the drilled pier foundations. The embedment depths should be determined by the backstop tension netting pole designer, to provide adequate axial, lateral, and uplift resistance. The soil information and properties provided include the following:

- soil layer depths;
- generalized soil descriptions;
- SPT “N” value used to develop the soil parameters;
- total unit weight;
- relative density;
- angle of internal friction (ϕ);
- passive lateral earth pressure coefficient;

The soil properties have been developed based on the subsurface conditions encountered in the borings, as well as in reference to recognized published soil properties and empirical relationships.

7.4.3 Groundwater Conditions for Design

Based on the subsurface conditions encountered, a general groundwater condition appeared to be present near a depth of about 6 feet. However, we recommend the groundwater conditions be assumed to exist at the surface to account for seasonal fluctuations. Therefore, submerged or saturated soil unit weights should be used in the foundation analyses for all soils. The submerged unit weight should be calculated by subtracting 62.4 pounds per cubic foot from the total unit weights provided in Table 2, within Appendix B. This is the same as effective or buoyant unit weight.

7.4.4 Factors of Safety for Design

The allowable end bearing capacities, as presented above, are based on a factor of safety of 3.0. The drilled pier foundations should be designed for a factor of safety of 2.0 against movement, under sustained lateral and overturning loading conditions. The factor of safety against lateral and overturning movement can be reduced to 1.5 for transient loading conditions. Soil properties presented in Table 2 are ultimate values and do not include any factors of safety.

7.5 Seismic Design Considerations

In accordance with ASCE 7-22 (referenced by the 2025 NYS building code), when the average shear wave velocity of the subsurface soil to a depth of 100 feet is not measured to determine the site classification, it can be estimated using correlations with suitable geotechnical parameters, such as Standard Penetration Testing (SPT) "N" values. However, ASCE 7-22 requires that the site classification using correlated shear wave velocities be evaluated using: the estimated shear wave velocity (v_s), v_s divided by 1.3, and v_s multiplied by 1.3.

Based on the SPT "N" values to a depth of 45 feet, we developed an estimated shear wave velocity for the site of 800 feet per second (ft/s). When considering the 30% uncertainty, the shear wave velocity for the site could range from the following values.

- 615 ft/s (Seismic Site Class "DE")
- 800 ft/s (Seismic Site Class "D")
- 1040 ft/s (Seismic Site class "CD")

Based on the above range, the designer should evaluate the project site for the three Seismic Site Classes and use the worst-case ground motion for a given period.

7.6 Asphalt Pavement

7.6.1 Pavement Design

It is our understanding that the site will be accessed predominately by passenger vehicles with occasional school busses. We have established 100,000 equivalent 18 kip equivalent axle loads (EALs) as being appropriate for an approximate 20 to 25 year design life for the facility. We anticipate the subgrades will have an effective Roadbed Soil Resilient Modulus (M_r) of about 5,000 pounds per square inch, following the completion of the subgrade preparation outlined in Section 8.3. Based on this data and the expected subgrade soil conditions following preparation, we developed the following recommended pavement section.

Recommend Asphalt Pavement Section

- 1.5 inches - Top Course
- 2.5 inches - Binder Course
- 10 inches - Subbase Course*
- Stabilization / Separation Geotextile
- Prepared Subgrade

** The Subbase Course thickness at the entrances / exits should be increased to 15 inches. In addition, it may be necessary to increase the subbase thickness in some areas to improve subgrade conditions and to promote drainage to underdrains, as discussed below.*

Materials for the above pavement structure components should consist of the following:

- A. Asphalt Concrete Top Course - NYSDOT Standard Specifications, Hot Mix Asphalt, 9.5 F2 Top Course.
- B. Asphalt Concrete Binder Course with a Thickness less than 3 inches - NYSDOT Standard Specifications, Hot Mix Asphalt, 19 F9 Binder Course.
- C. Asphalt Concrete Binder Course with a Thickness greater than 3 inches - NYSDOT Standard Specifications, Hot Mix Asphalt, 25 F9 Binder Course.
- D. Subbase Course – Recommendations included in Section 9.2 of the Report.
- E. Geotextile - Woven polypropylene stabilization/separation geotextile (i.e., Mirafi 600X or approved suitable equivalent).

7.6.2 Pavement Drainage

The installation of underdrains or edge drains are recommended to drain the pavement subbase course within any low points, to limit the potential for frost action and improve pavement structure performance and design life. Alternatively, the pavement subbase course can also be allowed to daylight/drain to an adjacent perimeter drainage swale. This could be accomplished by raising the pavement grade. Drainage of the pavement subgrades can be achieved by grading the subgrade to a slope of at least 2 percent to allow drainage to the underdrains or drainage swale.

Underdrains, if used, should include a non-woven geotextile (i.e. Mirafi 160N or suitable equivalent), selected considering drainage and filtration, installed around drainage stone surrounding a slotted or perforated drain pipe. The drainage stone should be sized in accordance with the pipe slotting or perforations. A crushed aggregate (½-inch washed gravel or stone) is generally acceptable for slotted underdrain pipe. The underdrain pipes should be set in the bottom of the subbase layer, or preferably below the top of the soil subgrade elevation. The drainage stone and surrounding geotextile should extend above the underdrain pipe and into the subbase layer. Underdrain pipes should be connected to the site storm water drainage system.

7.6.3 Pavement Construction

Placement of the pavement Subbase course can proceed, following proper subgrade preparation and subgrade filling as described in Section 8.3 of this report. Installation of adjacent geotextile panels should have minimum overlap of 12 to 18 inches. The Subbase should be placed and compacted in accordance with the recommendations in the Material Recommendations section of this report.

Construction of the Asphalt Concrete Pavement should be performed in accordance with NYSDOT Standard Specification Section 400. The Binder and Top Course compaction / evaluation should comply with NYSDOT Standard Specifications – 80 Series Compaction procedures, as a minimum, or as otherwise required by the jurisdictional agency. In addition, placement of asphalt concrete courses should not be permitted on wet or snow covered surfaces or when the subgrade surface is less than 40° F.

8.0 SITE PREPARATION AND CONSTRUCTION

8.1 Construction Dewatering

Surface water should be diverted away from open excavations and prevented from accumulating on exposed subgrades. It is anticipated that diversion berms, drainage ditches, and proper site grading should generally be sufficient to control surface water conditions.

A general groundwater condition appears to be present near a depth of about 6 feet, and is therefore not expected to be encountered within the relatively shallow spread foundation excavations. However, there is a potential to encounter some isolated zones of perched groundwater. If encountered, construction dewatering methods should be implemented, which will allow for construction to proceed in the dry.

The amount of groundwater that could be encountered will depend on the excavation depth, the surrounding soil conditions, and the actual groundwater conditions at the time of construction. It is anticipated that conventional sump and pump methods of dewatering will be required, as a minimum, should perched groundwater conditions be encountered.

Groundwater dewatering plans should include implementation of measures to control erosion, sedimentation, and the migration of soil fines. All dewatering activities should comply with New York State Department of Environmental Conservation (NYSDEC) storm water discharge requirements and/or applicable federal and local regulations for construction.

8.2 Excavation and Spread Foundation Construction

Excavations for construction of the spread foundations should be performed using a method which reduces disturbance to the subgrade soils. The proposed foundation bearing grades should be observed and evaluated by qualified geotechnical personnel, prior to placement of Engineered Fill and/or the foundation. Any placement and compaction of Structural Fill beneath foundations should also be observed / tested.

Exposed subgrades should be protected from precipitation and surface water, and should be adequately dewatered. The indigenous soils will be sensitive to disturbance and strength degradation when in the presence of excess moisture. Water should not be allowed to accumulate on the soil subgrades and the subgrades should not be allowed to freeze, either prior to or after construction of foundations. If subgrades are not protected and degrade, they must be undercut and replaced accordingly.

Where foundations are constructed directly on the indigenous soil bearing grades, and where construction of the foundations proceeds during seasonal wet periods and/or the foundations will not be constructed on the same day of the excavation, it may be desirable to place a 2 to 3 inch thick lean concrete mud mat in the excavation bottom to help protect the exposed subgrades and provide a suitable working surface for the foundation construction.

After completion of the foundation construction, the excavations should be backfilled as soon as possible and prior to construction of the superstructure. It is recommended that the foundation excavations, within slab-on-grade and pavement areas, be backfilled with a Suitable Granular Fill or Structural Fill, as described in Section 9.0 of this report.

8.3 Subgrade Preparation for Floor Slab and Pavement Construction

The site preparation work should be performed during seasonally dry periods to minimize potential degradation of the subgrade soils and undercuts which may be required to establish a stable base for construction. It should be understood that the existing soils encountered at the site are sensitive and can degrade and lose strength when they are wet and disturbed by construction equipment traffic. Accordingly, efforts should be made to maintain the subgrades in a dry and stable condition at all times, and minimize construction traffic directly over these soils.

All gravel / asphalt millings, vegetation, topsoil, organic soils, looser / softer soils, or other unsuitable soils within the proposed building and pavement areas, should be removed. If unsuitable soils are encountered at the subgrade elevation, some additional undercutting could become necessary to establish a firm and stable subgrade condition for installation of subgrade fill.

Following removal of the surface materials and excavation to the proposed subgrades, the exposed soil subgrades should be allowed to dry as necessary, and then be thoroughly compacted and densified. The subgrade compaction should be performed prior to any required fill placement, using a vibratory smooth drum roller weighing at least 10 tons. The roller should complete at least four passes over the exposed subgrades for the compaction/densification operation. If the exposed subgrade soils have a moisture content above their optimum moisture content, then operation of the roller in vibratory mode should be avoided.

Following compaction, the subgrades should be proof-rolled to determine if any soft or unstable conditions exist at the exposed subgrade. The proof-rolling should be performed just prior to the overlying fill placement using a tandem axle truck with a minimum gross weight of 40,000 pounds.

The subgrade proof-rolling procedure should be done under the guidance of, and observed by, qualified geotechnical personnel. Any areas, which appear wet, loose, soft, unstable, or otherwise contain unsuitable materials, should be undercut. Over excavation, which may be required as the result of the subgrade inspection and/or proof-rolling, should be performed based on evaluation of the conditions and guidance provided by qualified geotechnical personnel. The resulting over-excavations should be backfilled with compacted Structural Fill material. The placement of an initial lift of oversized stone fill material (i.e. "surge stone", "shot rock", etc.) encased in stabilization geotextile top and bottom, may be necessary in some cases to help stabilize the subgrades. All fill placement and compaction should be closely monitored and tested on a "full-time" basis by qualified geotechnical personnel.

New subgrade fill should be placed to a stable condition and should not "pump" or show signs of movement or significant deflection (i.e. unstable conditions) as it is being constructed. During construction the Contractor should take precautions to limit construction traffic over the subgrades for slab-on-grade construction. The fill subgrades should also be properly graded, drained, and protected from excessive moisture and frost. Placement of fill over wet, soft, snow covered or frozen subgrades is not acceptable. It is recommended that utility trenches located within slab-on-grade areas be backfilled with compacted Structural Fill.

We note that the recommended Subbase thicknesses for the slab-on-grade floor and asphalt pavement areas are not designed for carrying construction vehicle loads. Therefore, it may be desirable for the Contractor to temporarily increase the Subbase thickness within these areas to provide a suitable working surface to stage the construction, carry construction vehicle loads and protect the underlying subgrades. This will be particularly important if construction proceeds during seasonally wet periods. Any subgrades, including existing soil subgrades, new fill subgrades, or Subbase layers which become damaged, rutted, or unstable, should be undercut and repaired as necessary prior to placement of overlying fill courses, or construction of the floor slabs and asphalt pavement.

8.4 Drilled Pier Excavation and Construction

The excavations made for the drilled piers are expected to encounter groundwater conditions about 6 feet below the surface. Some shallower zones of perched groundwater should also be expected. Dewatering of the excavations does not appear to be practical, due to the relatively granular nature of the soils. In addition, dewatering could result in bottom heave of the looser granular soils. Therefore, procedures for excavation below groundwater should be considered. In this case, the excavations should be stabilized with temporary casing and / or a proper drilling slurry. The contractor, however, should be responsible for the means and methods of advancing the excavations to the design depths.

Initially, the excavation should extend to the proposed bearing grade. Disturbed soils and all drill cuttings should be removed from the bearing surface. The final bearing surface should be level or near level. Plumbness of the excavations should be maintained within 1% of the total length.

The concrete must be placed in a manner that displaces the slurry from the hole, such as utilizing tremie methods. The tremie pipe must be maintained below the water level and at least five feet below the top of the concrete during concrete placement. The fresh concrete must be above the bottom of the casing at all times. Casing removal during concrete placement should proceed in a manner that prevents the concrete from mixing with drilling mud, cave-in of the excavation and/or the formation of voids. In all cases, drilled pier construction should be monitored by qualified geotechnical personnel to ensure adequate soil conditions are encountered, and proper installation techniques are followed.

8.5 Testing and Inspection

The final site grading and foundation plans and project specifications should be reviewed by ATL, as the Geotechnical Engineer of Record, to verify that there has not been a misinterpretation of this report and/or ATL's understanding of the project.

All foundation installations should be continuously monitored by a Geotechnical Engineer to verify the stability and uniformity of the subgrade soil, to identify the presence of deleterious fill, and to ensure that adequate soil bearing capacity is obtained.

We recommend that ATL, as the Geotechnical Engineer of Record, be retained to perform Special Inspections in accordance with the Building Code of New York State, during foundation installations. An ATL geotechnical representative familiar with the findings and recommendations of this report will be able to assess the subsurface conditions encountered during construction, provide necessary remedial recommendations, and verify that adequate bearing capacities and proper foundation installation requirements are achieved.

All foundation construction and backfilling should be monitored and tested by an Independent Testing Agency, conforming to ASTM E-329, “Standard Specification for Agencies Engaged in Construction Inspection, Testing, or Special Inspection.” ATL conforms to ASTM E-329 and can be retained to perform required construction phase monitoring and testing services, including applicable Special Inspections and Structural Tests in accordance with the Building Code of New York State.

9.0 MATERIAL RECOMMENDATIONS

9.1 Structural Fill

Structural Fill, which is placed beneath foundations or used as backfill, should consist of a crushed ledge-rock or a crushed gravel product, which is free of clay, organics and friable or deleterious particles. The material should comply with NYSDOT Standard Specifications, Item No. 304.12 - Type 2 Subbase or Item No. 304.14 - Type 4 Subbase, with the condition that if a gravel and sand product is used (vs. a crusher run stone), the gravel should be a crushed gravel material, with at least 75% of the particles greater than ¼ inch, having a minimum of one crushed face. The Structural Fill should have the following gradation requirements.

Item 304.12 – Type 2 Subbase (crushed ledge-rock)		Item 304.14 – Type 4 Subbase (crushed gravel)	
Sieve Size Distribution	Percent Finer by Weight	Sieve Size Distribution	Percent Finer by Weight
2 inch	100	2 inch	100
¼ inch	25 to 60	¼ inch	30 to 65
no. 40	5 to 40	no. 40	5 to 40
no. 200	0 to 10	no. 200	0 to 10

The Structural Fill should be compacted to a minimum of 95 percent of the maximum dry density as measured by the modified Proctor test (ASTM D1557). Placement of fill should not exceed a maximum loose lift thickness of 8 to 10 inches. The loose lift thickness should be reduced in conjunction with the compaction equipment used so that the required density is attained. The Structural Fill should have a moisture content within two percent of the optimum moisture content at the time of compaction.

9.2 Subbase

The Subbase course placed as the aggregate beneath the slab-on-grade floor and pavement construction, should conform to the same material requirements as Structural Fill as stated above, and should be installed to the same requirements.

9.3 Suitable Granular Fill

Suitable, well graded from coarse to fine, soil material classified as GW, GP, GM, SW, SP and SM soils using the Unified Soil Classification System (ASTM D-2487) and having no more than 85 percent by weight material passing the No. 4 sieve, no more than 20 percent by weight material passing the No. 200 sieve and which is generally free of particles greater than 4-inches, will be acceptable as Suitable Granular Fill. It should also be free of topsoil, asphalt, concrete rubble, wood, debris, clay, and other deleterious materials.

Suitable Granular Fill can be used as foundation backfill and for raising site grades. Material meeting the requirements of New York State Department of Transportation, Standard Specifications, Item 203.07 – Select Granular Fill is acceptable for use as Suitable Granular Fill. The Suitable Granular Fill should be placed and compacted in accordance with the requirements stated above for the Structural Fill.

10.0 LIMITATIONS

The subsurface investigation logs, and this report in its entirety should be provided to the designers and contractors for information and interpretation. The subsurface investigation logs may not be representative of the entire site subsurface condition, but only what was encountered at the individual boring location at the time of the investigation. The subsurface soil and groundwater conditions encountered at the time of construction may be different from those described on the subsurface investigation logs.

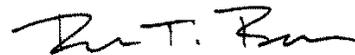
This report was prepared to present the findings of our subsurface investigation and engineering evaluation, and to outline concepts to be utilized in foundation design and construction. These concepts may require alterations to meet the specific design and economic considerations for this project.

Prepared by:



Thomas R. Seider, PE
Senior Engineer

Reviewed by:



Brian T. Barnes, PE
Director of Geotechnical Services

TRS/BTB/sb

APPENDIX A

FIGURES



Note: Figure developed from Google Earth.

Site Location Plan	Drawn by: TRS	Scale: Not to scale	Project No.: BD 298	Date: 01-09-2026
Proposed Baseball Field Improvements Prattsburgh Central School District Prattsburgh Pulteney Road Town of Prattsburgh, Steuben County, New York		ATLANTIC TESTING LABORATORIES, Limited		
		Albany, NY Elmira, NY Rochester, NY	Binghamton, NY Plattsburgh, NY Utica, NY	Buffalo, NY Poughkeepsie, NY Watertown, NY



North



- Notes: 1) Figure developed from Google Earth.
 2) Boring SB-5 completed for a separate project.

Boring Location Plan

Drawn by:
TRS

Scale:
Not to scale

Project No.:
BD 298

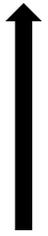
Date:
01-09-2026

**Proposed Baseball Field Improvements
 Prattsburgh Central School District
 Prattsburgh Pulteney Road
 Town of Prattsburgh, Steuben County, New York**

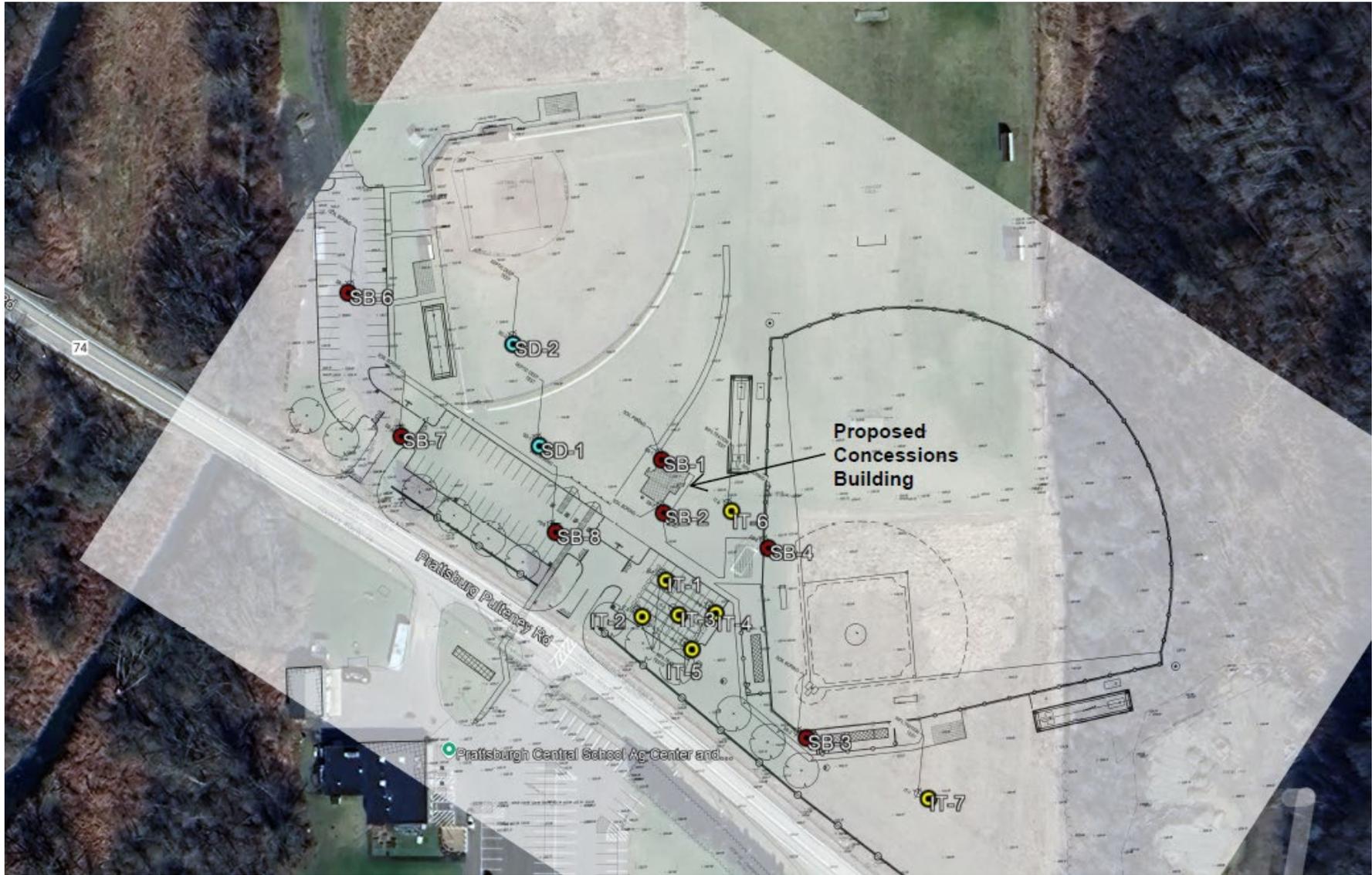


ATLANTIC TESTING LABORATORIES, Limited

Albany, NY	Binghamton, NY	Buffalo, NY	Canton, NY
Elmira, NY	Plattsburgh, NY	Poughkeepsie, NY	Syracuse, NY
Rochester, NY	Utica, NY	Watertown, NY	



North



Notes: 1) Figure developed from Google Earth and Testing Locations Plan - GE-1, prepared by Hunt.
 2) Boring SB-5 completed for a separate project.

<p>Proposed Improvements Plan</p>	<p>Drawn by: TRS</p>	<p>Scale: Not to scale</p>	<p>Project No.: BD 298</p>	<p>Date: 01-09-2026</p>
<p>Proposed Baseball Field Improvements Prattsburgh Central School District Prattsburgh Pulteney Road Town of Prattsburgh, Steuben County, New York</p>		<p>ATLANTIC TESTING LABORATORIES, Limited</p>		
<p>Albany, NY Elmira, NY Rochester, NY</p>		<p>Binghamton, NY Plattsburgh, NY Utica, NY</p>		<p>Buffalo, NY Poughkeepsie, NY Watertown, NY Canton, NY Syracuse, NY</p>

APPENDIX B

TABLES

TABLE 1
Boring Locations and Approximate Ground Surface Elevations

Proposed Baseball Field Improvements
7274 Prattsburgh Pulteney Road
Town of Prattsburgh, Steuben County, New York

Boring / Pavement Core Number	Approximate Exploration Locations GPS Coordinates		Approximate Ground Surface Elevation
	Latitude	Longitude	(feet)
Proposed Concessions Building			
SB-1	42° 30' 36.95"	77° 16' 35.51"	1324.1
SB-2	42° 30' 36.50"	77° 16' 35.50"	1323.7
Proposed Backstop Tension Netting Poles			
SB-3	42° 30' 34.55"	77° 16' 33.96"	1322.5
SB-4	42° 30' 36.17"	77° 16' 34.31"	1323.2
Proposed Asphalt Pavement Driveway and Parking Lot			
SB-6	42° 30' 38.47"	77° 16' 39.04"	1326.2
SB-7	42° 30' 37.19"	77° 16' 38.49"	1324.7
SB-8	42° 30' 36.37"	77° 16' 36.76"	1324.2
Proposed Stormwater Management Areas			
IT-1	42° 30' 35.93"	77° 16' 35.51"	1323.2
IT-2	42° 30' 35.63"	77° 16' 35.79"	1323.8
IT-3	42° 30' 35.63"	77° 16' 35.37"	1323.3
IT-4	42° 30' 35.63"	77° 16' 34.95"	1323.3
IT-5	42° 30' 35.34"	77° 16' 35.24"	1323.1
IT-6	42° 30' 36.49"	77° 16' 34.72"	1323.2
IT-7	42° 30' 33.99"	77° 16' 32.59"	1323.7
Proposed Septic System			
SD-1	42° 30' 37.12"	77° 16' 36.91"	1324.4
SD-2	42° 30' 37.98"	77° 16' 37.16"	1325.4

Note: Boring SB-5 completed for a separate project.

TABLE 2
Estimated Soil Properties for Backstop Tension Netting Pole Foundation Design

Proposed Baseball Field Improvements
7274 Prattsburgh Pulteney Road
Town of Prattsburgh, Steuben County, New York

Soil Layer Depth (ft. bgs)	Generalized Soil Description	Design SPT "N" Value	Total Unit Weight [see note 1] (pcf)	Relative Density (percent)	Angle of Internal Friction (ϕ) (degrees)	Passive Lateral Earth Pressure Coefficient
0 to 6	Sand and Gravel with Silt (Fill)	10	115	35	30	3.0
6 to 12	Gravel and Sand with Silt	15	125	45	33	3.4
12 to 18	Gravel and Sand with Silt	10	120	35	31	3.1

Notes:

1. Recommended depth to groundwater is 0 feet. Submerged soil unit weights should be used below this depth.
2. ft. bgs = feet below ground surface.
3. pcf = pounds per cubic foot.

APPENDIX C
SUBSURFACE INVESTIGATION LOGS

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Client: Prattsburgh Central School District
 Project: Subsurface Investigation
Proposed Baseball Field Improvements
Prattsburgh, New York

Report No.: BD298E-01-01-26
 Boring Location: See Boring Location Plan

Boring No.: SB-1 Sheet 1 of 1
 Coordinates _____ Sampler Hammer
 Latitude _____ Weight: 140 lbs.
 Longitude _____ Fall: 30 in.
 Hammer Type: Automatic
 Ground Elev.: 1324.1' Boring Advance By:
4 1/4" Auger

Start Date: 10/29/2025 Finish Date: 10/29/2025
 Groundwater Observations
 Date Time Depth Casing
10/29/2025 PM 9.7' 13.0'

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER	DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	Recovery (Inches)	
			From	To						
1	R I T C E	1	0.0	2.0	SS	2 5 10 12	0.5	6" TOPSOIL & ORGANIC MATERIAL Brown cmf SAND; and cf Gravel; little Silt; trace Organic Material (root hairs) (moist, non-plastic) Fill NO RECOVERY Brown cf GRAVEL; and cmf Sand; little Silt; trace Organic Material (root hairs) (moist, non-plastic) Fill Brown cf GRAVEL; and cmf Sand; little Silt (wet, non-plastic) Brown cmf SAND; and cf Gravel; little Silt (saturated, non-plastic) Brown f GRAVEL; and cmf Sand; little Silt (moist, non-plastic) Similar Soil (saturated, non-plastic)	7	
2		2	2.0	4.0		17 17 14 8			0	
3										
4		3	4.0	6.0		6 9 6 11			12	
5										
6		4	6.0	8.0		7 14 14 10	6.0		10	
7										
8	5	8.0	10.0	8 11 12 10		6				
9										
10	6	10.0	12.0	3 4 11 10		1				
11										
12										
13	7	13.0	15.0	3 4 5 5	15.0	6				
14										
15										
16										
17										
18										
19										
20										
21										
22										
23										
24										
25										

SS Split Spoon Sample
 NX Rock Core
 SH Undisturbed Sample (Shelby Tube)
 Estimated Groundwater

Drillers: Jason Porter; Trenton Coyne
 Inspector: _____

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Client: Prattsburgh Central School District
 Project: Subsurface Investigation
Proposed Baseball Field Improvements
Prattsburgh, New York

Report No.: BD298E-01-01-26
 Boring Location: See Boring Location Plan

Boring No.: SB-2 Sheet 1 of 2
 Coordinates _____ Sampler Hammer _____
 Latitude _____ Weight: 140 lbs.
 Longitude _____ Fall: 30 in.
 Hammer Type: Automatic
 Ground Elev.: 1323.7' Boring Advance By: _____
4 1/4" Auger

Start Date: 10/23/2025 Finish Date: 10/23/2025
 Groundwater Observations
 Date Time Depth Casing
10/23/2025 PM 23.0' 43.0'

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER	DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	Recovery (Inches)
			From	To					
1	RITGC	1	0.0	2.0	SS	2 5 10 15	0.5	6" TOPSOIL & ORGANIC MATERIAL Dark Brown of GRAVEL; and cmf Sand; little Silt; trace Organic Material (root hairs) (moist, non-plastic) Fill	6
2		2	2.0	4.0		13 20 23 19			12
3							4.0	Brown of GRAVEL; and cmf Sand; little Silt (moist, non-plastic) w = 5.9%	8
4		3	4.0	6.0	SS	7 14 13 13			6
5								Similar Soil (moist, non-plastic)	6
6		4	6.0	8.0	SS	7 11 7 10			6
7								Similar Soil (saturated, non-plastic)	6
8		5	8.0	10.0	SS	3 6 7 7			6
9								Similar Soil (saturated, non-plastic)	6
10		6	10.0	12.0	SS	4 6 6 4			6
11								Similar Soil (saturated, non-plastic)	6
12									6
13								Similar Soil (saturated, non-plastic)	6
14									6
15								Similar Soil (saturated, non-plastic)	6
16		7	15.0	17.0	SS	1 5 4 3			6
17								Similar Soil (saturated, non-plastic)	6
18							18.0		6
19								Brown cmf SAND; some f Gravel; little Silt (saturated, non-plastic)	7
20		8	20.0	22.0	SS	5 4 4 5			7
21								Brown cmf SAND; some f Gravel; little Silt (saturated, non-plastic)	7
22									7
23								Brown cmf SAND; some f Gravel; little Silt (saturated, non-plastic)	7
24									7
25								Brown cmf SAND; some f Gravel; little Silt (saturated, non-plastic)	7
									7

SS Split Spoon Sample
 NX Rock Core
 SH Undisturbed Sample (Shelby Tube)
 Estimated Groundwater

Drillers: Jason Porter; Trenton Coyne
 Inspector: _____

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Boring No.: SB-2

Report No.: BD298E-01-01-26

Sheet 2 of 2

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER				DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	RECOVERY (inches)	
			From	To									
26		9	25.0	27.0	SS	2	4	8	6		Brown cmf SAND; some Silt & Clay; little f Gravel (wet, low plasticity)	24	
27													
28													
29													
30		10	30.0	32.0	SS	13	7	6	4	33.0	Brown cmf SAND; some cf Gravel; little Silt (saturated, non-plastic)	6	
31													
32													
33													
34													
35		11	35.0	37.0	SS	4	5	9	12		Grey Clayey SILT; some f Gravel; little cmf Sand (wet, slight plasticity)	8	
36													
37													
38													
39													
40		12	40.0	42.0	SS	4	11	11	15	45.0	Similar Soil (wet, slight plasticity)	10	
41													
42													
43		13	43.0	45.0	SS	11	17	50	19		Similar Soil (wet, slight plasticity)	10	
44													
45													
46													
47													
48													
49													
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52													
53													
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56													
57													
58													
59													
60													
61													
62													

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

Boring terminated at 45.0 feet.

Notes:
1. Borehole backfilled with on-site soils.

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Client: Prattsburgh Central School District
 Project: Subsurface Investigation
Proposed Baseball Field Improvements
Prattsburgh, New York

Report No.: BD298E-01-01-26
 Boring Location: See Boring Location Plan

Boring No.: SB-3 Sheet 1 of 2
 Coordinates _____ Sampler Hammer
 Latitude _____ Weight: 140 lbs.
 Longitude _____ Fall: 30 in.
 Hammer Type: Automatic
 Ground Elev.: 1322.5' Boring Advance By:
4 1/4" Auger

Start Date: 10/23/2025 Finish Date: 10/23/2025
 Groundwater Observations
 Date Time Depth Casing
10/23/2025 AM 10.6' 23.0'

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER				DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	Recovery (Inches)
			From	To								
1	RITGC	1	0.0	2.0	SS	2	3	7	8	0.5	6" TOPSOIL & ORGANIC MATERIAL	8
2		2	2.0	4.0		11	11	13	13	2.0	Brown cmf SAND; and Silt; some f Gravel; trace Organic Material (root hairs) (moist, non-plastic) Fill	10
3											Brown cf GRAVEL; and cmf Sand; little Silt (moist, non-plastic)	
4		3	4.0	6.0	SS	7	23	16	8		Brown cf GRAVEL; some cmf Sand; little Silt (moist, non-plastic) w = 5.1%	8
5											Similar Soil (wet, non-plastic)	7
6		4	6.0	8.0	SS	6	13	15	10		Similar Soil (saturated, non-plastic)	3
7											Similar Soil (saturated, non-plastic)	6
8		5	8.0	10.0	SS	5	8	8	7		Similar Soil (saturated, non-plastic)	3
9												
10		6	10.0	12.0	SS	2	7	7	5		Similar Soil (saturated, non-plastic)	6
11												
12												
13												
14												
15		7	15.0	17.0	SS	1	1	4	4		Similar Soil (saturated, non-plastic)	3
16												
17												
18												
19												
20		8	20.0	22.0	SS	3	7	8	8	22.0	Brown cf GRAVEL; some cmf Sand; some Silt (saturated, non-plastic) w = 13.1%	12
21												
22												
23		9	23.0	25.0	SS	8	9	9	9	25.0	Brown cf GRAVEL; and cmf Sand; some Clayey Silt (saturated, slight plasticity)	6
24												
25												

SS Split Spoon Sample
 NX Rock Core
 SH Undisturbed Sample (Shelby Tube)
 Estimated Groundwater

Drillers: Jason Porter; Trenton Coyne
 Inspector: _____

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Boring No.: SB-3

Report No.: BD298E-01-01-26

Sheet 2 of 2

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER	DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	RECOVERY (inches)
			From	To					
26								f - fine m - medium c - coarse and - 35-50% some - 20-35% little - 10-20% trace - 0-10%	
27									
28									
29									
30									
31									
32									
33									
34									
35									
36									
37									
38									
39									
40									
41									
42									
43									
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56									
57									
58									
59									
60									
61									
62									

Boring terminated at 25.0 feet.

Notes:

1. Borehole backfilled with on-site soils.

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Client: Prattsburgh Central School District
 Project: Subsurface Investigation
Proposed Baseball Field Improvements
Prattsburgh, New York

Report No.: BD298E-01-01-26
 Boring Location: See Boring Location Plan

Boring No.: SB-4 Sheet 1 of 2

Start Date: 10/24/2025 Finish Date: 10/24/2025

Coordinates _____ Sampler Hammer
 Latitude _____ Weight: 140 lbs.
 Longitude _____ Fall: 30 in.
 Hammer Type: Automatic

Groundwater Observations			
Date	Time	Depth	Casing
<u>10/24/2025</u>	<u>AM</u>	<u>10.8'</u>	<u>23.0'</u>
_____	_____	_____	_____
_____	_____	_____	_____
_____	_____	_____	_____

Ground Elev.: 1323.2' Boring Advance By: _____
4 1/4" Auger

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER	DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	Recovery (Inches)		
			From	To							
1	RITGC	1	0.0	2.0	SS	3 3 10 14	0.5	6" TOPSOIL & ORGANIC MATERIAL	8		
2		2	2.0	4.0		15 17 17 13			13		
3		3	4.0	6.0		14 7 7 7			6		
4		4	6.0	8.0	SS	15 12 11 11	6.0	Brown cf GRAVEL; some cmf Sand; trace Silt; trace Organic Material (root hairs) (moist, non-plastic) Fill	8		
5		5	8.0	10.0		9 8 8 8			10		
6		6	10.0	12.0		2 2 7 2			5		
7											
8											
9											
10											
11											
12											
13											
14											
15		7	15.0	17.0	SS	5 6 6 2	6.0	Brown cf GRAVEL; little cmf Sand; trace Clayey Silt (saturated, slight plasticity) w = 18.2%	6		
16											
17											
18											
19											
20		8	20.0	22.0	SS	2 7 8 7	6.0	Similar Soil (saturated, slight plasticity)	2		
21											
22											
23		9	23.0	25.0	SS	9 5 3 5	6.0	Similar Soil (saturated, slight plasticity)	6		
24											
25											

SS Split Spoon Sample
 NX Rock Core
 SH Undisturbed Sample (Shelby Tube)
 Estimated Groundwater

Drillers: Jason Porter; Trenton Coyne
 Inspector: _____

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Boring No.: SB-4

Report No.: BD298E-01-01-26

Sheet 2 of 2

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER	DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	RECOVERY (inches)
			From	To					
26									
27							Boring terminated at 25.0 feet.		
28									
29							Notes:		
30							1. Borehole backfilled with on-site soils.		
31									
32									
33									
34									
35									
36									
37									
38									
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41									
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43									
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60									
61									
62									

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Client: Prattsburgh Central School District
 Project: Subsurface Investigation
Proposed Baseball Field Improvements
Prattsburgh, New York

Report No.: BD298E-01-01-26
 Boring Location: See Boring Location Plan

Boring No.: SB-6 Sheet 1 of 1
 Coordinates _____ Sampler Hammer _____
 Latitude _____ Weight: 140 lbs.
 Longitude _____ Fall: 30 in.
 Hammer Type: Automatic
 Ground Elev.: 1326.2' Boring Advance By: _____
4 1/4" Auger

Start Date: 10/29/2025 Finish Date: 10/29/2025
 Groundwater Observations
 Date Time Depth Casing
10/29/2025 AM 8.5' 8.0'

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER	DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	Recovery (Inches)
			From	To					
1	RIT G C C	1	0.0	2.0	SS	10 10 11 6	0.5	5" GRAVEL/MILLINGS	12
2		2	2.0	4.0	SS	9 8 6 6	6.0	Grey cmf SAND; and Clayey Silt; some of Gravel (moist, slight plasticity) Fill	6
3		3	4.0	6.0	SS	4 7 8 6		Grey SILT; little of Gravel; trace cmf Sand (moist, non-plastic) Fill	7
4								Brown cmf SAND; and of Gravel; trace Silt; trace Debris (brick fragments) (moist, non-plastic) Fill	
5		4	6.0	8.0	SS	5 12 9 11	10.0	Brown cmf SAND; and of Gravel; trace Silt (saturated, non-plastic)	6
6	5	8.0	10.0	SS	7 16 12 11	Similar Soil (saturated, non-plastic)		4	
7						Boring terminated at 10.0 feet.	Notes: 1. Borehole backfilled with on-site soils.		
8									
9									
10									
11									
12									
13									
14									
15									
16									
17									
18									
19									
20									
21									
22									
23									
24									
25									

SS Split Spoon Sample
 NX Rock Core
 SH Undisturbed Sample (Shelby Tube)
 Estimated Groundwater

Drillers: Jason Porter; Trenton Coyne
 Inspector: _____

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Client: Prattsburgh Central School District
 Project: Subsurface Investigation
Proposed Baseball Field Improvements
Prattsburgh, New York

Report No.: BD298E-01-01-26
 Boring Location: See Boring Location Plan

Boring No.: SB-7 Sheet 1 of 1
 Coordinates _____ Sampler Hammer
 Latitude _____ Weight: 140 lbs.
 Longitude _____ Fall: 30 in.
 Hammer Type: Automatic
 Ground Elev.: 1324.7' Boring Advance By:
4 1/4" Auger

Start Date: 10/29/2025 Finish Date: 10/29/2025
 Groundwater Observations
 Date Time Depth Casing
10/29/2025 AM DRY 8.0'

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER	DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	Recovery (Inches)
			From	To					
1	R I T C C	1	0.0	2.0	SS	5 14 9 7	0.3	4" TOPSOIL & ORGANIC MATERIAL Brown of GRAVEL; and cmf Sand; some Silt; trace Organic Material (root hairs) (moist, non-plastic) Fill w = 8.2% Similar Soil (moist, non-plastic) Fill w = 8.2%	7
2		2	2.0	4.0		5 5 5 7			6
3		3	4.0	6.0		3 11 8 11			4.0
4		4	6.0	8.0	SS	9 15 27 39	4.0	Brown of GRAVEL; and cmf Sand; some Silt (wet, non-plastic) Brown of GRAVEL; some cmf Sand; some Clayey Silt (wet, slight plasticity) Similar Soil (wet, slight plasticity)	12
5	5	8.0	10.0	21 33 26 36		7			
6	6					10.0			
7								Boring terminated at 10.0 feet. Notes: 1. Borehole backfilled with on-site soils.	
8									
9									
10									
11									
12									
13									
14									
15									
16									
17									
18									
19									
20									
21									
22									
23									
24									
25									

SS Split Spoon Sample
 NX Rock Core
 SH Undisturbed Sample (Shelby Tube)
 Estimated Groundwater

Drillers: Jason Porter; Trenton Coyne
 Inspector: _____

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Client: Prattsburgh Central School District
 Project: Subsurface Investigation
Proposed Baseball Field Improvements
Prattsburgh, New York

Report No.: BD298E-01-01-26
 Boring Location: See Boring Location Plan

Boring No.: SB-8 Sheet 1 of 1
 Coordinates _____ Sampler Hammer
 Latitude _____ Weight: 140 lbs.
 Longitude _____ Fall: 30 in.
 Hammer Type: Automatic
 Ground Elev.: 1324.2' Boring Advance By:
4 1/4" Auger

Start Date: 10/29/2025 Finish Date: 10/29/2025
 Groundwater Observations
 Date Time Depth Casing
10/29/2025 AM DRY 8.0'

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER	DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	Recovery (Inches)
			From	To					
1	R I T C C	1	0.0	2.0	SS	2 6 9 9	0.5	6" TOPSOIL & ORGANIC MATERIAL Brown of GRAVEL; and cmf Sand; little Silt; trace Organic Material (root hairs) (moist, non-plastic) Fill w = 9.1% Similar Soils (moist, non-plastic) Fill	7
2		2	2.0	4.0		14 19 16 13			8
3									
4		3	4.0	6.0	SS	5 11 11 10	4.0	Brown of GRAVEL; and cmf Sand; little Silt (moist, non-plastic) Similar Soil (moist, non-plastic)	8
5									
6		4	6.0	8.0		10 18 12 7			12
7					SS	7 7 7 7		Similar Soil (wet, non-plastic)	6
8		5	8.0	10.0					
9									
10							10.0	Boring terminated at 10.0 feet. Notes: 1. Borehole backfilled with on-site soils.	
11									
12									
13									
14									
15									
16									
17									
18									
19									
20									
21									
22									
23									
24									
25									

SS Split Spoon Sample
 NX Rock Core
 SH Undisturbed Sample (Shelby Tube)
 Estimated Groundwater

Drillers: Jason Porter; Trenton Coyne
 Inspector: _____

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Client: Prattsburgh Central School District
 Project: Subsurface Investigation
Proposed Baseball Field Improvements
Prattsburgh, New York

Report No.: BD298E-01-01-26
 Boring Location: See Boring Location Plan

Boring No.: SD-1 Sheet 1 of 1
 Coordinates _____ Sampler Hammer _____
 Latitude _____ Weight: 140 lbs.
 Longitude _____ Fall: 30 in.
 Hammer Type: Automatic
 Ground Elev.: 1324.4' Boring Advance By: _____
4 1/4" Auger

Start Date: 10/23/2025 Finish Date: 10/23/2025
 Groundwater Observations
 Date Time Depth Casing
10/23/2025 AM 7.5' 10.0'

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER				DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	Recovery (Inches)
			From	To								
1	RITGC	1	0.0	2.0	SS	3	5	10	10	0.4	5" TOPSOIL & ORGANIC MATERIAL Brown of GRAVEL; and cmf Sand; little Silt; trace Organic Material (root hairs) (moist, non-plastic) Fill Brown of GRAVEL; and cmf Sand; little Silt (moist, non-plastic) Fill w = 4.2% Brown of GRAVEL; and cmf Sand; little Silt (wet, non-plastic) Similar Soil (saturated, non-plastic) Similar Soil (saturated, non-plastic)	10
2		2	2.0	4.0		8	12	14	12			12
3		3	4.0	6.0		13	9	13	12			12
4		4	6.0	8.0		6	6	10	10			6
5		5	8.0	10.0		7	5	5	6			8
6										10.0	Boring terminated at 10.0 feet. Notes: 1. A temporary observation well was installed at a depth of 10.0 feet within the borehole.	
7												
8												
9												
10												
11												
12												
13												
14												
15												
16												
17												
18												
19												
20												
21												
22												
23												
24												
25												

SS Split Spoon Sample
 NX Rock Core
 SH Undisturbed Sample (Shelby Tube)
 Estimated Groundwater

Drillers: Jason Porter; Trenton Coyne
 Inspector: _____

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Client: Prattsburgh Central School District
 Project: Subsurface Investigation
Proposed Baseball Field Improvements
Prattsburgh, New York

Report No.: BD298E-01-01-26
 Boring Location: See Boring Location Plan

Boring No.: SD-2 Sheet 1 of 1
 Coordinates _____ Sampler Hammer _____
 Latitude _____ Weight: 140 lbs.
 Longitude _____ Fall: 30 in.
 Hammer Type: Automatic
 Ground Elev.: 1325.4' Boring Advance By: _____
4 1/4" Auger

Start Date: 10/22/2025 Finish Date: 10/22/2025
 Groundwater Observations
 Date Time Depth Casing
10/22/2025 PM 9.5' 10.0'

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER	DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	Recovery (Inches)
			From	To					
1	RITGC	1	0.0	2.0	SS	2 5 12 15	0.5	6" TOPSOIL & ORGANIC MATERIAL	12
2		2	2.0	4.0		25 19 23 28			12
3		3	4.0	6.0		8 7 12 12	4.0	Brown of GRAVEL; and cmf Sand; little Silt (wet, non-plastic)	12
4		4	6.0	8.0		9 7 6 7			7
5		5	8.0	10.0		5 4 4 4			10.0
6									
7									
8									
9									
10									
11									
12									
13									
14									
15									
16									
17									
18									
19									
20									
21									
22									
23									
24									
25									

Drillers: Jason Porter; Trenton Coyne
 Inspector: _____

SS Split Spoon Sample
 NX Rock Core
 SH Undisturbed Sample (Shelby Tube)
 Estimated Groundwater

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Client: Prattsburgh Central School District
 Project: Subsurface Investigation
Proposed Baseball Field Improvements
Prattsburgh, New York

Report No.: BD298E-01-01-26
 Boring Location: See Boring Location Plan

Boring No.: IT-1 Sheet 1 of 1
 Coordinates _____ Sampler Hammer
 Latitude _____ Weight: 140 lbs.
 Longitude _____ Fall: 30 in.
 Hammer Type: Automatic
 Ground Elev.: 1323.2' Boring Advance By:
4 1/4" Auger

Start Date: 10/28/2025 Finish Date: 10/28/2025
 Date Time Depth Casing
10/28/2025 AM DRY 8.0'

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER	DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	Recovery (Inches)
			From	To					
1	P I V C	1	0.0	2.0	SS	4 5 11 8	0.5	6" TOPSOIL & ORGANIC MATERIAL Brown cmf SAND; some cf Gravel; little Silt; trace Organic Material (root hairs) (moist, non-plastic) Fill Brown cf GRAVEL; and cmf Sand; little Silt (moist, non-plastic) Fill Brown cf GRAVEL; little cmf Sand; trace Silt (moist, non-plastic) w = 3.7% Similar Soil (wet, non-plastic) w = 3.7% Brown f GRAVEL; and cmf Sand; little Silt (saturated, non-plastic)	10
2		2	2.0	4.0		7 14 9 9	7		
3		3	4.0	6.0		6 13 13 9	4.0		
4		4	6.0	8.0		7 8 7 7	2		
5		5	8.0	10.0		6 7 6 5	2		
6						10.0	Boring terminated at 10.0 feet. Notes: 1. Borehole backfilled with on-site soils. 2. Offset 4.0 feet from boring IT-1 and advanced augers to 6.1 feet. Installed 4" PVC infiltration test pipe and performed infiltration testing.		
7									
8									
9									
10									
11									
12									
13									
14									
15									
16									
17									
18									
19									
20									
21									
22									
23									
24									
25									

SS Split Spoon Sample
 NX Rock Core
 SH Undisturbed Sample (Shelby Tube)
 Estimated Groundwater

Drillers: Jason Porter; Trenton Coyne
 Inspector: _____

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Client: Prattsburgh Central School District
 Project: Subsurface Investigation
Proposed Baseball Field Improvements
Prattsburgh, New York

Report No.: BD298E-01-01-26
 Boring Location: See Boring Location Plan

Start Date: 10/27/2025 Finish Date: 10/27/2025

Boring No.: IT-2 Sheet 1 of 1

Coordinates
 Latitude _____ Longitude _____
 Sampler Hammer Weight: 140 lbs.
 Fall: 30 in.
 Hammer Type: Automatic

Ground Elev.: 1323.8' Boring Advance By: 4 1/4" Auger

Groundwater Observations			
Date	Time	Depth	Casing
<u>10/27/2025</u>	<u>PM</u>	<u>DRY</u>	<u>8.0'</u>
_____	_____	_____	_____
_____	_____	_____	_____
_____	_____	_____	_____

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER	DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	Recovery (Inches)
			From	To					
1	P I V C	1	0.0	2.0	SS	2 5 6 14	0.5	6" TOPSOIL & ORGANIC MATERIAL Brown cmf SAND; and cf Gravel; little Silt; trace Organic Material (root hairs) (moist, non-plastic) Fill Brown cf GRAVEL; and cmf Sand; little Silt (moist, non-plastic) Similar Soil (moist, non-plastic) Similar Soil (wet, non-plastic) Similar Soil (wet, non-plastic)	12
2		2	2.0	4.0		10 12 12 24	2.0		10
3		3	4.0	6.0		12 18 13 14			10
4		4	6.0	8.0		9 12 12 7			10
5		5	8.0	10.0		6 10 7 10			10
6						10.0	Boring terminated at 10.0 feet. Notes: 1. Borehole backfilled with on-site soils. 2. Offset 4.0 feet from boring IT-2 and advanced augers to 5.8 feet. Installed 4" PVC infiltration test pipe and performed infiltration testing.		
7									
8									
9									
10									
11									
12									
13									
14									
15									
16									
17									
18									
19									
20									
21									
22									
23									
24									
25									

SS Split Spoon Sample
 NX Rock Core
 SH Undisturbed Sample (Shelby Tube)
 Estimated Groundwater

Drillers: Jason Porter; Trenton Coyne
 Inspector: _____

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Client: Prattsburgh Central School District
 Project: Subsurface Investigation
Proposed Baseball Field Improvements
Prattsburgh, New York

Report No.: BD298E-01-01-26
 Boring Location: See Boring Location Plan

Boring No.: IT-3 Sheet 1 of 1
 Coordinates _____ Sampler Hammer
 Latitude _____ Weight: 140 lbs.
 Longitude _____ Fall: 30 in.
 Hammer Type: Automatic
 Ground Elev.: 1323.3' Boring Advance By:
4 1/4" Auger

Start Date: 10/28/2025 Finish Date: 10/28/2025
 Date Time Depth Casing
10/28/2025 AM DRY 8.0'

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER				DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	Recovery (Inches)
			From	To		1	2	6	14			
1	P I V C	1	0.0	2.0	SS	1	2	6	14	0.8	9" TOPSOIL & ORGANIC MATERIAL	11
2		2	2.0	4.0		13	13	12	11	2.0	Brown cmf SAND; some cf Gravel; little Silt; trace Organic Material (root hairs) (moist, non-plastic) Fill	12
3		3	4.0	6.0		8	11	11	11		Similar Soil (moist, non-plastic)	8
4		4	6.0	8.0		7	13	11	6		Similar Soil (moist, non-plastic) w = 6.6%	6
5		5	8.0	10.0		3	9	6	3		Brown f GRAVEL; and cmf Sand; little Silt (saturated, non-plastic)	2
6										10.0		
7												
8												
9												
10												
11												
12												
13												
14												
15												
16												
17												
18												
19												
20												
21												
22												
23												
24												
25												

SS Split Spoon Sample
 NX Rock Core
 SH Undisturbed Sample (Shelby Tube)
 Estimated Groundwater

Drillers: Jason Porter; Trenton Coyne
 Inspector: _____

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Client: Prattsburgh Central School District
 Project: Subsurface Investigation
Proposed Baseball Field Improvements
Prattsburgh, New York

Report No.: BD298E-01-01-26
 Boring Location: See Boring Location Plan

Start Date: 10/28/2025 Finish Date: 10/28/2025

Boring No.: IT-4 Sheet 1 of 1

Groundwater Observations			
Date	Time	Depth	Casing
<u>10/28/2025</u>	<u>PM</u>	<u>DRY</u>	<u>8.0'</u>

Coordinates
 Latitude _____ Longitude _____
 Sampler Hammer Weight: 140 lbs.
 Fall: 30 in.
 Hammer Type: Automatic
 Ground Elev.: 1323.3' Boring Advance By:
4 1/4" Auger

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER	DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	Recovery (Inches)
			From	To					
1	PVC	1	0.0	2.0	SS	1 4 8 10	0.7	8" TOPSOIL & ORGANIC MATERIAL	14
2		2	2.0	4.0		10 11 17 11	4.0	Brown SILT; and cmf Sand; some of Gravel; trace Organic Material (root hairs) (moist, non-plastic) Fill	6
3		3	4.0	6.0		3 14 9 14		Brown of GRAVEL; and cmf Sand; little Silt (moist, non-plastic) Fill	8
4								4	6.0
5		5	8.0	10.0		6 7 7 6	10.0	Brown of GRAVEL; and cmf Sand; little Silt (saturated, non-plastic)	3
6							Boring terminated at 10.0 feet. Notes: 1. Borehole backfilled with on-site soils. 2. Offset 4.0 feet from boring IT-4 and advanced augers to 5.9 feet. Installed 4" PVC infiltration test pipe and performed infiltration testing.		
7									
8									
9									
10									
11									
12									
13									
14									
15									
16									
17									
18									
19									
20									
21									
22									
23									
24									
25									

SS Split Spoon Sample
 NX Rock Core
 SH Undisturbed Sample (Shelby Tube)
 Estimated Groundwater

Drillers: Jason Porter; Trenton Coyne
 Inspector: _____

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Client: Prattsburgh Central School District
 Project: Subsurface Investigation
Proposed Baseball Field Improvements
Prattsburgh, New York

Report No.: BD298E-01-01-26
 Boring Location: See Boring Location Plan

Boring No.: IT-5 Sheet 1 of 1
 Coordinates _____ Sampler Hammer _____
 Latitude _____ Weight: 140 lbs.
 Longitude _____ Fall: 30 in.
 Hammer Type: Automatic
 Ground Elev.: 1323.1' Boring Advance By: _____
4 1/4" Auger

Start Date: 10/28/2025 Finish Date: 10/28/2025
 Groundwater Observations
 Date Time Depth Casing
10/28/2025 PM DRY 8.0'

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER	DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	Recovery (Inches)
			From	To					
1	PVC	1	0.0	2.0	SS	4 5 10 11	0.5	6" TOPSOIL & ORGANIC MATERIAL Brown cmf SAND; and cf Gravel; little Silt; trace Organic Material (root hairs) (moist, non-plastic) Fill Brown cf GRAVEL; and cmf Sand; little Silt (moist, non-plastic) Fill	10
2		2	2.0	4.0		16 12 10 7			8
3									
4		3	4.0	6.0	SS	4 9 7 12	4.0	Brown cf GRAVEL; and cmf Sand; little Silt (wet, non-plastic) NO RECOVERY	7
5									
6		4	6.0	8.0		6 8 7 6			0
7					SS			Brown f GRAVEL; some cmf Sand; little Silt (wet, non-plastic)	
8		5	8.0	10.0		6 8 8 8			1
9									
10							10.0		
11								Boring terminated at 10.0 feet. Notes: 1. Borehole backfilled with on-site soils. 2. Offset 4.0 feet from boring IT-5 and advanced augers to 5.8 feet. Installed 4" PVC infiltration test pipe and performed infiltration testing.	
12									
13									
14									
15									
16									
17									
18									
19									
20									
21									
22									
23									
24									
25									

SS Split Spoon Sample
 NX Rock Core
 SH Undisturbed Sample (Shelby Tube)
 Estimated Groundwater

Drillers: Jason Porter; Trenton Coyne
 Inspector: _____

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Client: Prattsburgh Central School District
 Project: Subsurface Investigation
Proposed Baseball Field Improvements
Prattsburgh, New York

Report No.: BD298E-01-01-26
 Boring Location: See Boring Location Plan

Boring No.: IT-6 Sheet 1 of 1
 Coordinates _____ Sampler Hammer
 Latitude _____ Weight: 140 lbs.
 Longitude _____ Fall: 30 in.
 Hammer Type: Automatic
 Ground Elev.: 1323.2' Boring Advance By:
4 1/4" Auger

Start Date: 10/29/2025 Finish Date: 10/29/2025
 Groundwater Observations
 Date Time Depth Casing
10/29/2025 AM DRY 8.0'

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER				DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	Recovery (Inches)
			From	To		1	2	7	9			
1	PVC	1	0.0	2.0	SS	1	2	7	9	0.5	6" TOPSOIL & ORGANIC MATERIAL Brown cmf SAND; and cf Gravel; little Silt; trace Organic Material (root hairs) (moist, non-plastic) Fill	10
2		2	2.0	4.0		12	12	10	9			8
3		3	4.0	6.0		7	11	16	20			8
4		4	6.0	8.0	SS	14	14	8	9	6.0	Brown cf GRAVEL; and cmf Sand; trace Silt (moist, non-plastic) Fill Brown cf GRAVEL; some cmf Sand; little Clayey Silt (wet, slight plasticity) w = 6.1%	8
5		5	8.0	10.0		8	5	7	10			6
6												
7										10.0	Boring terminated at 10.0 feet. Notes: 1. Borehole backfilled with on-site soils. 2. Offset 4.0 feet from boring IT-6 and advanced augers to 5.7 feet. Installed 4" PVC infiltration test pipe and performed infiltration testing.	
8												
9												
10												
11												
12												
13												
14												
15												
16												
17												
18												
19												
20												
21												
22												
23												
24												
25												

SS Split Spoon Sample
 NX Rock Core
 SH Undisturbed Sample (Shelby Tube)
 Estimated Groundwater

Drillers: Jason Porter; Trenton Coyne
 Inspector: _____

ATLANTIC TESTING LABORATORIES, Limited

Subsurface Investigation

Client: Prattsburgh Central School District
 Project: Subsurface Investigation
Proposed Baseball Field Improvements
Prattsburgh, New York

Report No.: BD298E-01-01-26
 Boring Location: See Boring Location Plan

Boring No.: IT-7 Sheet 1 of 1
 Coordinates _____ Sampler Hammer _____
 Latitude _____ Weight: 140 lbs.
 Longitude _____ Fall: 30 in.
 Hammer Type: Automatic
 Ground Elev.: 1323.7' Boring Advance By: _____
4 1/4" Auger

Start Date: 10/24/2025 Finish Date: 10/24/2025
 Groundwater Observations
 Date Time Depth Casing
10/24/2025 PM DRY 8.0'

ATL-LOG1 LL BD298 PRATTSBURGH CENTRAL SCHOOL DISTRICT - PRATTSBURGH.GPJ ATL4-08.GDT 1/16/26

DEPTH	METHOD OF ADVANCE	SAMPLE NO.	DEPTH OF SAMPLE		SAMPLE TYPE	BLOWS ON SAMPLER PER 6" 2" O.D. SAMPLER	DEPTH OF CHANGE	CLASSIFICATION OF MATERIAL	Recovery (Inches)
			From	To					
1	PVC	1	0.0	2.0	SS	2 4 11 11	0.5	6" TOPSOIL & ORGANIC MATERIAL	17
2		2	2.0	4.0		15 14 12 12		Brown cmf SAND; and cf Gravel; trace Silt; trace Organic Material (root hairs) (moist, non-plastic) Fill NO RECOVERY	0
3									
4		3	4.0	6.0		9 10 10 14	6.0	Brown cf GRAVEL; some cmf Sand; trace Silt (moist, non-plastic) Fill	8
5									
6	4	6.0	8.0	21 12 9 14	10.0	Brown cf GRAVEL; some cmf Sand; trace Silt (moist, non-plastic) w = 6.1%	12		
7									
8	5	8.0	10.0	18 10 27 25		Brown Clayey SILT; some cf Gravel; some cmf Sand (wet, slight plasticity)	8		
9									
10									
11									
12									
13									
14									
15									
16									
17									
18									
19									
20									
21									
22									
23									
24									
25									

SS Split Spoon Sample
 NX Rock Core
 SH Undisturbed Sample (Shelby Tube)
 Estimated Groundwater

Drillers: Jason Porter; Trenton Coyne
 Inspector: _____

APPENDIX D

GEOTECHNICAL LABORATORY TEST RESULTS



LABORATORY DETERMINATION OF MOISTURE CONTENT OF SOILS
ASTM D 2216

PROJECT INFORMATION

Client: Hunt Engineers, Architects, Land Surveyors &
Landscape Architect, DPC
Project: Proposed Baseball Field Improvements

ATL Report No.: BD298SL-01-01-26
Report Date: January 7, 2026
Date Received: December 1, 2025

TEST DATA

Boring No.	Sample No.	Depth (ft)	Moisture Content (%)
SB-2	S - 3	4.0 - 6.0	5.9
SB-3	S - 3	4.0 - 6.0	5.1
SB-3	S - 8	20.0 - 22.0	13.1
SB-4	S - 5	8.0 - 10.0	11.5
SB-4	S - 7	15.0 - 17.0	18.2
SB-7	S - 1,2	0.5 - 4.0	8.2
SB-8	S - 1	0.5 - 2.0	9.1
SD-1	S - 2	2.0 - 4.0	4.2
IT-1	S - 3,4	4.0 - 8.0	3.7
IT-3	S - 4	6.0 - 8.0	6.6
IT-6	S - 4	6.0 - 8.0	6.1
IT-7	S - 4	6.0 - 8.0	6.1

REMARKS

1. No material was excluded from the test sample.

Reviewed By: 

Date: 1-8-2026



ATLANTIC TESTING LABORATORIES

Particle Size Distribution Report

ASTM D 422

Project: Proposed Baseball Field Improvements

Report No.: BD298SL-01-01-26

Client: Hunt Engineers

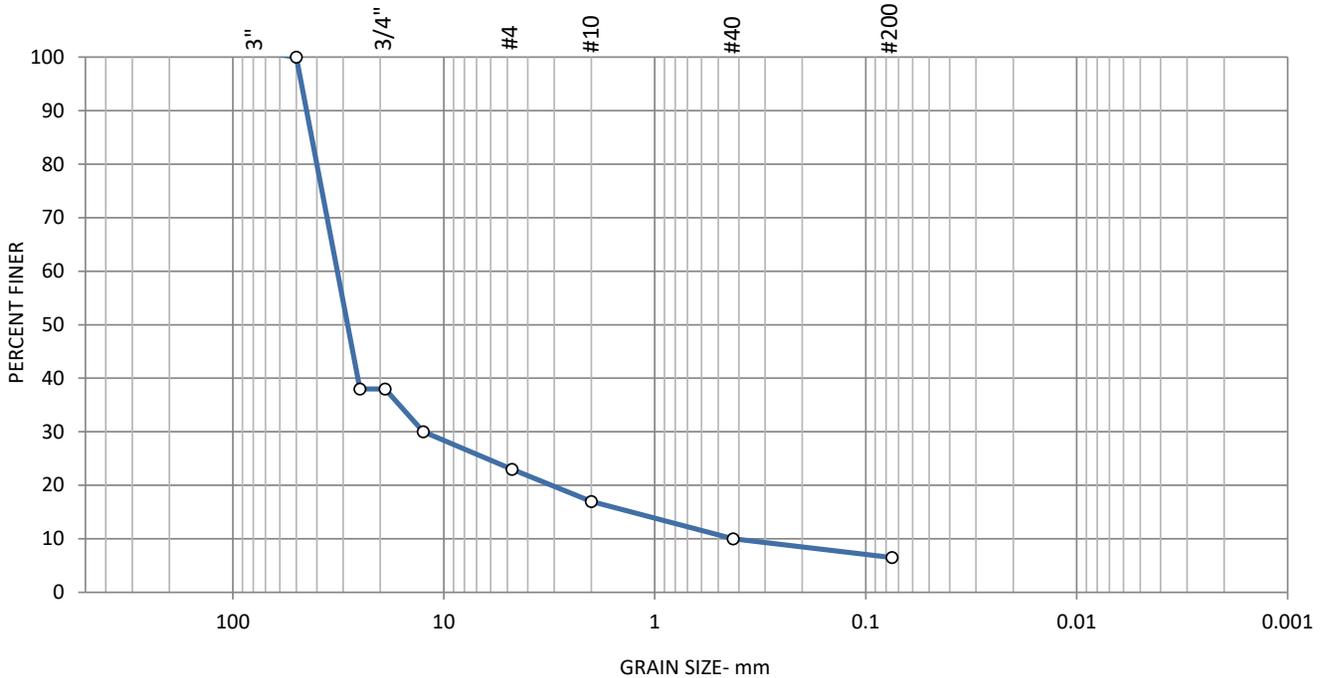
Test Date: 1/7/2026

Sample No: IT-1, S-3 & S-4

Source of Sample: Boring Sample

Location: In-place

Elev./Depth (ft): 4.0-8.0



% Cobbles	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
	62	15	6	7	3		

SIEVE SIZE	PERCENT FINER	SPEC. PERCENT	OUT OF SPEC.
4"			
3"			
2"	100		
1"	38		
3/4"	38		
1/2"	30		
#4	23		
#10	17		
#40	10		
#200	6.5		

Soil Description
Brown of GRAVEL; little cmf Sand; trace Silt

Moisture Content = 3.7%

Atterburg Limits
PL= LL= PI=

Coefficients
D₆₀= 33.845 D₃₀= 12.147 D₁₀= 0.559
C_u= 60.546 C_c= 7.799

Classification
USCS= GP AASHTO=

Remarks

ATLANTIC TESTING LABORATORIES, LIMITED

Reviewed by: Paul Lynch

Date: 1-8-2026



ATLANTIC TESTING LABORATORIES

Particle Size Distribution Report

ASTM D 422

Project: Proposed Baseball Field Improvements

Report No.: BD298SL-01-01-26

Client: Hunt Engineers

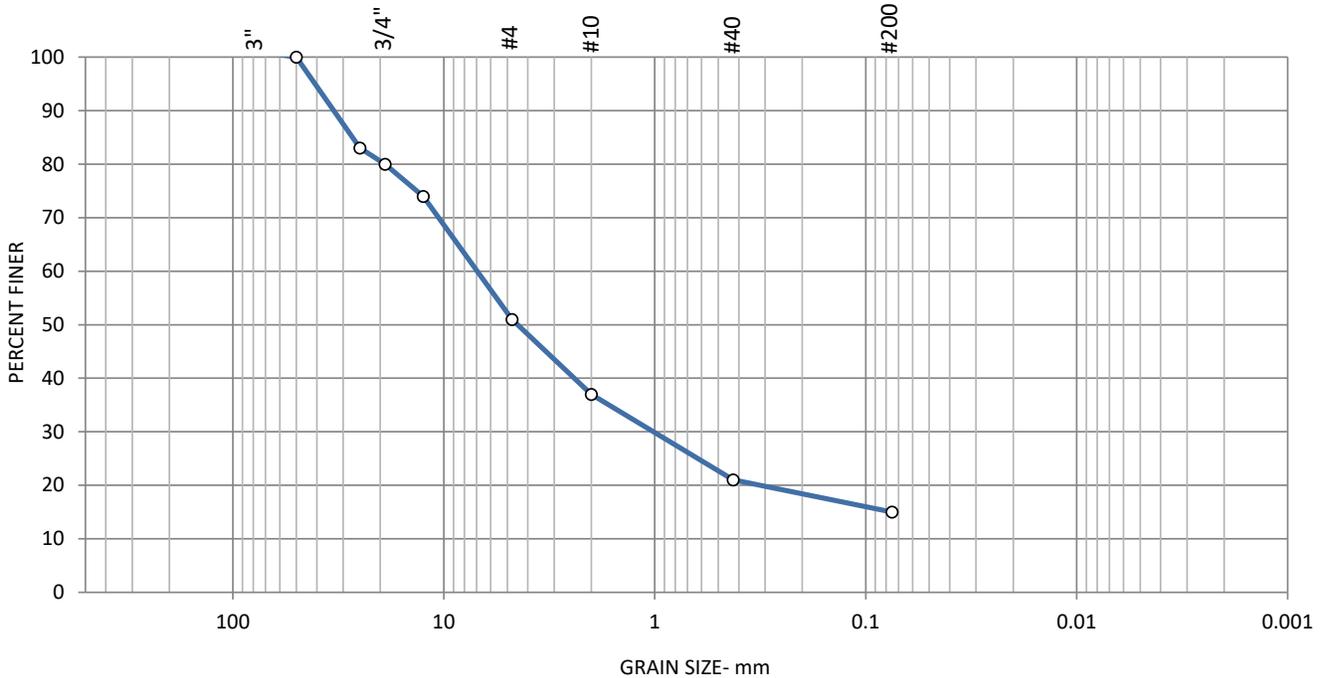
Test Date: 1/7/2026

Sample No: IT-3 S-4

Source of Sample: Boring Sample

Location: In-place

Elev./Depth (ft): 6.0-8.0



% Cobbles	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
	20	29	14	16	6	15	

SIEVE SIZE	PERCENT FINER	SPEC. PERCENT	OUT OF SPEC.
4"			
3"			
2"	100		
1"	83		
3/4"	80		
1/2"	74		
#4	51		
#10	37		
#40	21		
#200	15		

Soil Description
Brown of GRAVEL; and cmf Sand; little Silt

Moisture Content = 6.6%

Atterburg Limits
PL= LL= PI=

Coefficients
D₆₀= 7.329 D₃₀= 1.218 D₁₀=
C_u= NA C_c= NA

Classification
USCS= GP AASHTO=

Remarks

ATLANTIC TESTING LABORATORIES, LIMITED

Reviewed by: Paul Smith

Date: 1-8-2026



ATLANTIC TESTING LABORATORIES

Particle Size Distribution Report

ASTM D 422

Project: Proposed Baseball Field Improvements

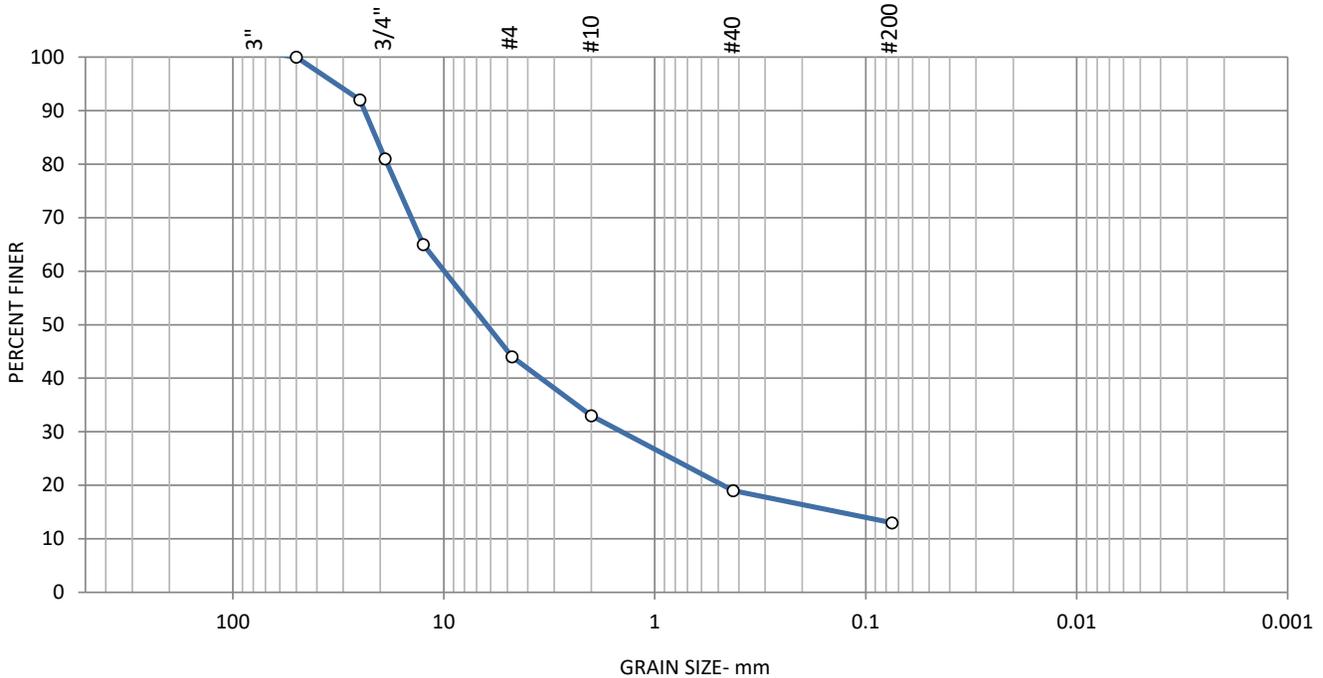
Report No.: BD298SL-01-01-26

Client: Hunt Engineers

Test Date: 1/7/2026

Sample No: IT-6, S-4
Location: In-place

Source of Sample: Boring Sample
Elev./Depth (ft): 6.0-8.0



% Cobbles	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
	19	37	11	14	6	13	

SIEVE SIZE	PERCENT FINER	SPEC. PERCENT	OUT OF SPEC.
4"			
3"			
2"	100		
1"	92		
3/4"	81		
1/2"	65		
#4	44		
#10	33		
#40	19		
#200	13		

Soil Description
Brown of GRAVEL; some cmf Sand; little Clayey Silt

Moisture Content = 6.1%

Atterburg Limits
PL= LL= PI=

Coefficients
D₆₀= 10.322 D₃₀= 1.612 D₁₀=
C_u= NA C_c= NA

Classification
USCS= GP AASHTO=

Remarks

ATLANTIC TESTING LABORATORIES, LIMITED

Reviewed by: Paul Lynch

Date: 1-8-2026



ATLANTIC TESTING LABORATORIES

Particle Size Distribution Report

ASTM D 422

Project: Proposed Baseball Field Improvements

Report No.: BD298SL-01-01-26

Client: Hunt Engineers

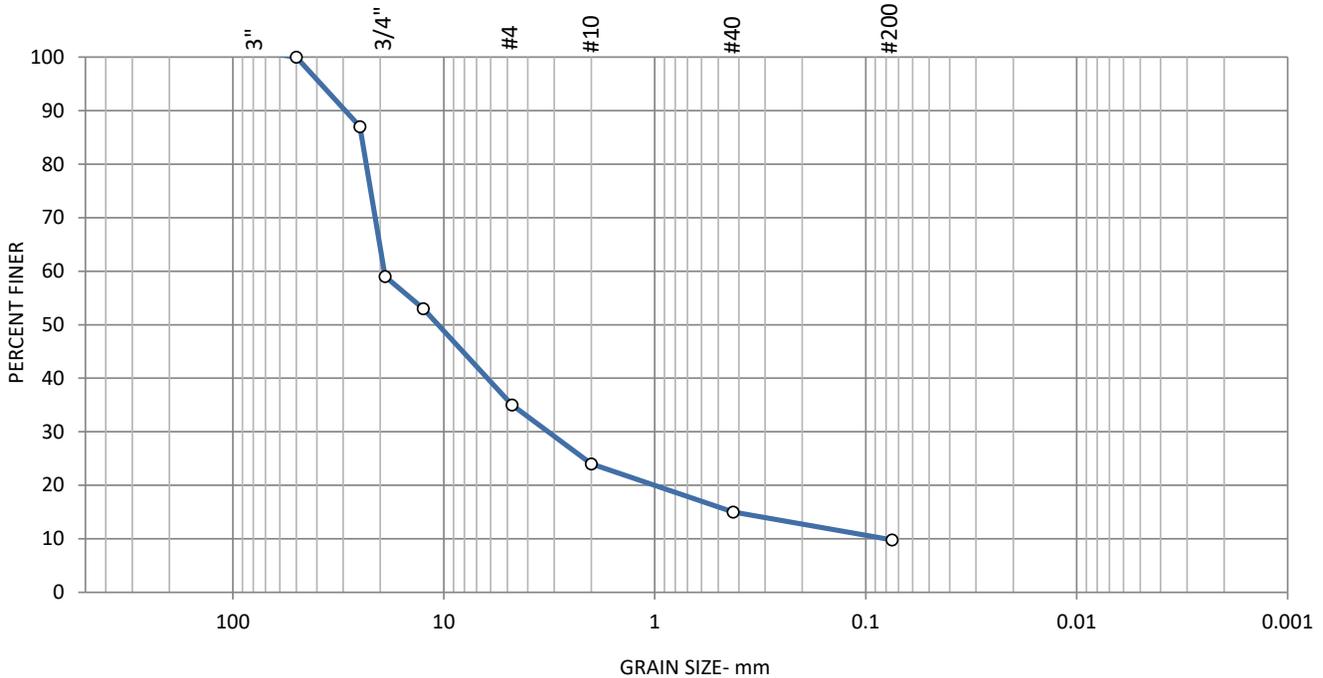
Test Date: 1/7/2026

Sample No: IT-7, S-4

Source of Sample: Boring Sample

Location: In-place

Elev./Depth (ft): 6.0-8.0



% Cobbles	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
	41	24	11	9	5	10	

SIEVE SIZE	PERCENT FINER	SPEC. PERCENT	OUT OF SPEC.
4"			
3"			
2"	100		
1"	87		
3/4"	59		
1/2"	53		
#4	35		
#10	24		
#40	15		
#200	9.8		

Soil Description
Brown of GRAVEL; some cmf Sand; trace Silt

Moisture Content = 6.1%

Atterburg Limits
PL= LL= PI=

Coefficients
D₆₀= 19.175 D₃₀= 3.437 D₁₀= 0.083
C_u= 231.02 C_c= 7.422

Classification
USCS= GP AASHTO=

Remarks

ATLANTIC TESTING LABORATORIES, LIMITED

Reviewed by: Paul Lynch

Date: 1-8-2026



ATLANTIC TESTING LABORATORIES

Particle Size Distribution Report

ASTM D 422

Project: Proposed Baseball Field Improvements

Report No.: BD298SL-01-01-26

Client: Hunt Engineers

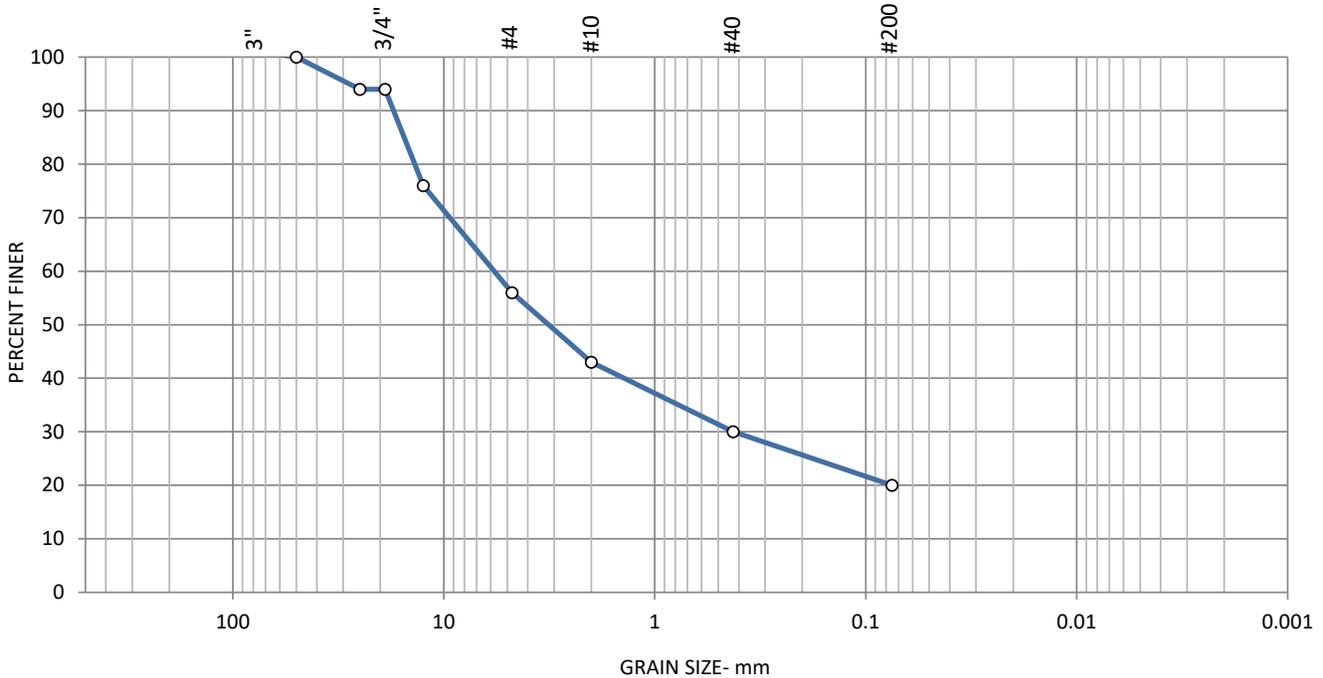
Test Date: 1/7/2026

Sample No: SB-2, S-3

Source of Sample: Boring Sample

Location: In-place

Elev./Depth (ft): 4.0-6.0



% Cobbles	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
	6	38	13	13	10	20	

SIEVE SIZE	PERCENT FINER	SPEC. PERCENT	OUT OF SPEC.
4"			
3"			
2"	100		
1"	94		
3/4"	94		
1/2"	76		
#4	56		
#10	43		
#40	30		
#200	20		

Soil Description
Brpwn of GRAVEL; and cmf Sand; little Silt

Moisture Content = 5.9%

Atterburg Limits
PL= LL= PI=

Coefficients
D₆₀= 6.024 D₃₀= 0.574 D₁₀=
C_u= NA C_c= NA

Classification
USCS= GP AASHTO=

Remarks

ATLANTIC TESTING LABORATORIES, LIMITED

Reviewed by: Paul Lynch

Date: 1-8-2026



Particle Size Distribution Report

ASTM D 422

Project: Proposed Baseball Field Improvements

Report No.: BD298SL-01-01-26

Client: Hunt Engineers

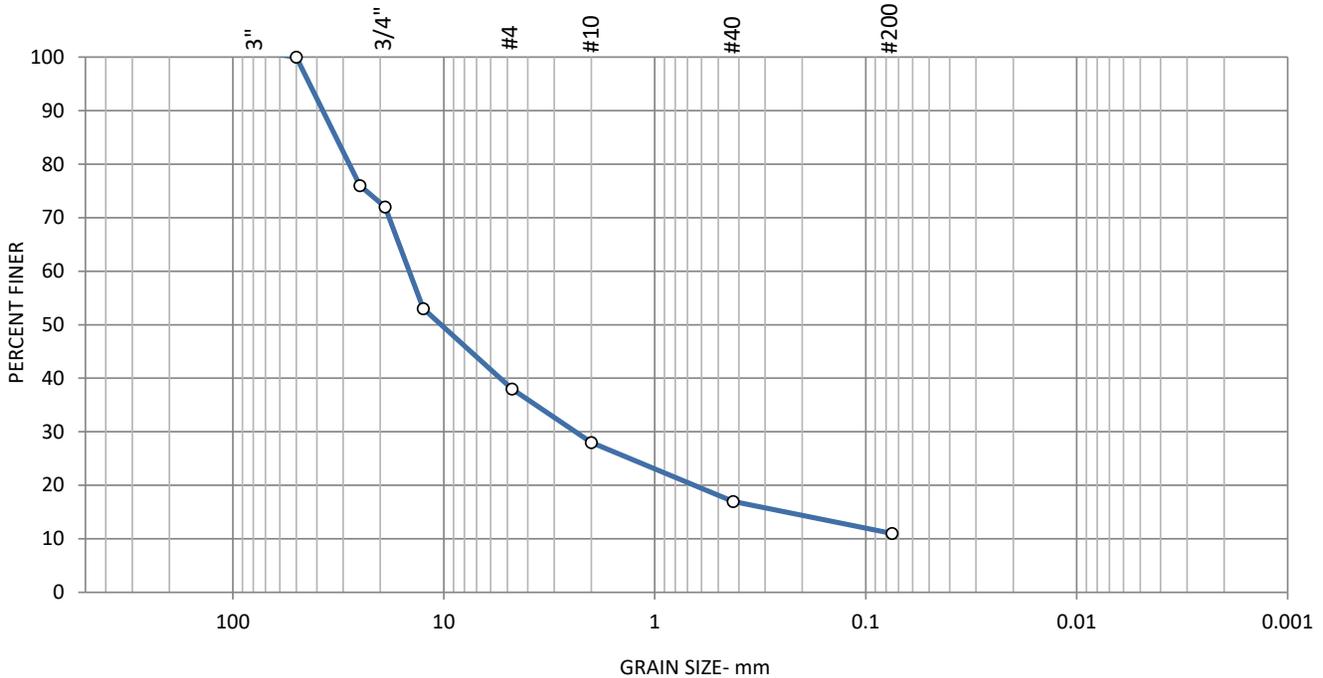
Test Date: 1/7/2026

Sample No: SB-3, S-3

Source of Sample: Boring Sample

Location: In-place

Elev./Depth (ft): 4.0-6.0



% Cobbles	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
	28	34	10	11	6	11	

SIEVE SIZE	PERCENT FINER	SPEC. PERCENT	OUT OF SPEC.
4"			
3"			
2"	100		
1"	76		
3/4"	72		
1/2"	53		
#4	38		
#10	28		
#40	17		
#200	11		

Soil Description
Brown of GRAVEL; some cmf Sand; little Silt

Moisture Content = 5.1%

Atterburg Limits
PL= LL= PI=

Coefficients
D₆₀= 14.976 D₃₀= 2.583 D₁₀=
C_u= NA C_c= NA

Classification
USCS= GP AASHTO=

Remarks

ATLANTIC TESTING LABORATORIES, LIMITED

Reviewed by: Paul Lynch

Date: 1-8-2026



ATLANTIC TESTING LABORATORIES

Particle Size Distribution Report

ASTM D 422

Project: Proposed Baseball Field Improvements

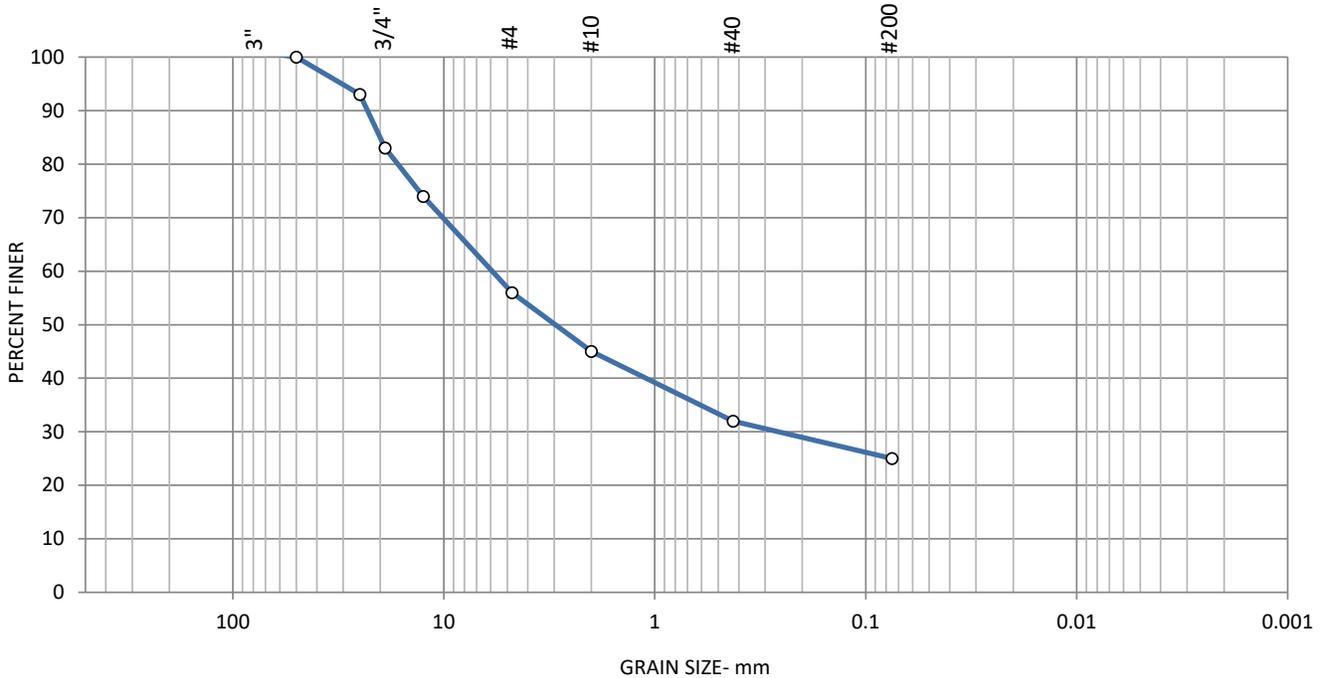
Report No.: BD298SL-01-01-26

Client: Hunt Engineers

Test Date: 1/7/2026

Sample No: SB-3, S-8
Location: In-place

Source of Sample: Boring Sample
Elev./Depth (ft): 20.0-22.0



% Cobbles	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
	17	27	11	13	7	25	

SIEVE SIZE	PERCENT FINER	SPEC. PERCENT	OUT OF SPEC.
4"			
3"			
2"	100		
1"	93		
3/4"	83		
1/2"	74		
#4	56		
#10	45		
#40	32		
#200	25		

Soil Description
Brown of GRAVEL; some cmf Sand; some Silt

Moisture Content = 13.1%

Atterburg Limits
PL= LL= PI=

Coefficients
D₆₀= 6.3 D₃₀= 0.42 D₁₀=
C_u= NA C_c= NA

Classification
USCS= GP AASHTO=

Remarks

ATLANTIC TESTING LABORATORIES, LIMITED

Reviewed by: Paul Smith

Date: 1-8-2026



ATLANTIC TESTING LABORATORIES

Particle Size Distribution Report

ASTM D 422

Project: Proposed Baseball Field Improvements

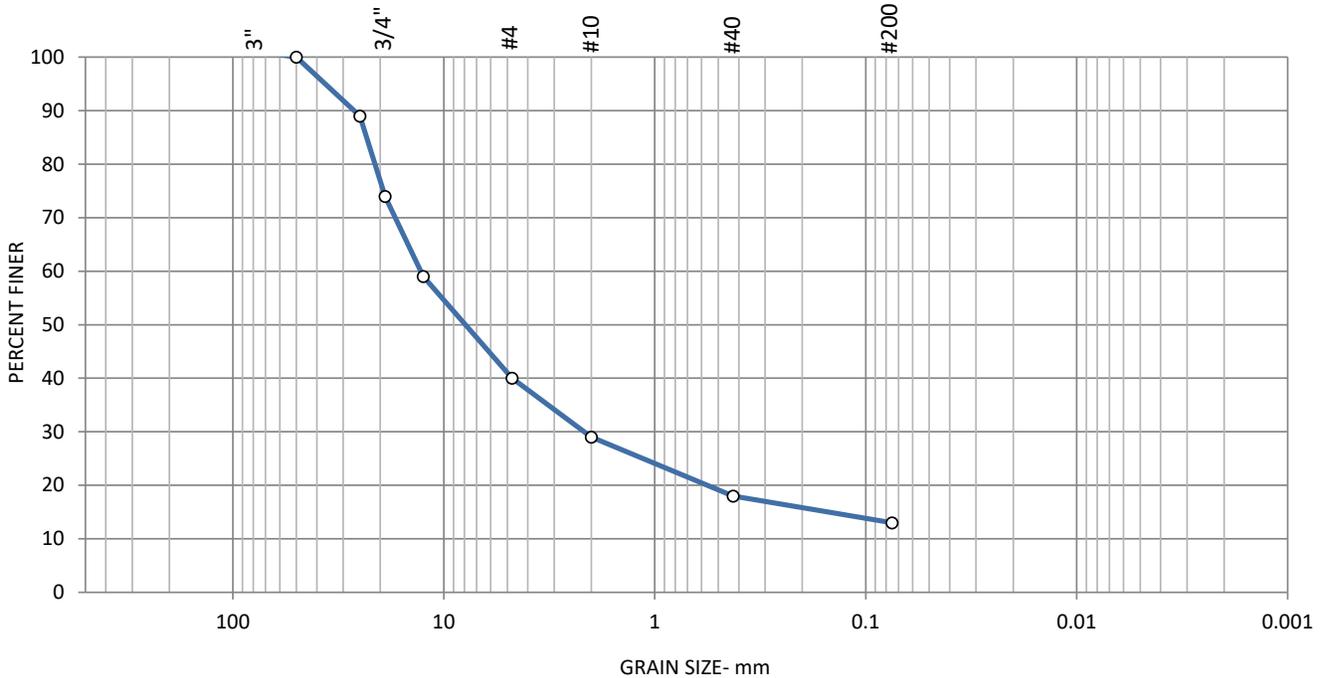
Report No.: BD298SL-01-01-26

Client: Hunt Engineers

Test Date: 1/7/2026

Sample No: SB-4, S-5
Location: In-place

Source of Sample: Boring Sample
Elev./Depth (ft): 8.0-10.0



% Cobbles	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
	26	34	11	11	5	13	

SIEVE SIZE	PERCENT FINER	SPEC. PERCENT	OUT OF SPEC.
4"			
3"			
2"	100		
1"	89		
3/4"	74		
1/2"	59		
#4	40		
#10	29		
#40	18		
#200	13		

Soil Description
Brown of GRAVEL; some cmf Sand; little Clayey Silt

Moisture Content = 11.5%

Atterburg Limits
PL= LL= PI=

Coefficients
D₆₀= 13.076 D₃₀= 2.175 D₁₀=
C_u= NA C_c= NA

Classification
USCS= GP AASHTO=

Remarks

ATLANTIC TESTING LABORATORIES, LIMITED

Reviewed by: Paul Lynch

Date: 1-8-2026



ATLANTIC TESTING LABORATORIES

Particle Size Distribution Report

ASTM D 422

Project: Proposed Baseball Field Improvements

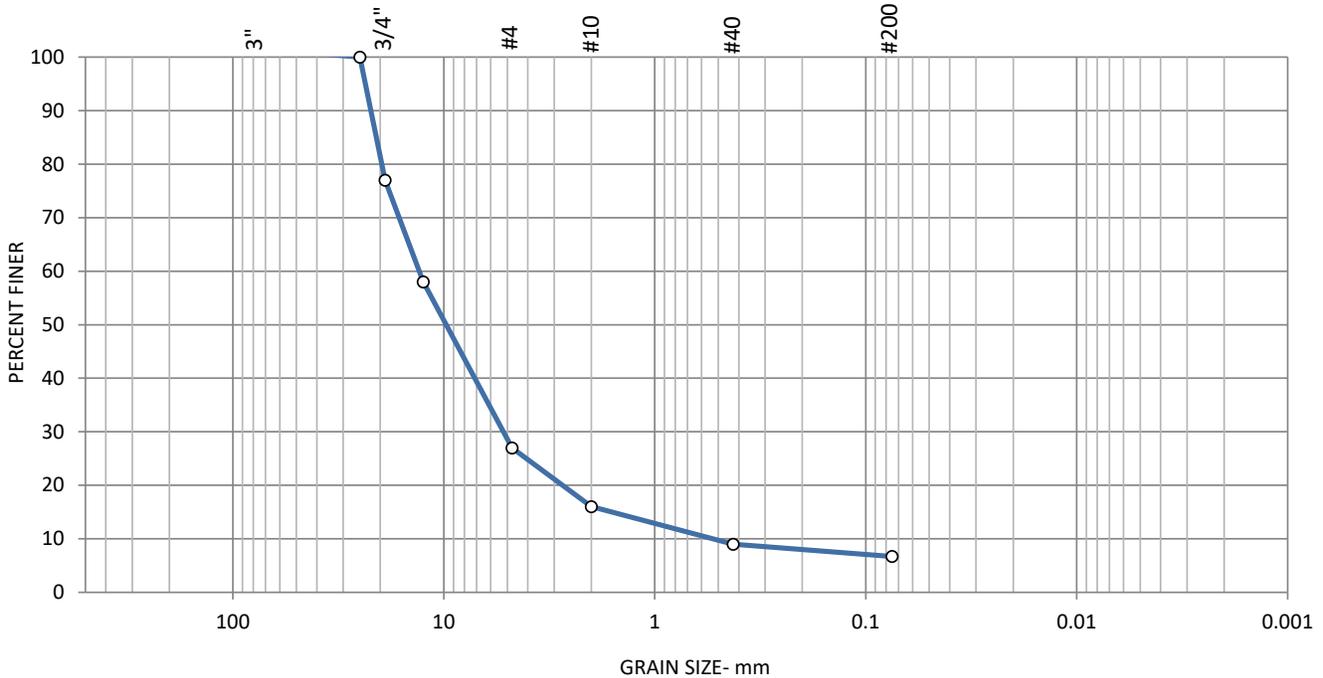
Report No.: BD298SL-01-01-26

Client: Hunt Engineers

Test Date: 1/7/2026

Sample No: SB-4, S-7
Location: In-place

Source of Sample: Boring Sample
Elev./Depth (ft): 15.0-17.0



% Cobbles	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
	23	50	11	7	2		

SIEVE SIZE	PERCENT FINER	SPEC. PERCENT	OUT OF SPEC.
4"			
3"			
2"			
1"	100		
3/4"	77		
1/2"	58		
#4	27		
#10	16		
#40	9		
#200	6.7		

Soil Description
Brown of GRAVEL; little cmf Sand; trace Clayey Silt

Moisture Content = 18.2%

Atterburg Limits
PL= LL= PI=

Coefficients
D₆₀= 13.122 D₃₀= 5.567 D₁₀= 0.68
C_u= 19.297 C_c= 3.473

Classification
USCS= GP AASHTO=

Remarks

ATLANTIC TESTING LABORATORIES, LIMITED

Reviewed by: Paul Lynch

Date: 1-8-2026



ATLANTIC TESTING LABORATORIES

Particle Size Distribution Report

ASTM D 422

Project: Proposed Baseball Field Improvements

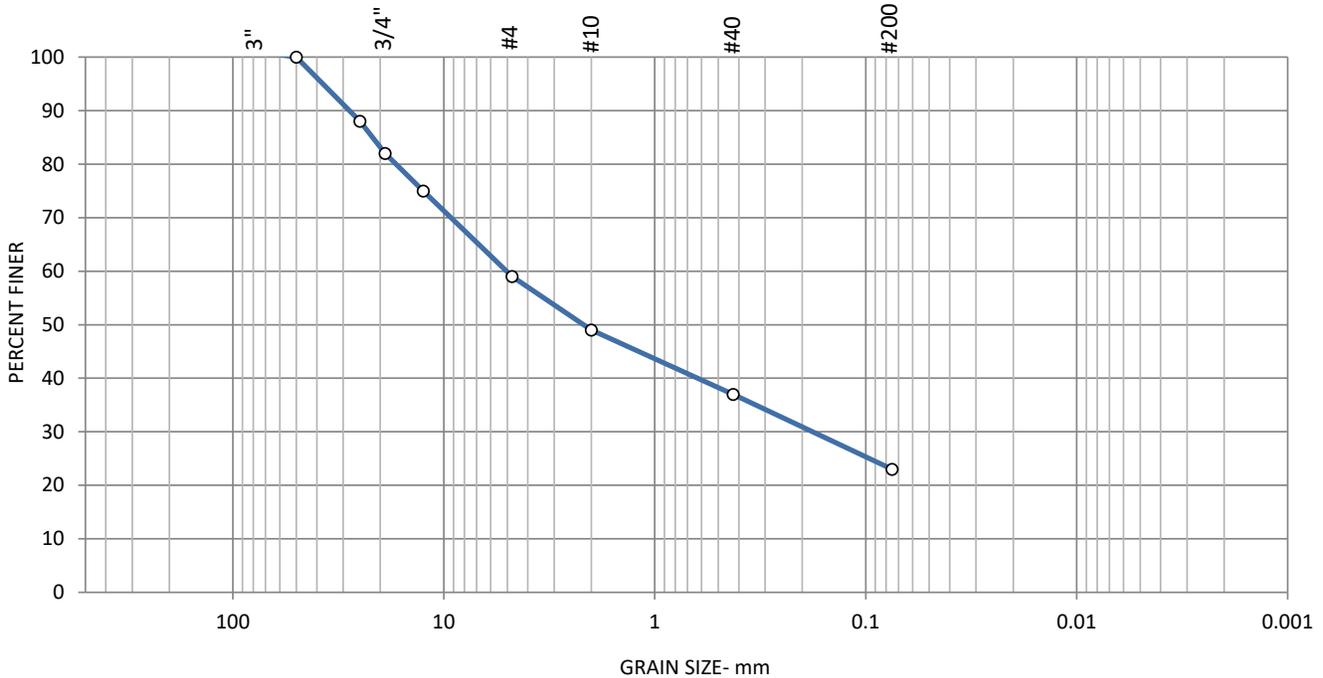
Report No.: BD298SL-01-01-26

Client: Hunt Engineers

Test Date: 1/7/2026

Sample No: SB-7, S-1 & S-2
Location: In-place

Source of Sample: Boring Sample
Elev./Depth (ft): 0.5-4.0



% Cobbles	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
	18	23	10	12	14	23	

SIEVE SIZE	PERCENT FINER	SPEC. PERCENT	OUT OF SPEC.
4"			
3"			
2"	100		
1"	88		
3/4"	82		
1/2"	75		
#4	59		
#10	49		
#40	37		
#200	23		

Soil Description
Brown of GRAVEL; and cmf Sand; some Silt

Moisture Content = 8.2%

Atterburg Limits
PL= LL= PI=

Coefficients
D₆₀= 5.267 D₃₀= 0.275 D₁₀=
C_u= NA C_c= NA

Classification
USCS= GP AASHTO=

Remarks

ATLANTIC TESTING LABORATORIES, LIMITED

Reviewed by: Paul Lynch

Date: 1-8-2026



ATLANTIC TESTING LABORATORIES

Particle Size Distribution Report

ASTM D 422

Project: Proposed Baseball Field Improvements

Report No.: BD298SL-01-01-26

Client: Hunt Engineers

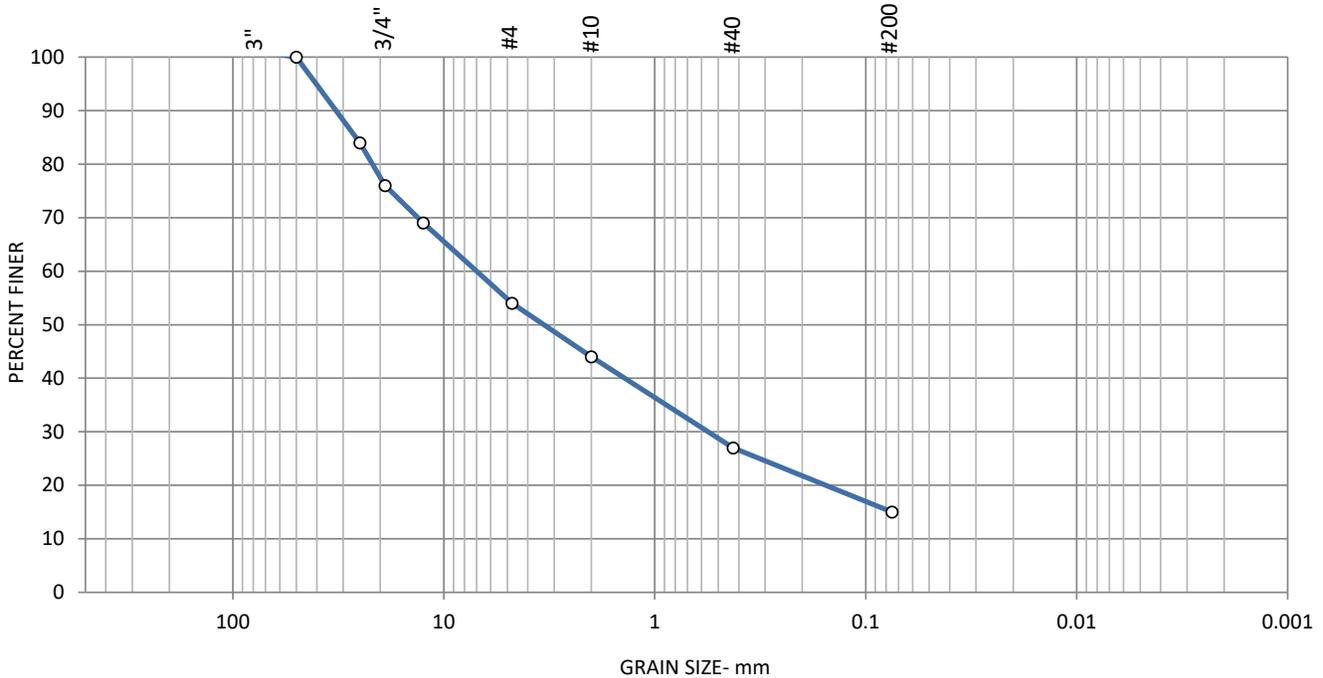
Test Date: 1/7/2026

Sample No: SB-8, S-1

Source of Sample: Boring Sample

Location: In-place

Elev./Depth (ft): 0.5-2.0



% Cobbles	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
	24	22	10	17	12	15	

SIEVE SIZE	PERCENT FINER	SPEC. PERCENT	OUT OF SPEC.
4"			
3"			
2"	100		
1"	84		
3/4"	76		
1/2"	69		
#4	54		
#10	44		
#40	27		
#200	15		

Soil Description
Brown of GRAVEL; and cmf Sand; little Silt

Moisture Content = 9.1%

Atterburg Limits
PL= LL= PI=

Coefficients
D₆₀= 7.235 D₃₀= 0.731 D₁₀=
C_u= NA C_c= NA

Classification
USCS= GP AASHTO=

Remarks

ATLANTIC TESTING LABORATORIES, LIMITED

Reviewed by: Paul Smith

Date: 1-8-2026



ATLANTIC TESTING LABORATORIES

Particle Size Distribution Report

ASTM D 422

Project: Proposed Baseball Field Improvements

Report No.: BD298SL-01-01-26

Client: Hunt Engineers

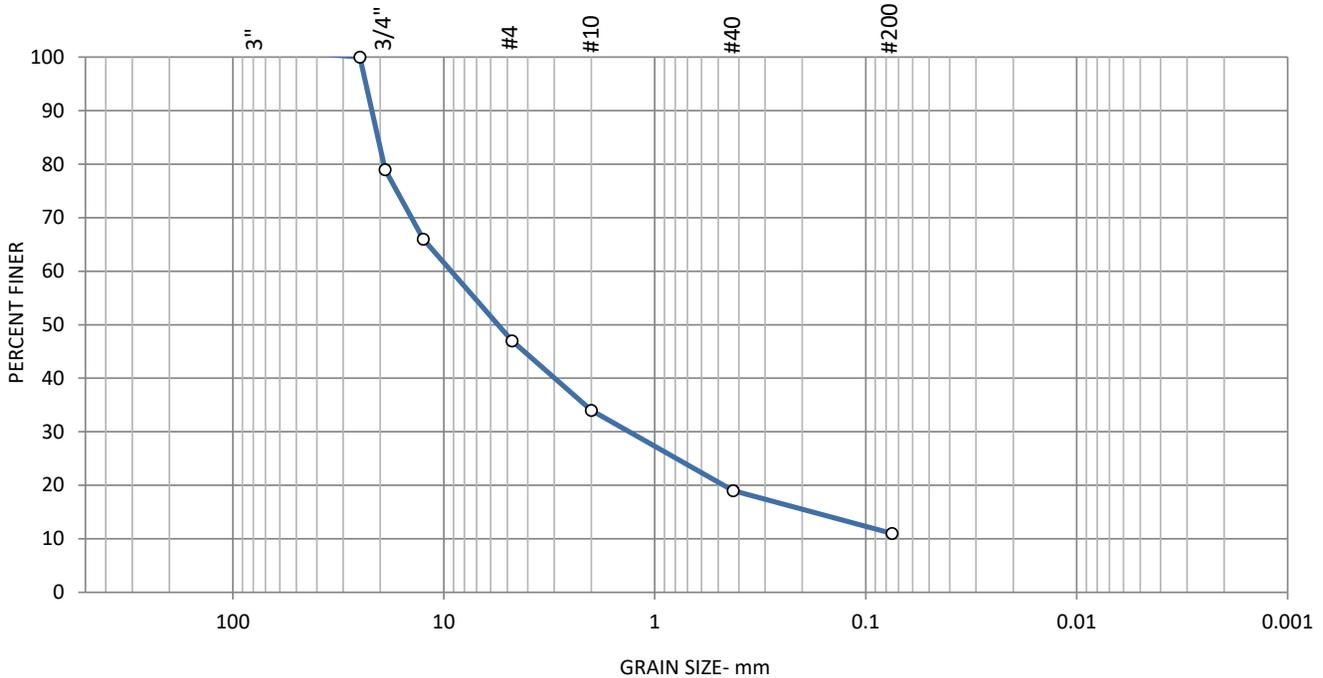
Test Date: 1/7/2026

Sample No: SD-1, S-2

Source of Sample: Boring Sample

Location: In-place

Elev./Depth (ft): 2.0-4.0



% Cobbles	% Gravel		% Sand			% Fines	
	Coarse	Fine	Coarse	Medium	Fine	Silt	Clay
	21	32	13	15	8	11	

SIEVE SIZE	PERCENT FINER	SPEC. PERCENT	OUT OF SPEC.
4"			
3"			
2"			
1"	100		
3/4"	79		
1/2"	66		
#4	47		
#10	34		
#40	19		
#200	11		

Soil Description
Brown of GRAVEL; and cmf Sand; little Silt

Moisture Content = 4.2%

Atterburg Limits
PL= LL= PI=

Coefficients
D₆₀= 9.023 D₃₀= 1.581 D₁₀=
C_u= NA C_c= NA

Classification
USCS= GP AASHTO=

Remarks

ATLANTIC TESTING LABORATORIES, LIMITED

Reviewed by: Paul Smith

Date: 1-8-2026

APPENDIX E
INFILTRATION TEST DATA SUMMARIES



**ATLANTIC
TESTING
LABORATORIES**
A PHENNA GROUP COMPANY

INFILTRATION TEST DATA SUMMARY

PROJECT: Prattsburgh Central School District

LOCATION: Prattsburgh, New York

PROJECT NO.: BD298

INFILTRATION

TEST POINT: I-1

PRESOAK DATE: 10/27/2025

TEST DATA

TEST DATE: 10/28/2025

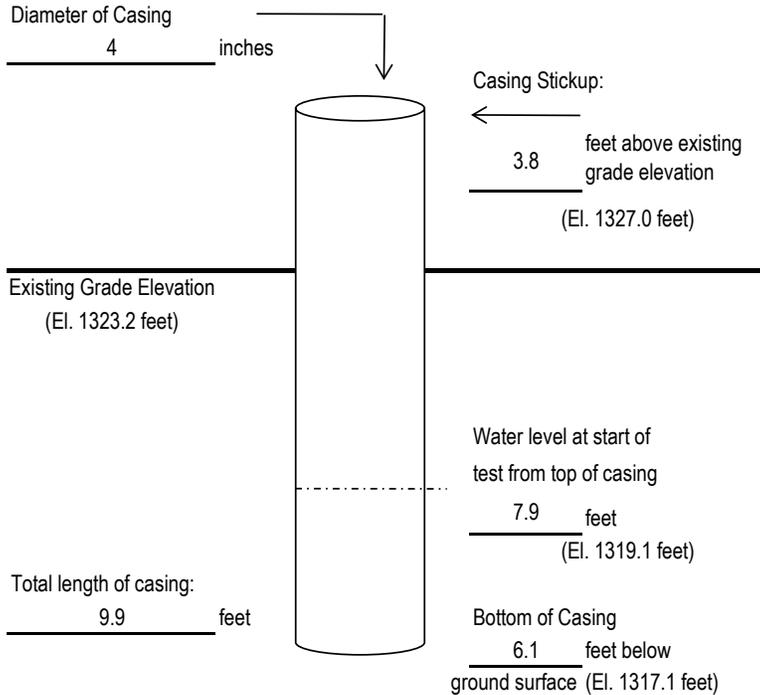
START OF TEST TIME: 7:45 AM

IS THERE PRESOAK WATER IN TEST CASING?

YES NO

IF YES, WHAT DEPTH:

N/A FEET FROM TOP OF CASING.



RUN NUMBER	START TIME (HOURS)	END TIME (HOURS)	ELAPSED TIME (MIN)	DROP IN WATER LEVEL DURING TEST RUN (FEET)	REFILLED WITH WATER, LEVEL FROM TOP OF CASING (FEET)
RUN #1	7:45	8:45	< 60	2.0	7.9
RUN #2	8:45	9:15	< 30	2.0	7.9
RUN #3	9:15	9:20	5	2.0	7.9
RUN #4	9:20	9:23	3	2.0	7.9
RUN #5	9:23	9:26	3	2.0	7.9

AVERAGE INFILTRATION RATE: 2.0 FEET PER 3.7 MIN.
 AVERAGE INFILTRATION RATE: 32 FEET PER HOUR
 AVERAGE INFILTRATION RATE: 384 INCHES PER HOUR

TESTED BY: T. Coyne



ATLANTIC TESTING LABORATORIES
A PHENNA GROUP COMPANY

INFILTRATION TEST DATA SUMMARY

PROJECT: Prattsburgh Central School District

LOCATION: Prattsburgh, New York

PROJECT NO.: BD298

INFILTRATION

TEST POINT: I-2
PRESOAK DATE: 10/27/2025

TEST DATA

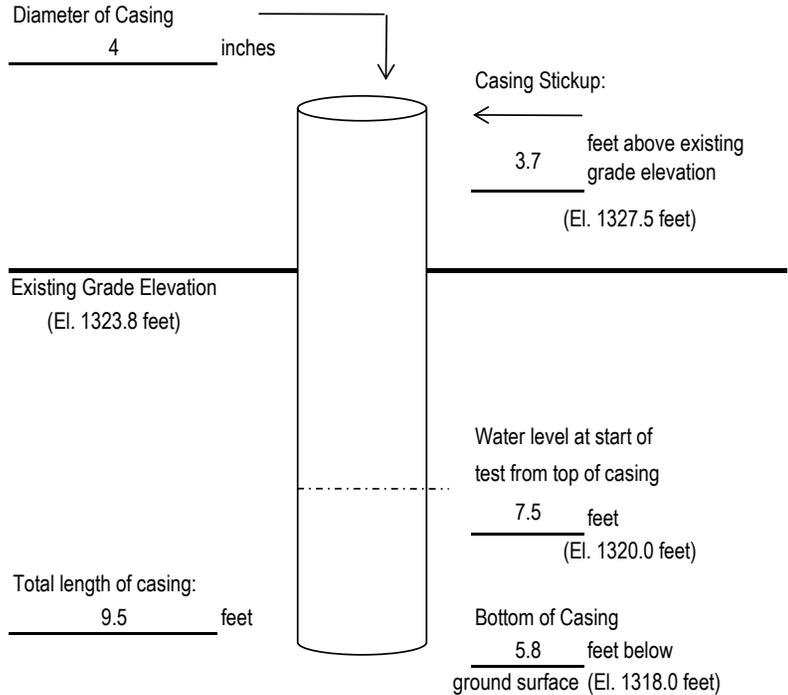
TEST DATE: 10/28/2025
START OF TEST TIME: 7:35 AM

IS THERE PRESOAK WATER IN TEST CASING?

YES NO

IF YES, WHAT DEPTH:

N/A FEET FROM TOP OF CASING.



RUN NUMBER	START TIME (HOURS)	END TIME (HOURS)	ELAPSED TIME (MIN)	DROP IN WATER LEVEL DURING TEST RUN (FEET)	REFILLED WITH WATER, LEVEL FROM TOP OF CASING (FEET)
RUN #1	7:35	8:35	< 60	2.0	7.5
RUN #2	8:35	9:05	< 30	2.0	7.5
RUN #3	9:05	9:20	15	2.0	7.5
RUN #4	9:20	9:40	20	2.0	7.5

AVERAGE INFILTRATION RATE: 2.0 FEET PER 17.5 MIN
 AVERAGE INFILTRATION RATE: 6.8 FEET PER HOUR
 AVERAGE INFILTRATION RATE: 81 INCHES PER HOUR

TESTED BY: T. Coyne



ATLANTIC TESTING LABORATORIES
A PHENNA GROUP COMPANY

INFILTRATION TEST DATA SUMMARY

PROJECT: Prattsburgh Central School District

LOCATION: Prattsburgh, New York

PROJECT NO.: BD298

INFILTRATION

TEST POINT: I-3
PRESOAK DATE: 10/27/2025

TEST DATA

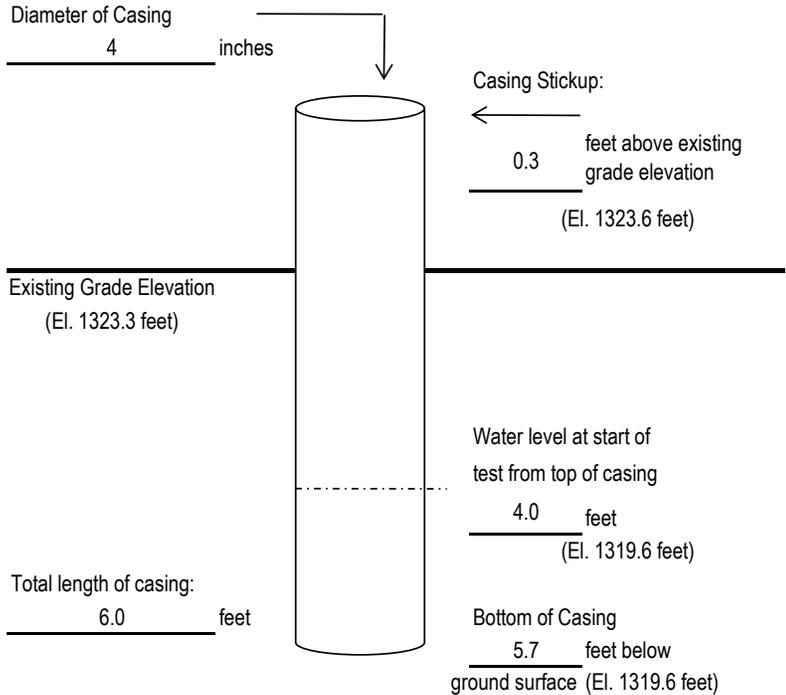
TEST DATE: 10/28/2025
START OF TEST TIME: 7:30 AM

IS THERE PRESOAK WATER IN TEST CASING?

YES NO

IF YES, WHAT DEPTH:

N/A FEET FROM TOP OF CASING.



RUN NUMBER	START TIME (HOURS)	END TIME (HOURS)	ELAPSED TIME (MIN)	DROP IN WATER LEVEL DURING TEST RUN (FEET)	REFILLED WITH WATER, LEVEL FROM TOP OF CASING (FEET)
RUN #1	7:30	8:30	60	1.2	4.0
RUN #2	8:30	9:30	60	1.5	4.0
RUN #3	9:30	10:30	60	1.1	4.0
RUN #4	10:30	11:30	60	1.1	4.0

AVERAGE INFILTRATION RATE: 1.2 FEET PER HOUR
AVERAGE INFILTRATION RATE: 14.4 INCHES PER HOUR

TESTED BY: J. Porter



ATLANTIC TESTING LABORATORIES
A PHENNA GROUP COMPANY

INFILTRATION TEST DATA SUMMARY

PROJECT: Prattsburgh Central School District

LOCATION: Prattsburgh, New York

PROJECT NO.: BD298

INFILTRATION

TEST POINT: I-4
PRESOAK DATE: 10/27/2025

TEST DATA

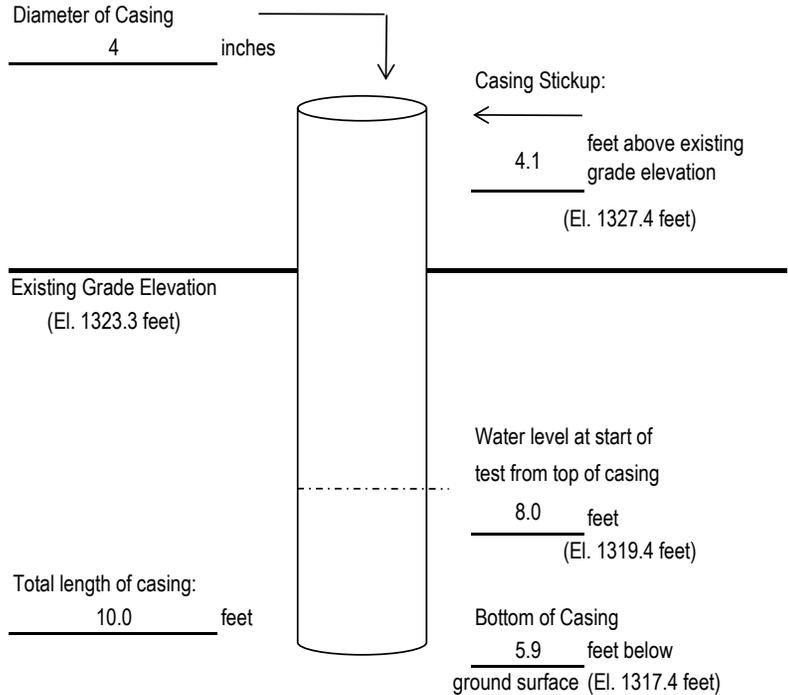
TEST DATE: 10/28/2025
START OF TEST TIME: 7:35 AM

IS THERE PRESOAK WATER IN TEST CASING?

YES NO

IF YES, WHAT DEPTH:

N/A FEET FROM TOP OF CASING.



RUN NUMBER	START TIME (HOURS)	END TIME (HOURS)	ELAPSED TIME (MIN)	DROP IN WATER LEVEL DURING TEST RUN (FEET)	REFILLED WITH WATER, LEVEL FROM TOP OF CASING (FEET)
RUN #1	7:35	8:35	60	1.8	8.0
RUN #2	8:35	9:35	60	1.9	8.0
RUN #3	9:35	10:35	60	1.9	8.0
RUN #4	10:35	11:35	60	1.8	8.0

AVERAGE INFILTRATION RATE: 1.85 FEET PER HOUR
AVERAGE INFILTRATION RATE: 22.2 INCHES PER HOUR

TESTED BY: J. Porter



ATLANTIC TESTING LABORATORIES
A PHENNA GROUP COMPANY

INFILTRATION TEST DATA SUMMARY

PROJECT: Prattsburgh Central School District

LOCATION: Prattsburgh, New York

PROJECT NO.: BD298

INFILTRATION

TEST POINT: I-5
PRESOAK DATE: 10/27/2025

TEST DATA

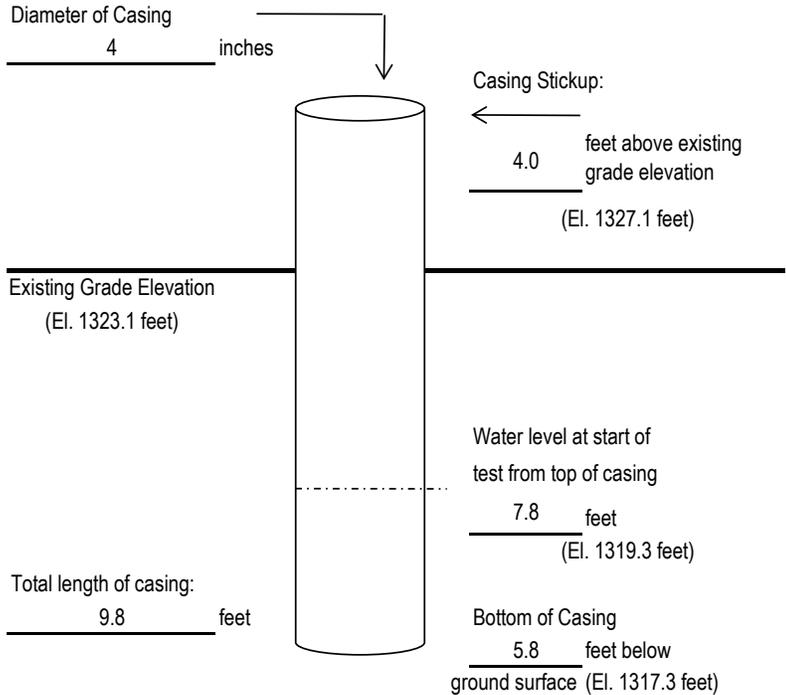
TEST DATE: 10/28/2025
START OF TEST TIME: 7:40 AM

IS THERE PRESOAK WATER IN TEST CASING?

YES NO

IF YES, WHAT DEPTH:

N/A FEET FROM TOP OF CASING.



RUN NUMBER	START TIME (HOURS)	END TIME (HOURS)	ELAPSED TIME (MIN)	DROP IN WATER LEVEL DURING TEST RUN (FEET)	REFILLED WITH WATER, LEVEL FROM TOP OF CASING (FEET)
RUN #1	7:40	8:40	< 60	2.0	7.8
RUN #2	8:40	9:10	30	2.0	7.8
RUN #3	9:10	9:25	15	2.0	7.8
RUN #4	9:25	9:55	30	2.0	7.8

AVERAGE INFILTRATION RATE: 2.0 FEET PER 25 MIN
 AVERAGE INFILTRATION RATE: 4.8 FEET PER HOUR
 AVERAGE INFILTRATION RATE: 57 INCHES PER HOUR

TESTED BY: T. Coyne



ATLANTIC TESTING LABORATORIES
A PHENNA GROUP COMPANY

INFILTRATION TEST DATA SUMMARY

PROJECT: Prattsburgh Central School District

LOCATION: Prattsburgh, New York

PROJECT NO.: BD298

INFILTRATION

TEST POINT: I-6
PRESOAK DATE: 10/27/2025

TEST DATA

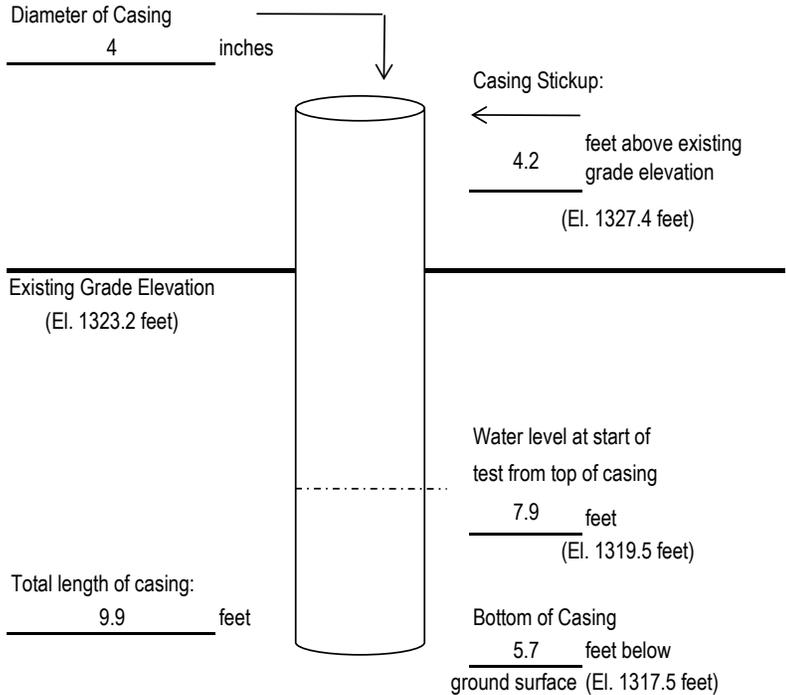
TEST DATE: 10/28/2025
START OF TEST TIME: 7:40 AM

IS THERE PRESOAK WATER IN TEST CASING?

YES NO

IF YES, WHAT DEPTH:

N/A FEET FROM TOP OF CASING.



RUN NUMBER	START TIME (HOURS)	END TIME (HOURS)	ELAPSED TIME (MIN)	DROP IN WATER LEVEL DURING TEST RUN (FEET)	REFILLED WITH WATER, LEVEL FROM TOP OF CASING (FEET)
RUN #1	7:40	8:40	60	2.0	7.9
RUN #2	8:40	9:10	30	2.0	7.9
RUN #3	9:10	9:35	25	2.0	7.9
RUN #4	9:35	10:35	60	1.8	7.9

APPROX. INFILTRATION RATE: 1.8 FEET PER HOUR
APPROX. INFILTRATION RATE: 21.6 INCHES PER HOUR

TESTED BY: J. Porter



ATLANTIC TESTING LABORATORIES
A PHENNA GROUP COMPANY

INFILTRATION TEST DATA SUMMARY

PROJECT: Prattsburgh Central School District

LOCATION: Prattsburgh, New York

PROJECT NO.: BD298

INFILTRATION

TEST POINT: I-7
PRESOAK DATE: 10/27/2025

TEST DATA

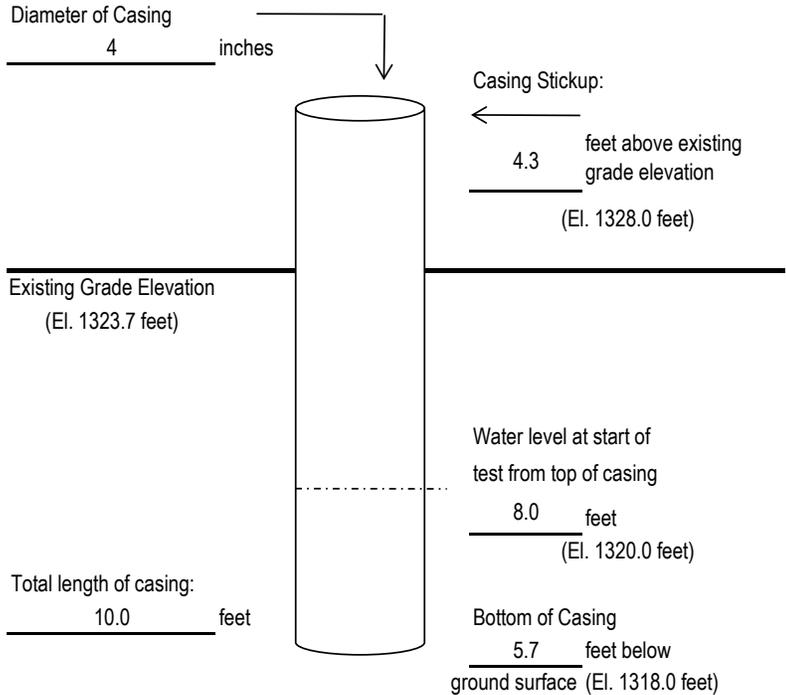
TEST DATE: 10/28/2025
START OF TEST TIME: 7:25 AM

IS THERE PRESOAK WATER IN TEST CASING?

YES NO

IF YES, WHAT DEPTH:

N/A FEET FROM TOP OF CASING.



RUN NUMBER	START TIME (HOURS)	END TIME (HOURS)	ELAPSED TIME (MIN)	DROP IN WATER LEVEL DURING TEST RUN (FEET)	REFILLED WITH WATER, LEVEL FROM TOP OF CASING (FEET)
RUN #1	7:25	8:25	60	1.0	8.0
RUN #2	8:25	9:25	60	0.7	8.0
RUN #3	9:25	10:25	60	0.6	8.0
RUN #4	10:25	11:25	60	0.5	8.0

APPROX. INFILTRATION RATE: 0.5 FEET PER HOUR
APPROX. INFILTRATION RATE: 6 INCHES PER HOUR

TESTED BY: J. Porter

SECTION 22 10 05
PLUMBING PIPING AND SPECIALTIES

PART 1 GENERAL

1.1 SECTION INCLUDES

- A. Sanitary waste piping, buried within 5 feet of building.
- B. Sanitary waste piping, above grade.
- C. Domestic water piping, buried within 5 feet of building.
- D. Domestic water piping, above grade.

1.2 RELATED REQUIREMENTS

- A. Section 07 84 00 - Firestopping.
- B. Section 22 05 53 - Identification for Plumbing Piping and Equipment.
- C. Section 22 07 19 - Plumbing Piping Insulation.
- D. Section 31 23 16 - Excavation.
- E. Section 31 23 23 - Fill.

1.3 REFERENCE STANDARDS

- A. ANSI Z21.22 - American National Standard for Relief Valves for Hot Water Supply Systems; 2015 (Reaffirmed 2020).
- B. ANSI Z223.1 - National Fuel Gas Code; 2024.
- C. ASME A112.6.4 - Roof, Deck, and Balcony Drains; 2022.
- D. ASME B16.3 - Malleable Iron Threaded Fittings: Classes 150 and 300; 2021.
- E. ASME B16.18 - Cast Copper Alloy Solder Joint Pressure Fittings; 2021.
- F. ASME B16.22 - Wrought Copper and Copper Alloy Solder-Joint Pressure Fittings; 2021.
- G. ASME B16.23 - Cast Copper Alloy Solder Joint Drainage Fittings: DWV; 2021.
- H. ASME B16.26 - Cast Copper Alloy Fittings for Flared Copper Tubes; 2024.
- I. ASME B16.29 - Wrought Copper and Wrought Copper Alloy Solder-Joint Drainage Fittings—DWV; 2022.
- J. ASME B31.9 - Building Services Piping; 2020.
- K. ASME BPVC-IV - Boiler and Pressure Vessel Code, Section IV - Rules for Construction of Heating Boilers; 2023, with Errata (2024).
- L. ASME BPVC-IX - Boiler and Pressure Vessel Code, Section IX - Qualification Standard for Welding, Brazing, and Fusing Procedures; Welders; Brazers; and Welding, Brazing, and Fusing Operators; 2023, with Errata (2024).
- M. ASSE 1003 - Water Pressure Reducing Valves for Potable Water Distribution Systems; 2023.

- N. ASTM A53/A53M - Standard Specification for Pipe, Steel, Black and Hot-Dipped, Zinc-Coated, Welded and Seamless; 2022.
- O. ASTM A74 - Standard Specification for Cast Iron Soil Pipe and Fittings; 2021.
- P. ASTM A234/A234M - Standard Specification for Piping Fittings of Wrought Carbon Steel and Alloy Steel for Moderate and High Temperature Service; 2023a.
- Q. ASTM B32 - Standard Specification for Solder Metal; 2020.
- R. ASTM B42 - Standard Specification for Seamless Copper Pipe, Standard Sizes; 2020.
- S. ASTM B68/B68M - Standard Specification for Seamless Copper Tube, Bright Annealed; 2019.
- T. ASTM B75/B75M - Standard Specification for Seamless Copper Tube; 2020.
- U. ASTM B88 - Standard Specification for Seamless Copper Water Tube; 2022.
- V. ASTM B88M - Standard Specification for Seamless Copper Water Tube (Metric); 2020.
- W. ASTM B306 - Standard Specification for Copper Drainage Tube (DWV); 2020.
- X. ASTM B813 - Standard Specification for Liquid and Paste Fluxes for Soldering of Copper and Copper Alloy Tube; 2016.
- Y. ASTM B828 - Standard Practice for Making Capillary Joints by Soldering of Copper and Copper Alloy Tube and Fittings; 2023.
- Z. ASTM C564 - Standard Specification for Rubber Gaskets for Cast Iron Soil Pipe and Fittings; 2020a.
- AA. ASTM D2239 - Standard Specification for Polyethylene (PE) Plastic Pipe (SIDR-PR) Based on Controlled Inside Diameter; 2022.
- BB. ASTM D2564 - Standard Specification for Solvent Cements for Poly(Vinyl Chloride) (PVC) Plastic Piping Systems; 2020.
- CC. ASTM D2609 - Standard Specification for Plastic Insert Fittings for Polyethylene (PE) Plastic Pipe; 2021.
- DD. ASTM D2665 - Standard Specification for Poly(Vinyl Chloride) (PVC) Plastic Drain, Waste, and Vent Pipe and Fittings; 2020.
- EE. ASTM D2855 - Standard Practice for the Two-Step (Primer and Solvent Cement) Method of Joining Poly (Vinyl Chloride) (PVC) or Chlorinated Poly (Vinyl Chloride) (CPVC) Pipe and Piping Components with Tapered Sockets; 2020.
- FF. ASTM D3034 - Standard Specification for Type PSM Poly(Vinyl Chloride) (PVC) Sewer Pipe and Fittings; 2023.
- GG. ASTM F477 - Standard Specification for Elastomeric Seals (Gaskets) for Joining Plastic Pipe; 2014 (Reapproved 2021).
- HH. ASTM F679 - Standard Specification for Poly(Vinyl Chloride) (PVC) Large-Diameter Plastic Gravity Sewer Pipe and Fittings; 2021.
- II. AWWA C105/A21.5 - Polyethylene Encasement for Ductile-Iron Pipe Systems; 2018.
- JJ. AWWA C111/A21.11 - Rubber-Gasket Joints for Ductile-Iron Pressure Pipe and Fittings; 2023.
- KK. AWWA C151/A21.51 - Ductile-Iron Pipe, Centrifugally Cast; 2023.
- LL. AWWA C550 - Protective Interior Coatings for Valves and Hydrants; 2024.
- MM. AWWA C651 - Disinfecting Water Mains; 2023.

- NN. AWWA C901 - Polyethylene (PE) Pressure Pipe and Tubing, 3/4 In. (19 mm) Through 3 In. (76 mm), for Water Service; 2020.
- OO. CISPI 301 - Standard Specification for Hubless Cast Iron Soil Pipe and Fittings for Sanitary and Storm Drain, Waste, and Vent Piping Applications; 2021.
- PP. CISPI 310 - Specification for Coupling for Use in Connection with Hubless Cast Iron Soil Pipe and Fittings for Sanitary and Storm Drain, Waste, and Vent Piping Applications; 2020.
- QQ. ICC (IFGC) - International Fuel Gas Code; Most Recent Edition Adopted by Authority Having Jurisdiction, Including All Applicable Amendments and Supplements.
- RR. ICC-ES AC01 - Acceptance Criteria for Expansion Anchors in Masonry Elements; 2018, with Editorial Revision (2020).
- SS. ICC-ES AC106 - Acceptance Criteria for Predrilled Fasteners (Screw Anchors) in Masonry; 2018, with Editorial Revision (2020).
- TT. ICC-ES AC193 - Acceptance Criteria for Mechanical Anchors in Concrete Elements; 2017, with Editorial Revision (2020).
- UU. ICC-ES AC308 - Acceptance Criteria for Post-Installed Adhesive Anchors in Concrete Elements; 2023.
- VV. MSS SP-58 - Pipe Hangers and Supports - Materials, Design, Manufacture, Selection, Application, and Installation; 2018, with Amendment (2019).
- WW. MSS SP-71 - Gray Iron Swing Check Valves, Flanged and Threaded Ends; 2018.
- XX. MSS SP-78 - Gray Iron Plug Valves, Flanged and Threaded Ends; 2011.
- YY. MSS SP-80 - Bronze Gate, Globe, Angle, and Check Valves; 2019.
- ZZ. MSS SP-85 - Gray Iron Globe and Angle Valves, Flanged and Threaded Ends; 2011.
- AAA. MSS SP-110 - Ball Valves Threaded, Socket-Welding, Solder Joint, Grooved and Flared Ends; 2010, with Errata .
- BBB. NEMA EN 10250 - Enclosures for Electrical Equipment (1000 Volts Maximum); 2024.
- CCC. NSF 61 - Drinking Water System Components - Health Effects; 2024.
- DDD. NSF 372 - Drinking Water System Components - Lead Content; 2024.

1.4 SUBMITTALS

- A. See Section 01 30 00 - Administrative Requirements for submittal procedures.
- B. Product Data: Provide data on pipe materials, pipe fittings, valves, hangers, supports and accessories. Provide manufacturers catalog information. Indicate valve data and ratings.
- C. Project Record Documents: Record actual locations of valves.
- D. Hangers and Supports: Submit manufacturers catalog information including load capacity.
- E. Maintenance Materials: Furnish the following for Owner's use in maintenance of project.
 - 1. See Section 01 60 00 - Product Requirements for additional provisions.
 - 2. Valve Repacking Kits: One for each type and size of valve.

1.5 QUALITY ASSURANCE

- A. Perform work in accordance with applicable codes.
- B. Perform Work in accordance with standards of the State of New York.

- C. Valves: Manufacturer's name and pressure rating marked on valve body.
- D. Welding Materials and Procedures: Comply with ASME BPVC-IX and applicable state labor regulations.
- E. Welder Qualifications: Certified in accordance with ASME BPVC-IX.
- F. Identify pipe with marking including size, ASTM material classification, ASTM specification, potable water certification, water pressure rating.

1.6 REGULATORY REQUIREMENTS

- A. Conform to applicable code for installation of backflow prevention devices.
- B. Provide certificate of compliance from authority having jurisdiction indicating approval of installation of backflow prevention devices.

1.7 DELIVERY, STORAGE, AND HANDLING

- A. Accept valves on site in shipping containers with labeling in place. Inspect for damage.
- B. Protect piping systems from entry of foreign materials by temporary covers, completing sections of the work, and isolating parts of completed system.

1.8 FIELD CONDITIONS

- A. Do not install underground piping when bedding is wet or frozen.

PART 2 PRODUCTS

2.1 GENERAL REQUIREMENTS

- A. Potable Water Supply Systems: Provide piping, pipe fittings, and solder and flux (if used), that comply with NSF 61 and NSF 372 for maximum lead content; label pipe and fittings.

2.2 SANITARY WASTE PIPING, BURIED WITHIN 5 FEET OF BUILDING

- A. Cast Iron Pipe: ASTM A74 extra heavy weight.
 - 1. Fittings: Cast iron.
 - 2. Joints: Hub-and-spigot, CISPI HSN compression type with ASTM C564 neoprene gaskets or lead and oakum.
- B. PVC Pipe: ASTM D2665 or ASTM D3034.
 - 1. Fittings: PVC.
 - 2. Joints: Solvent welded, with ASTM D2564 solvent cement.

2.3 SANITARY WASTE PIPING, ABOVE GRADE

- A. Cast Iron Pipe: CISPI 301, hubless, service weight.
 - 1. Fittings: Cast iron.
 - 2. Joints: CISPI 310, neoprene gaskets and stainless steel clamp-and-shield assemblies.
- B. Copper Tube: ASTM B306, DWV, Type L.
 - 1. Fittings: ASME B16.29, wrought copper, or ASME B16.23, sovent.
 - 2. Joints: Solder, lead free, ASTM B32, 95-5 tin-antimony, or tin and silver, with melting range 430 to 535 degrees F.

- C. PVC Pipe - (Not For Use in Return Air Plenums or Exposed in Places of Assembly.): ASTM D2665.
 - 1. Fittings: PVC.
 - 2. Joints: Solvent welded, with ASTM D2564 solvent cement.

2.4 DOMESTIC WATER PIPING, BURIED WITHIN 5 FEET OF BUILDING

- A. Copper Pipe: ASTM B42, hard drawn, 2-1/2 inches and smaller.
 - 1. Fittings: ASME B16.18, cast copper alloy or ASME B16.22 wrought copper and bronze.
 - 2. Joints: ASTM B32, alloy Sn95 solder.
- B. Ductile Iron Pipe: AWWA C151/A21.51, 3 inches and larger.
 - 1. Fittings: AWWA C110, ductile iron, standard thickness. Cement Mortar lining in conformance with AWWA C-104.
 - 2. Joints: AWWA C111/A21.11, styrene butadiene rubber (SBR) or vulcanized SBR gasket with 3/4 inch diameter rods.
 - 3. Jackets: AWWA C105/A21.5 polyethylene jacket.
- C. PEX Pipe: Polyethylene cross-linked for Potable water (non-oxygen barrier). Color coded: Blue for cold domestic water and Red for hot domestic water. Complies with ASTM F876, F877, F1807, F2159, 2023, CSA B137.5.
 - 1. Fittings: PEX designed for use with Potable water piping.

2.5 DOMESTIC WATER PIPING, ABOVE GRADE

- A. Copper Tubing for pipe 2 1/2 inches and smaller: ASTM B 88 (ASTM B 88M), Type L (B), Drawn (H)
 - 1. Fittings: ASME B16.18, cast copper alloy or ASME B16.22, wrought copper and bronze.
 - 2. Joints: ASTM B 32, alloy Sn95 solder. Lead free.
- B. Copper Tubing for pipe 3 inches and larger: ASTM B88, Type L (B), hard drawn, rolled grooved ends
 - 1. Fittings: ASTM B584 bronze sand castings, grooved ends.
 - 2. Joints: Grooved mechanical couplings meeting ASTM F1476.
 - a. Housing Clamps: ASTM A395/A395M and ASTM A536 ductile iron, enamel coated, compatible with copper tubing sizes, to engage and lock designed to permit some angular deflection, contraction, and expansion.
 - b. Gasket: Elastomer composition for operating temperature range from -30 degrees F to 180 degrees F.
 - c. Accessories: Stainless steel bolts, nuts, and washers.
 - 3. Mechanically pressed fitting are allowed for this application.

2.6 STORM DRAINAGE PIPING, BURIED WITHIN 5 FEET OF BUILDING

- A. Cast Iron Pipe: ASTM A74 service weight, bell and spigot ends.
 - 1. Fittings: Cast iron, ASTM A74.
 - 2. Joint Seals: ASTM C 564 neoprene gaskets.
- B. PVC Pipe: ASTM D2665 or ASTM D3034, polyvinyl chloride (PVC) material.
 - 1. Fittings: PVC, ASTM D2665 or ASTM D3034.
 - 2. Joints: Solvent welded, with ASTM D2564 solvent cement.
- C. PVC Pipe: ASTM D2665, ASTM D3034, or ASTM F679, polyvinyl chloride (PVC) material.
 - 1. Fittings: PVC, ASTM D2665, ASTM D3034, or ASTM F679.
 - 2. Joints: Push-on, using ASTM F477 elastomeric gaskets.

2.7 STORM DRAINAGE PIPING, ABOVE GRADE

- A. Cast Iron Pipe: CISPI 301, hubless, service weight.
 - 1. Fittings: Cast iron, CISPI 301.
 - 2. Joints: Neoprene gaskets and stainless steel clamp-and-shield assemblies.

- B. PVC Pipe (Not For Use in Return Air Plenums or Exposed in Places of Assembly.): ASTM D2665 or ASTM D3034.
 - 1. Fittings: ASTM D2665 or ASTM D3034, PVC.
 - 2. Joints: Solvent welded, with ASTM D2564 solvent cement.

2.8 ROOF DRAINS

- A. Retrofit Roof Drain:
 - 1. Assembly: ASME A112.6.4.
 - 2. Body: #16 gage Type 304 stainless steel.
 - 3. Strainer: Removable aluminum dome.
 - 4. Accessories: Coordinate with roofing type:
 - a. Membrane flange and membrane clamp with integral gravel stop.
 - b. Roof sump receiver.
 - c. Waterproofing flange.
 - d. Controlled flow weir.
 - e. Leveling frame.
 - f. Perforated or slotted ballast guard extension for inverted roof.
 - g. Perforated stainless steel ballast guard extension.
 - h. Dura-Coated cast iron clamp collar.
 - i. Stainless steel hardware.
 - j. Neoprene gasket.

2.9 PIPE FLANGES, UNIONS, AND COUPLINGS

- A. Unions for Pipe Sizes 2 inches and Under:
 - 1. Copper Tube and Pipe: Class 150 bronze unions with soldered joints.
 - 2. PVC Piping: PVC
 - 3. CPVC Piping: PVC
- B. Flanges for Pipe Size Over 2 inches:
 - 1. Copper Tube and Pipe: Class 150 slip-on bronze flanges; preformed neoprene gaskets.
 - 2. Dielectric Connections: Union with galvanized or plated steel threaded end, copper solder end, water impervious isolation barrier.
 - 3. PVC Piping: PVC.
 - 4. CPVC Piping: PVC.
 - 5. Gaskets: 1/16 inch thick preformed neoprene gaskets.

2.10 PIPE HANGERS AND SUPPORTS

- A. Provide hangers and supports that comply with MSS SP-58.
 - 1. If type of hanger or support for a particular situation is not indicated, select appropriate type using MSS SP-58 recommendations.
 - 2. Overhead Supports: Individual steel rod hangers attached to structure or to trapeze hangers.
 - 3. Trapeze Hangers: Welded steel channel frames attached to structure.
 - 4. Vertical Pipe Support: Steel riser clamp.
- B. Plumbing Piping - Drain, Waste, and Vent:
 - 1. Conform to ASME B31.9.
 - 2. Hangers for Pipe Sizes 1/2 to 1-1/2 inch: Malleable iron, adjustable swivel, split ring.
 - 3. Hangers for Pipe Sizes 2 inch and Over: Carbon steel, adjustable, clevis.
 - 4. Multiple or Trapeze Hangers: Steel channels with welded spacers and hanger rods.
 - 5. Wall Support for Pipe Sizes to 3 inch: Cast iron hook.
 - 6. Wall Support for Pipe Sizes 4 inch and Over: Welded steel bracket and wrought steel clamp.
 - 7. Vertical Support: Steel riser clamp.
 - 8. Floor Support: Cast iron adjustable pipe saddle, lock nut, nipple, floor flange, and concrete pier or steel support.
 - 9. Copper Pipe Support: Carbon steel ring, adjustable, copper plated.

- C. Plumbing Piping - Water:
 - 1. Conform to ASME B31.9.
 - 2. Hangers for Pipe Sizes 1/2 to 1-1/2 inch: Malleable iron, adjustable swivel, split ring.
 - 3. Hangers for Cold Pipe Sizes 2 inch and Over: Carbon steel, adjustable, clevis.
 - 4. Hangers for Hot Pipe Sizes 2 to 4 inch: Carbon steel, adjustable, clevis.
 - 5. Hangers for Hot Pipe Sizes 6 inch and Larger: Adjustable steel yoke, cast iron pipe roll, double hanger.
 - 6. Multiple or Trapeze Hangers: Steel channels with welded supports or spacers and hanger rods.
 - 7. Multiple or Trapeze Hangers for Hot Pipe Sizes 6 Inches and Over: Steel channels with welded supports or spacers and hanger rods, cast iron roll.
 - 8. Wall Support for Pipe Sizes Up to 3 inch: Cast iron hook.
 - 9. Wall Support for Pipe Sizes 4 inch and Larger: Welded steel bracket and wrought steel clamp.
 - 10. Wall Support for Hot Pipe Sizes 6 inch and Larger: Welded steel bracket and wrought steel clamp with adjustable steel yoke and cast iron pipe roll.
 - 11. Vertical Support: Steel riser clamp.
 - 12. Floor Support for Cold Pipe: Cast iron adjustable pipe saddle, lock nut, nipple, floor flange, and concrete pier or steel support.
 - 13. Floor Support for Hot Pipe Sizes to 4 inch: Cast iron adjustable pipe saddle, locknut, nipple, floor flange, and concrete pier or steel support.
 - 14. Floor Support for Hot Pipe Sizes 6 inch and Larger: Adjustable cast iron pipe roll and stand, steel screws, and concrete pier or steel support.
 - 15. Copper Pipe Support: Carbon steel ring, adjustable, copper plated.
- D. Hanger Fasteners: Attach hangers to structure using appropriate fasteners, as follows:
 - 1. Concrete Wedge Expansion Anchors: Comply with ICC-ES AC193.
 - 2. Masonry Wedge Expansion Anchors: Comply with ICC-ES AC01.
 - 3. Concrete Screw Type Anchors: Comply with ICC-ES AC193.
 - 4. Masonry Screw Type Anchors: Comply with ICC-ES AC106.
 - 5. Concrete Adhesive Type Anchors: Comply with ICC-ES AC308.
- E. INSERTS
 - 1. Inserts: Malleable iron case of galvanized steel shell and expander plug for threaded connection with lateral adjustment, top slot for reinforcing rods, lugs for attaching to forms; size inserts to suit threaded hanger rods.
- F. FLASHING
 - 1. Metal Flashing: 26 gage thick galvanized steel.
 - 2. Metal Counterflashing: 22 gage thick galvanized steel.
 - 3. Lead Flashing:
 - a. Waterproofing: 5 lb./sq. ft sheet lead.
 - b. Soundproofing: 1 lb./sq. ft sheet lead.
 - 4. Flexible Flashing: 47 mil thick sheet compatible with roofing.
 - 5. Caps: Steel, 22 gage minimum; 16 gage at fire resistant elements.
- G. SLEEVES
 - 1. Sleeves for Pipes through Non-fire Rated Floors: 18 gage thick galvanized steel.
 - 2. Sleeves for Pipes through Non-fire Rated Beams, Walls, Footings, and Potentially Wet Floors: Steel pipe or 18 gage thick galvanized steel.
 - 3. Sealant: See Section 07 92 00 - Joint Sealants.
- H. MECHANICAL SLEEVE SEALS
 - 1. Product Description: Modular mechanical type, consisting of interlocking synthetic rubber links shaped to continuously fill annular space between object and sleeve, connected with bolts and pressure plates causing rubber sealing elements to expand when tightened, providing watertight seal and electrical insulation.
- I. FORMED STEEL CHANNEL
 - 1. Product Description: Galvanized 12 gage thick steel. With holes 1-1/2 inches on center.

J. FIRESTOPPING

1. See Section 07 84 00 - Firestopping for firestopping requirements.

2.11 PIPE SLEEVE-SEAL SYSTEMS

A. Modular Mechanical Seals:

1. Elastomer-based interlocking links continuously fill annular space between pipe and wall-sleeve, wall or casing opening.
2. Watertight seal between pipe and wall-sleeve, wall or casing opening.
3. Size and select seal component materials in accordance to service requirements.
4. Glass reinforced plastic pressure end plates.

2.12 BALL VALVES

A. Manufacturers:

1. Substitutions: See Section 01 60 00 - Product Requirements.

- B. Construction, 4 Inches and Smaller: MSS SP-110, Class 150, 400 psi CWP, bronze body, chrome plated brass ball, regular port, teflon seats and stuffing box ring, blow-out proof stem, lever handle, solder or threaded ends with union. Lead free.

2.13 PLUG VALVES

- A. Construction 2-1/2 Inches and Larger: MSS SP-78, 175 psi CWP, cast iron body and plug, pressure lubricated, teflon or Buna N packing, flanged or grooved ends. Provide lever operator with set screw.

2.14 HORIZONTAL SWING CHECK VALVES

A. Up to 2 Inches:

1. MSS SP-80, 150, bronze body and cap, bronze swing disc with rubber seat, solder or threaded ends. Lead free.

2.15 SPRING LOADED CHECK VALVES

A. Up to 2 inches:

1. MSS SP 80, Class 250, bronze body, in-line spring lift check, silent closing, Buna-N disc, integral seat, threaded ends. Lead free.

B. 2-1/2 inches and Larger:

- C. MSS SP 71, Class 125, wafer style, cast iron body, bronze seat, center guided bronze disc, stainless steel spring and screws, flanged ends.

2.16 PRESSURE REDUCING VALVES

A. 2 inch and Smaller:

1. MSS SP-80, bronze body, stainless steel and thermoplastic internal parts, fabric reinforced diaphragm, strainer, threaded single union ends. Lead free.
2. Pressure Reducing Pilot-Operator:
 - a. Operating Range: 5 to 50 psi.
 - b. Connected into brass or bronze pilot piping and fittings.
 - c. Fixed flow restrictor, pressure gauges, and isolation valves.
3. Provide pressure gauge onto inlet side and outlet side piping of valve.

B. 2 inch and Larger:

1. ASSE 1003, cast iron body with interior lining complying with AWWA C550, bronze fitted, elastomeric diaphragm and seat disc, flanged.
2. Pressure Reducing Pilot-Operator:

- a. Operating Range: 5 to 50 psi.
 - b. Connected into brass or bronze pilot piping and fittings.
 - c. Fixed flow restrictor, strainer, pressure gauges, and isolation valves.
3. Provide pressure gauge onto inlet side and outlet side piping of valve.

2.17 CONTROL AND SERVICE VALVES

- A. Flow Control Valves:
 1. Size: Required to match service flow 15 gpm, Class 150 flange ends.
 2. AWWA C550 compliant cast iron globe valve, elastomeric diaphragm, seat disc and epoxy coated both internally and externally.
 3. Differential Pressure Based Rate-of-Flow Pilot-Operator:
 - a. Operating Range: 25 to 50 psi.
 - b. Connected into brass, bronze, or copper pilot piping and fittings.
 - c. Precision fixed-flow restrictor, strainer, pressure gauges, and isolation valves.
- B. Electronic Control Valves:
 1. Size: 2 to 20 inch, Class 150 flange ends.
 2. AWWA C550 compliant, single chamber, normally open cast iron globe valve, elastomeric diaphragm, seat disc and epoxy coated both internally and externally.
 3. _____ 2-Position Control:
 - a. Control Function: UV intensity meter alarm signal solenoid to close valve.
 - b. Connected into brass or ductile iron pilot piping and fittings.
 - c. NEMA EN 10250 Type 4X, 120 VAC, 50/60 Hz pilot connected solenoid valve.
 - d. Watertight high and low speed adjusting valves, strainer, and isolation valves.

2.18 PRESSURE GAUGES

- A. Gauge: ASME B40.1, UL 393 with bourdon tube, rotary brass movement, brass socket, front calibration adjustment, black scale on white background.
 1. Case: Steel
 2. Bourdon Tube: Type 316 stainless steel.
 3. Dial Size: 3-1/2 inch diameter.
 4. Mid-Scale Accuracy: One percent.
 5. Scale: Psi.

2.19 PRESSURE GAUGE TAPS

- A. Needle Valve: Brass, 1/4 inch NPT for minimum 300 psi.
- B. Ball Valve: Brass, 1/4 inch NPT for 250 psi.
- C. Pulsation Damper: Pressure snubber, brass with 1/4 inch NPT connections.

2.20 STEM TYPE THERMOMETERS

- A. Thermometer: ASTM E1, adjustable angle, red appearing indicator, lens front tube, cast aluminum case with enamel finish, cast aluminum adjustable joint with positive locking device.
 1. Size: 9 inch scale.
 2. Window: Clear Lexan.
 3. Stem: Brass, 3/4 inch NPT, 3-1/2 inch long.
 4. Accuracy: 2 percent.
 5. Calibration: Degrees F.
 6. Indicator shall be non-mercury.

2.21 WATER METER

- A. Provide Lead Free water meter and remote reader as recommended by water service provider. Meter to register flow in Gallons. Plumbing Contractor to install meter and reader. Install meter in accordance with AWWA M6, with isolating valves on inlet and outlet.
- B. Obtain meter from water service provider. If not possible, obtain approval of proposed meter from Director of Utility of Water Service Provider prior to water meter purchase.

2.22 HYDRANTS

- A. Wall Hydrant, HYD-1: Lead-Free, interior use, self-draining type with lockable recessed box hose thread spout, locks shield and removable key, and integral vacuum breaker.
- B. Wall Hydrant, HYD-2: Lead-Free, non-freeze, self-draining type with lockable recessed box hose thread spout, locks shield and removable key, and integral vacuum breaker.

2.23 BACKFLOW PREVENTERS

- A. Double Check Valve Assemblies, DCV-1:
 - 1. Comply with ASSE 1012. Lead Free.
 - 2. Bronze body with corrosion resistant internal parts and stainless steel springs.
 - 3. Two independently operating check valves with intermediate atmospheric vent.
 - 4. Provide testing by State certified backflow prevention device tester, and document of Certification.

2.24 WATER HAMMER ARRESTOR -LEAD FREE

- A. Stainless steel bellow type, complies with and sized in accordance with PDI WH-201.
- B. Pre-charged suitable for operation in temperature range 34 to 250 degrees F and maximum 150 psi working pressure.

2.25 THERMOSTATIC MIXING VALVE

- A. Valve: Chrome plated cast brass body, stainless steel or nickel plated bellows, integral temperature adjustment.
- B. Accessories:
 - 1. Check valve on inlets.
 - 2. Volume control shut-off valve on outlet.
 - 3. Stem thermometer on outlet.
 - 4. Strainer stop checks on inlets.
- C. Cabinet: 16 gage enameled steel, for surface mounting with keyed lock.

2.26 FLOOR DRAIN / FLOOR SINK

- A. Floor Drain, FD-1: ASME A112.21.1; cast iron two piece body with double drainage flange, weep holes, 1/2 inch trap primer connection, reversible clamping collar, and round adjustable nickel-bronze strainer.
- B. Floor Sink, FS-1: 12 inch x 12 inch x 6 inch floor receptor, full nickel-bronze grate, enamel interior and top, interior bottom dome strainer and 1/2 inch trap primer connection.
- C. Floor Drain / Floor Sink Trap Primer Valve: ASSE 1018, corrosion resistant brass, piston operated, no springs or diaphragms, adjustable in line pressure, 1/2 inch inlet and outlet openings.

2.27 CLEANOUTS

- A. Cleanout, Interior Finished Floor Area, CO-1: cast iron body with anchor flange, reversible clamping collar, threaded top assembly, and round polished bronze scoriated cover.
- B. Cleanout, Interior Unfinished Inline Accessible Area, CO-2: cast iron body ferrule type with ABS countersunk plug.
- C. Wall Cleanout, Interior Finished Wall Area, WCO-1: cast iron body with lacquered ABS tapered threaded plug and round stainless steel wall access cover with securing screw.

PART 3 EXECUTION

3.1 EXAMINATION

- A. Verify that excavations are to required grade, dry, and not over-excavated.

3.2 PREPARATION

- A. Ream pipe and tube ends. Remove burrs. Bevel plain end ferrous pipe.
- B. Remove scale and dirt, on inside and outside, before assembly. Protect open ends with temporary plugs or caps.
- C. Prepare piping connections to equipment with flanges or unions.

3.3 INSTALLATION

- A. Install in accordance with manufacturer's instructions.
- B. Provide non-conducting dielectric connections wherever jointing dissimilar metals.
- C. Route piping in orderly manner and maintain gradient. Route parallel and perpendicular to walls.
- D. Install piping to maintain headroom, conserve space, and not interfere with use of space.
- E. Group piping whenever practical at common elevations.
- F. Provide clearance in hangers and from structure and other equipment for installation of insulation and access to valves and fittings.
- G. Provide access where valves and fittings are not exposed.
- H. Install vent piping penetrating roofed areas to maintain integrity of roof assembly.
- I. Where pipe support members are welded to structural building framing, scrape, brush clean, and apply one coat of zinc-rich primer to welding.
- J. Provide support for utility meters in accordance with requirements of utility companies.
- K. Excavate in accordance with Section 31 23 16.
- L. Backfill in accordance with Section 31 23 23.
- M. Trench - Provide 3 inches of sand for bedding material at trench bottom to provide uniform bedding for piping. Level bedding materials and install pipe on prepared bedding. Encase installed piping with 6 inches of pea gravel. Provide fill material to trench and compact to 90 percent maximum density. Route pipe in straight line.
- N. Install bell and spigot pipe with bell end upstream.
- O. Install water piping to ASME B31.9.

- P. Copper Pipe and Tube: Make soldered joints in accordance with ASTM B828, using specified solder, and flux meeting ASTM B813; in potable water systems use flux also complying with NSF 61 and NSF 372.
- Q. PVC Pipe: Make solvent-welded joints in accordance with ASTM D2855.
- R. Sleeve pipes passing through partitions, walls, and floors.
- S. Install brass male adapters each side of valves in copper piped system. Solder adapters to pipe.
- T. PVC piping is not allowed to be installed in places of assembly, plenum spaces, exit discharge corridors or stairs. Use cast iron or copper piping in these locations.
- U. Install firestopping at fire rated construction perimeters and openings containing penetrating sleeves and piping.
- V. Install water hammer arrestors complete with accessible isolation valve on hot and cold water supply piping to fixtures to prevent hammer or install air chambers on hot and cold water supply piping to each fixture or group of fixtures (each washroom). Fabricate same size as supply pipe or 3/4 inch minimum, and minimum 18 inches long.
- W. Inserts:
 - 1. Provide inserts for placement in concrete formwork.
 - 2. Provide inserts for suspending hangers from reinforced concrete slabs and sides of reinforced concrete beams.
 - 3. Provide hooked rod to concrete reinforcement section for inserts carrying pipe over 4 inches.
 - 4. Where concrete slabs form finished ceiling, locate inserts flush with slab surface.
 - 5. Where inserts are omitted, drill through concrete slab from below and provide through-bolt with recessed square steel plate and nut above slab.
- X. Pipe Hangers and Supports:
 - 1. Install in accordance with ASME B31.9.
 - 2. Support horizontal piping as indicated.
 - 3. Install hangers to provide minimum 1/2 inch space between finished covering and adjacent work.
 - 4. Place hangers within 12 inches of each horizontal elbow.
 - 5. Use hangers with 1-1/2 inch minimum vertical adjustment. Design hangers for pipe movement without disengagement of supported pipe.
 - 6. Support vertical piping at every other floor. Support riser piping independently of connected horizontal piping.
 - 7. Where several pipes can be installed in parallel and at same elevation, provide multiple or trapeze hangers.
 - 8. Provide copper plated hangers and supports for copper piping.
 - 9. Prime coat exposed steel hangers and supports. Hangers and supports located in crawl spaces, pipe shafts, and suspended ceiling spaces are not considered exposed.
 - 10. Support cast iron drainage piping at every joint.
- Y. Pipe Sleeve-Seal Systems:
 - 1. Install manufactured sleeve-seal systems in sleeves located in grade slabs and exterior concrete walls at piping entrances into building.
 - 2. Provide sealing elements of the size, quantity, and type required for the piping and sleeve inner diameter or penetration diameter.
 - 3. Locate piping in center of sleeve or penetration.
 - 4. Install field assembled sleeve-seal system components in annular space between sleeve and piping.
 - 5. Tighten bolting for a watertight seal.
 - 6. Install in accordance with manufacturer's recommendations.
- Z. Equipment Bases and Supports
 - 1. Provide housekeeping pads of concrete, minimum 3-1/2 inches thick and extending 6 inches beyond supported equipment. Refer to Section 03 30 00.

2. Using templates furnished with equipment, install anchor bolts, and accessories for mounting and anchoring equipment.
3. Construct supports of steel members or formed steel channel. Brace and fasten with flanges bolted to structure.

AA. Flashing

1. Provide flexible flashing and metal counterflashing where piping penetrates weather or waterproofed walls, floors, and roofs.
2. Flash vent and soil pipes projecting 3 inches minimum above finished roof surface with lead worked 1 inch minimum into hub, 8 inches minimum clear on sides with 24 x 24 inches sheet size. For pipes through outside walls, turn flanges back into wall and caulk, metal counter-flash, and seal.
3. Flash floor drains in floors with topping over finished areas with lead, 10 inches clear on sides with minimum 36 x 36 inch sheet size. Fasten flashing to drain clamp device.
4. Seal floor, shower, and mop sink drains watertight to adjacent materials.
5. Adjust storm collars tight to pipe with bolts; caulk around top edge. Use storm collars above roof jacks. Screw vertical flange section to face of curb.

BB. Sleeves

1. Set sleeves in position in forms. Provide reinforcing around sleeves.
2. Size sleeves large enough to allow for movement due to expansion and contraction. Provide for continuous insulation wrapping.
3. Extend sleeves through floors 1 inch above finished floor level. Caulk sleeves.
4. Where piping penetrates floor, ceiling, or wall, close off space between pipe and adjacent work with fire stopping, insulation and caulk airtight. Provide close fitting metal collar or escutcheon covers at both sides of penetration.
5. Install chrome plated steel escutcheons at finished surfaces.

3.4 APPLICATION

- A. Install unions downstream of valves and at equipment or apparatus connections.
- B. Install brass male adapters each side of valves in copper piped system. Solder adapters to pipe.
- C. Install ball valves for shut-off and to isolate equipment, part of systems, branch piping, or vertical risers.
- D. Install globe valves for throttling, bypass, or manual flow control services.

3.5 TOLERANCES

- A. Sanitary Drainage Piping: Establish invert elevations, slopes for drainage to 1/8 inch per foot minimum on mains 4 inches and larger. Install branch mains smaller than 4 inch with 1/4 inch per foot minimum.
- B. Storm Drainage Piping: Establish invert elevations, slopes for drainage to 1/8 inch per foot minimum.

3.6 DISINFECTION OF DOMESTIC WATER PIPING SYSTEM

- A. Final water samples shall be sent to a State Department of Health approved testing lab in the State of New York and sample test results shall be submitted to A/E of record.
- B. Prior to starting work, verify system is complete, flushed, and clean.
- C. Ensure acidity (pH) of water to be treated is between 7.4 and 7.6 by adding alkali (caustic soda or soda ash) or acid (hydrochloric).
- D. Inject disinfectant, free chlorine in liquid, powder, tablet, or gas form throughout system to obtain 50 to 80 mg/L residual.

- E. Bleed water from outlets to ensure distribution and test for disinfectant residual at minimum 15 percent of outlets.
- F. Maintain disinfectant in system for 24 hours.
- G. If final disinfectant residual tests less than 25 mg/L, repeat treatment.
- H. Flush disinfectant from system until residual equal to that of incoming water or 1.0 mg/L.
- I. Take samples no sooner than 24 hours after flushing, from 10 percent of outlets and from water entry, and analyze in accordance with AWWA C651.

3.7 SERVICE CONNECTIONS

- A. Provide new water service complete with approved reduced pressure backflow preventer and water meter with by-pass valves, pressure reducing valve, and sand strainer.
- B. Test sanitary waste, vent piping and storm drainage system in accordance with Plumbing Code of the State of New York.
- C. Test backflow prevention device in accordance with ASSE 5013, by State certified backflow prevention device tester.
 - 1. Provide test results and Certification of tester.
- D. Test domestic water piping system in accordance with Plumbing Code of the State of New York.
- E. Provide new gas piping into building. Building gas service distribution piping to have pressure of [1/2] psi.
- F. Test 1/2 psi gas piping system at 10 psi for one hour in accordance with Fuel Gas Code of the State of New York and New York State SED Manual of Planning Standards.

3.8 SCHEDULES

- A. Pipe Hanger Spacing:
 - 1. Metal Piping:
 - a. Pipe Size: 1/2 inch to 1-1/4 inch:
 - 1) Maximum Hanger Spacing: 6.5 ft.
 - 2) Hanger Rod Diameter: 3/8 inches.
 - b. Pipe Size: 1-1/2 inch to 2 inch:
 - 1) Maximum Hanger Spacing: 10 ft.
 - 2) Hanger Rod Diameter: 3/8 inch.
 - c. Pipe Size: 2-1/2 inch to 3 inch:
 - 1) Maximum Hanger Spacing: 10 ft.
 - 2) Hanger Rod Diameter: 1/2 inch.
 - d. Pipe Size: 4 inch to 6 inch:
 - 1) Maximum Hanger Spacing: 10 ft.
 - 2) Hanger Rod Diameter: 5/8 inch.
 - 2. Cast Iron (All Sizes) pipe length less than 10':
 - a. Maximum hanger Spacing: 5 ft.
 - b. Hanger rod diameter: 5/8 inch
 - 3. Cast Iron (All Sizes) with 10 foot length of pipe
 - a. Maximum hanger Spacing: 10 ft.
 - b. Hanger rod diameter: 5/8 inch
 - 4. CPVC, 1 inch and smaller
 - a. Maximum hanger Spacing: 3 ft.
 - b. Hanger rod diameter: 1/2 inch
 - 5. CPVC, 1-1/4 inches and larger
 - a. Maximum hanger Spacing: 4 ft.
 - b. Hanger rod diameter: 1/2 inch

6. Copper Tube, 1-1/4 inches and smaller
 - a. Maximum hanger Spacing: 6 ft.
 - b. Hanger rod diameter: 1/2 inch
7. Copper Tube, 1-1/2 inches and larger
 - a. Maximum hanger Spacing: 10 ft.
 - b. Hanger rod diameter: 1/2 inch
8. PVC (All Sizes)
 - a. Maximum hanger Spacing: 4 ft.
 - b. Hanger rod diameter: 3/8 inch

END OF SECTION

SECTION 28 20 00
VIDEO SURVEILLANCE

PART 1 GENERAL

1.1 SECTION INCLUDES

- A. Video surveillance system requirements.
- B. Video recording and viewing equipment.
- C. Accessories.

1.2 RELATED REQUIREMENTS

- A. Section 07 84 00 - Firestopping.
- B. Section 26 05 19 - Low-Voltage Electrical Power Conductors and Cables.
- C. Section 26 05 26 - Grounding and Bonding for Electrical Systems.
- D. Section 26 05 29 - Hangers and Supports for Electrical Systems.
- E. Section 26 05 33.13 - Conduit for Electrical Systems.
- F. Section 26 05 33.16 - Boxes for Electrical Systems
- G. Section 27 05 26 - Grounding and Bonding For Communications Systems.
- H. Section 27 05 53 - Identification For Communications Systems.
- I. Section 27 10 05 - Communications Copper Cabling: Data cables for IP video surveillance system network connections.
- J. Section 27 15 55 - Communications Cable Testing.

1.3 ADMINISTRATIVE REQUIREMENTS

- A. Coordination:
 - 1. Coordinate the placement of cameras with structural members, ductwork, piping, equipment, luminaires, diffusers, fire suppression system components, and other potential conflicts installed under other sections or by others.
 - 2. Coordinate the work with Manufacturer's Representative Services supplier for cameras and equipment, installation, testing, adjusting, integration, and system start-up.
 - 3. Notify Architect of any conflicts with or deviations from the contract documents. Obtain direction before proceeding with work.
- B. Preinstallation Meetings:
 - 1. Conduct meeting with facility representative to review camera and equipment locations and camera field of view objectives.
 - 2. Conduct meeting with facility representative and other related equipment manufacturers to discuss video surveillance system interface requirements.

1.4 SUBMITTALS

- A. See Section 01 30 00 - Administrative Requirements, for submittal procedures.

- B. Shop Drawings: Include plan views indicating locations of system components and proposed size, type, and routing of conduits and/or cables. Include elevations and details of proposed equipment arrangements. Include system interconnection schematic diagrams. Include requirements for interface with other systems.
- C. Evidence of qualifications for installer.
- D. Field quality control test reports.
- E. Project Record Documents: Record actual locations of cameras and routing of cables.
- F. Operation and Maintenance Data: Include detailed information on system operation, equipment programming and setup, replacement parts, and recommended maintenance procedures and intervals.
 - 1. Include contact information for entity that will be providing contract maintenance and trouble call-back service.

1.5 QUALITY ASSURANCE

- A. Comply with the following:
 - 1. NFPA 70.
 - 2. Applicable TIA/EIA standards.
- B. Maintain at the project site a copy of each referenced document that prescribes execution requirements.
- C. Manufacturer Qualifications: Company specializing in manufacturing the products specified in this section with minimum three years documented experience and with service facilities within 100 miles of project.
- D. Installer Qualifications: Company specializing in performing the work of this section with minimum three years documented experience with video surveillance systems of similar size, type, and complexity.
- E. Product Listing Organization Qualifications: An organization recognized by OSHA as a Nationally Recognized Testing Laboratory (NRTL) and acceptable to authorities having jurisdiction.

1.6 DELIVERY, STORAGE, AND HANDLING

- A. Receive, inspect, handle, and store products in accordance with manufacturer's instructions and NECA 303.
- B. Store products in manufacturer's packaging, keep dry and protect from damage until ready for installation.

1.7 FIELD CONDITIONS

- A. Maintain field conditions within manufacturer's required service conditions during and after installation.

1.8 WARRANTY

- A. See Section 01 78 00 - Closeout Submittals, for additional warranty requirements.
- B. Provide minimum one year manufacturer warranty covering repair or replacement due to defective materials or workmanship.

PART 2 PRODUCTS

2.1 SYSTEM DESCRIPTION

- A. The intent of this specification is to lay out the infrastructure requirements for an expansion of the Owner's Digital Video Management System (DVMS) and coordinate the installation of the security equipment furnished to the electrical contractor at points indicated on the Drawings.
- B. Provide all structured cabling, terminations, boxes, conduit, penetrations, sleeves, wire-mold, fasteners, and common installation material such that the project has a complete and workable video surveillance system compliant with this Section. Hardware products which do not meet this design as laid out in 27 05 33.13 - Conduit for Communications Systems, Sections 27 05 29 - Hangers and Supports, for Communications Systems and 27 10 05 - Communications Copper Cabling, shall not be acceptable.
- C. Install all equipment furnished by the Manufacturer's Representative Services supplier referred to in this specification as the Integrator. The electrical contractor shall coordinate with the Integrator the transmittal of equipment, verification of the camera schedule, field installation, final aiming and commissioning of the communications cabling system that supports the system.
- D. The electrical contractor shall provide all necessary coordination with the Integrator to produce a fully commissioned DVMS & IP Security Camera system.

2.2 OWNER-FURNISHED PRODUCTS AND SERVICES

- A. DVMS equipment for the project shall be purchased by the Owner via New York State Contract.
 - 1. Identified products shall be installed by the Owner or System Integrator.
 - 2. Remaining products identified as furnished by the Owner shall be turned over to the Electrical Contractor for installation.
 - 3. Refer to the Responsibility Matrix later in this Section for product listing.
- B. The Owner has further entered into a separate contract for Manufacturer's Representative Services.
 - 1. The term Manufacturer's Representative Services supplier shall be synonymous with and interchangeable with the terms Integrator or System Integrator.
 - 2. The Manufacturer's Representative Services supplier for the project is:
 - a. Day Automation Systems, Inc. 7931 Rae Boulevard Rochester, NY 14475 phone: 800-836-0969.
 - 3. Refer to Responsibility Matrix later in this Section for description of services provided.
- C. For a complete listing of Owner-Furnished products including Manufacturer, model, and description, contact the Manufacturer's Representative Services supplier.

2.3 PRODUCTS

- A. Provide such equipment as outlined in the responsibility matrix below, including but not limited to:
 - 1. Patch Cables: As specified in Section 27 10 05 - Communications Copper Cabling.
 - 2. Data Cable Surge Suppression: As specified in Section 27 05 26 - Grounding and Bonding For Communications Systems.
 - 3. Patch Panels: As specified in Section 27 10 05 - Communications Copper Cabling.
- B. Install equipment, identified in the responsibility matrix below, as supplied by the Owner, but not installed by the Owner or Integrator.
- C. Provide wiring, conduit, wire terminations, back boxes, wire-mold, fasteners and common installation material required to connect devices furnished as part of, or integral to the DVMS system regardless of the source of the supply.
 - 1. Provide all wiring and terminations for the DVMS system in accordance with the specifications, contract drawings, and detailed engineered drawings provided by factory representative.
- D. Provide all other devices required for proper complete system operation including, but not limited to, electrical switches, transformers, disconnect switches, sensors, safety devices, power supplies, enclosure, and circuit breakers.

E. Reference the responsibility matrix below:

EC - Prime Electrical Contractor
Owner - Project Owner
Integrator - System Integrator

PRODUCTS	FURNISHED BY	INSTALLED BY	SIGNAL WIRING BY	PROGRAMMED BY
NETWORK VIDEO RECORDER	OWNER	INTEGRATOR	EC	INTEGRATOR
CAMERAS & MOUNTS	OWNER	EC	EC	INTEGRATOR
CAMERA SOFTWARE LICENSE	OWNER	INTEGRATOR	N/A	INTEGRATOR
PATCH CABLES	EC	EC	EC	N/A
DATA CABLE SURGE SUPPRESSION	OWNER	EC	EC	N/A
PATCH PANELS	EC	EC	EC	N/A
NETWORK SWITCHES	OWNER	OWNER	EC	OWNER

PART 3 EXECUTION

3.1 EXAMINATION

- A. Verify that field measurements are as shown on the drawings.
- B. Verify that ratings and configurations of system components are consistent with the indicated requirements.
- C. Verify that mounting surfaces are ready to receive system components.
- D. Verify that branch circuit wiring installation is completed, tested, and ready for connection to system.
- E. Verify that conditions are satisfactory for installation prior to starting work.

3.2 INSTALLATION

- A. Install video surveillance system in accordance with NECA 1 (general workmanship) and NECA 303.
- B. Comply with the provisions of NFPA 70.
- C. Comply with manufacturer’s instructions and recommendations for installation of product in the applications indicated. Anchor products securely in place, accurately located and aligned with other work.
- D. Provide required support and attachment in accordance with Section 26 05 29.
- E. Wiring Method: Unless otherwise indicated, use cables (not in conduit).
 1. Use listed plenum rated cables in all spaces.
 2. Conduit: Comply with Section 26 05 33.13.
 3. Conceal all cables unless specifically indicated to be exposed.
 4. Cables in the following areas may be exposed, unless otherwise indicated:
 - a. Equipment closets.
 5. Route exposed cables parallel or perpendicular to building structural members and surfaces.
 6. Include service loop cable lengths to allow relocation of cameras within 10 ft of installed location.

- F. Provide grounding and bonding in accordance with Section 27 05 26.
- G. Install firestopping to preserve fire resistance rating of partitions and other elements, using materials and methods specified in Section 07 84 00.
- H. Identify system wiring and components in accordance with Section 27 05 53.
- I. Label all cameras per Owner requirements and to match as in software naming convention.
- J. For IP Cameras: Test all cabling per Section 27 15 55.
- K. The Contractor shall carefully follow instructions in documentation provided by the manufacturers to insure all steps have been taken to provide a reliable system.
- L. Coordinate with the Manufacturer's Representative Services supplier to ensure the following:
 - 1. All cameras are verified for start up and software programming.
 - 2. All equipment has been tested and configured in accordance with instructions provided by the manufacturer prior to installation.
- M. Coordinate all final locations with owner. Get sign off from owner on final view of camera.

3.3 DVMS - GROUNDING

- A. The Grounding wires connected from the Surge devices and earth ground to be grounded according to NEC and NFPA related codes. Grounding to common electrical building ground to be verified prior to install via measurement of resistance to ground in grounding system connection points. Grounding shall eliminate potential equipment damage from possible ground loops created by multiple ground rods. It shall allow the electrical potential of the entire facility to rise and fall in a uniform manner, reducing the possibility of excessive current flow on the grounding system.
- B. Whenever possible, the conductor length to earth ground should be less than the conductor length from the surge suppression device to the protected equipment.
- C. When connecting a parallel surge suppression device to a building ground system, the leads from the surge suppression device should be as short and straight as possible.
- D. The ground system should have a maximum resistance of 25 ohms. 5 ohms, or less, is the preferred level for optimum performance of the surge suppression device.

3.4 INTERFACE WITH OTHER PRODUCTS

- A. Interface installation of video surveillance with security access and intrusion detection systems.

3.5 FIELD QUALITY CONTROL

- A. See Section 01 40 00 - Quality Requirements, for additional requirements.
- B. Coordinate with the Manufacturer's Representative Services supplier for the following:
 - 1. Perform inspection and testing.
 - 2. Prepare and start system in accordance with manufacturer's instructions.
 - 3. Adjust cameras to provide desired field of view and produce suitable images under all service lighting conditions.
 - 4. Program system parameters according to requirements of Owner.
 - 5. Test for proper interface with other systems.
- C. Each shall correct defective work, adjust for proper operation, and retest until entire system complies with contract documents.
- D. Submit detailed reports indicating inspection and testing results and corrective actions taken.

3.6 CLEANING

- A. Clean exposed surfaces to remove dirt, paint, or other foreign material and restore to match original factory finish.

3.7 CLOSEOUT ACTIVITIES

- A. See Section 01 78 00 - Closeout Submittals, for closeout submittals.
- B. Coordinate with the Manufacturer's Representative Services supplier for the following:
 - 1. Demonstration: Demonstrate proper operation of system to Owner.
 - a. Provide personnel to correct deficiencies or make adjustments as directed.
- C. The Contractor and Manufacturer's Representative Services supplier shall each provide a Hardware Manual as it relates to the products supplied under their scope of work.
 - 1. The manual shall describe all equipment furnished including:
 - a. General description and specifications.
 - b. Installation and check out procedures.
 - c. Equipment layout and electrical schematics to the component level.
 - d. System layout drawings and schematics.
 - e. Alignment and calibration procedures.
 - f. Manufacturers' repair parts list indicating sources of supply.
- D. As-Built Drawings: During system installation, the Contractor shall maintain a separate hard copy set of drawings, elementary diagrams, and wiring diagrams of the DVMS to be used for record drawings. This set shall be accurately kept up to date by the Contractor with all changes and additions to the DVMS. Copies of the final as-built drawings shall be provided to the end user in PDF format.

3.8 PROTECTION

- A. Protect installed system components from subsequent construction operations.

END OF SECTION

SECTION 33 14 16
WATER UTILITY DISTRIBUTION PIPING

PART 1 GENERAL

1.1 SECTION INCLUDES

- A. Water pipe for site conveyance lines.
- B. Pipe valves.
- C. DR11 HDPE Pipe and Fittings.
- D. Tracer Wire, Underground Warning tape.
- E. Pipe Supports.
- F. Joint Restraint Appurtenances.
- G. Meters & Appurtenances.

1.2 RELATED REQUIREMENTS

- A. Section 31 23 16.13 - Trenching: Excavating, bedding, and backfilling.

1.3 REFERENCE STANDARDS

- A. ASME B16.18 - Cast Copper Alloy Solder Joint Pressure Fittings; 2021.
- B. ASME B16.22 - Wrought Copper and Copper Alloy Solder-Joint Pressure Fittings; 2021.
- C. ASTM B88 - Standard Specification for Seamless Copper Water Tube; 2022.
- D. AWS A5.8M/A5.8 - Specification for Filler Metals for Brazing and Braze Welding; 2019.
- E. AWWA C115/A21.15 - Flanged Ductile-Iron Pipe with Ductile-Iron or Gray-Iron Threaded Flanges; 2020.
- F. AWWA C500 - Metal-Seated Gate Valves for Water Supply Service; 2019.
- G. AWWA C509 - Resilient-Seated Gate Valves for Water Supply Service; 2023.
- H. AWWA C600 - Installation of Ductile-Iron Mains and Their Appurtenances; 2023.
- I. AWWA C606 - Grooved and Shouldered Joints; 2022.
- J. AWWA C906 - AWWA Standard for Polyethylene (PE) Pressure Pipe and Fittings, 4 In. Through 63 In., for Water Distribution and Transmission.
- K. AWWA M55 - PE Pipe - Design and Installation, Second Edition; 2020.
- L. NSF 61 - Drinking Water System Components - Health Effects; 2024.
- M. ASTM D1784 - 11 Standard Specification for Rigid Poly(Vinyl Chloride) (PVC) Compounds and Chlorinated Poly(Vinyl Chloride) (CPVC) Compounds.
- N. ASTM D3261 - 10a Standard Specification for Butt Heat Fusion Polyethylene (PE) Plastic Fittings for Polyethylene (PE) Plastic Pipe and Tubing.

1.4 SUBMITTALS

- A. See Section 01 30 00 - Administrative Requirements, for submittal procedures.
- B. Product Data: Provide data on pipe materials, pipe fittings, valves and accessories.
- C. Manufacturer's Certificate: Certify that products meet or exceed specified requirements.
- D. Project Record Documents: Record actual locations of piping mains, valves, connections, thrust restraints, and invert elevations. Identify and describe unexpected variations to subsoil conditions or discovery of uncharted utilities.

1.5 QUALITY ASSURANCE

- A. Perform Work in accordance with municipality requirements.

1.6 DELIVERY, STORAGE, AND HANDLING

- A. Deliver and store valves in shipping containers with labeling in place.

PART 2 PRODUCTS

2.1 GENERAL

- A. Potable Water Supply Systems: All materials contacting potable water shall be certified compliant with NSF 61 for maximum lead content; label pipe and fittings.

2.2 WATER PIPE

- A. Copper Tubing: ASTM B88, Type K, Annealed:
 - 1. Fittings: ASME B16.18, cast copper, or ASME B16.22, wrought copper.
 - 2. Joints: Compression connection or AWS A5.8M/A5.8, BCuP silver braze.
- B. HDPE Pipe: AWWA M55
 - 1. Materials used for the manufacturing of polyethylene pipe and fittings shall be high density polyethylene (HDPE). The HDPE materials also shall be certified as suitable for potable water products by the National Sanitation Foundation (NSF) and AWWA (AWWA standard is C906). The material shall have a minimum hydrostatic design basis (HDB) of 1600 psi at 73° F.
 - 2. Pipe shall be manufactured in accordance with AWWA C-906. Pipe shall be furnished with squarely cut, plain ends in lengths that will allow for easy unloading, storage and installation. Nominal diameter, dimension ration, and pressure class shall be as shown on the contract drawings or herein specified.
 - 3. Permanent identification of piping service shall be provided by co-extruding blue material into the pipe's outside surface. The material used shall be the same material as the pipe except for color. Stripes printed or painted on the outside surface shall not be acceptable.
 - 4. Fittings shall conform to ASTM D3261 for butt-type and shall be molded or thermoformed from sections of pipe.
 - 5. All pipe lengths and fittings shall be joined by thermal butt-fusion in accordance with the manufacturer requirements. Mechanical joint fittings and HDPE/MJ adapters with internal stiffening ring shall join all connections to existing watermains and new appurtenances (HD C110 full body MJ gland, stiffener and stainless steel tee bolts). Nominal diameter and dimension ratio shall be as shown on the contract drawings.
 - 6. The pipe and fitting Manufacturer shall have an established quality control program responsible for inspecting incoming and outgoing materials. Incoming polyethylene materials shall be inspected for density, melt flow rate, and contamination. The cell classification properties of the material shall be certified by the supplier. Incoming materials shall be approved by Quality Control before processing into finished goods. Outgoing products shall be tested as required in AWWA C906.

7. The Manufacturer shall maintain permanent Quality Control (QC) and Quality Assurance (AQ) records. Certification or copy of these records shall be made available to the Engineer on request.

2.3 VALVES

- A. Gate Valves Up To 3 Inches:
 1. Brass or Bronze body, non-rising stem, inside screw, single wedge or disc, compression ends, with control rod, post indicator, valve key, and extension box.
- B. Gate Valves 3 Inches and Over:
 1. AWWA C509, ductile iron body, bronze trim, non-rising stem with square nut, single wedge, resilient seat, mechanical joint ends, stainless steel bonnet bolts, valve key, and extension box.

2.4 BEDDING AND COVER MATERIALS

- A. Bedding: As specified in Section 31 23 16.13.
- B. Cover: As specified in Section 31 23 16.13.

2.5 ACCESSORIES

- A. Concrete for Thrust Restraints: Concrete type specified in Section 03 30 00.

2.6 UNDERGROUND MARKING

- A. Underground markings shall comply with AWWA Uniform Color Code.
- B. Tracer Wire: Continuous 12 AWG, Solid, Copper clad high strength steel, with minimum 30 mil HDPE insulation, blue in color, suitable for direct burial.
- C. Warning Tape:
 1. Detectable Warning Tape: Magnetic detectable warning tape, 6 inches wide, 5 mil thick tape with aluminum backing, clear plastic covering, imprinted with "Water Service" in large black letters on blue background.

2.7 JOINT RESTRAINT APPURTENANCES

- A. M/J Joint Thrust Restraint Glands shall consist of multiple gripping wedges incorporated into a follower gland meeting the applicable requirements of AWWA C110 and have a working pressure rating of 350 psi.
- B. Push on Joint Thrust Restraint shall meet or exceed the performance criteria of U.S. Pipe Field Lok Gaskets, Gripper Gaskets, or Approved Equal.

2.8 METERS & APPURTENANCES

- A. Electromagnetic flow Meter
 1. Meter shall be acceptable for use in potable water applications and shall be NSF certified.
 2. Meter shall have a flow range of 0.1 – 33 fps
 3. Accuracy < 0.25%
 4. Electrode materials: stainless steel, hastelloy C, gold/platinum plated, tantalum, platinum/rhodium.
 5. Maximum pressure: 300 psi
 6. Coil power: pulsed DC
 7. Ambient temperature: -4oC to 122 oF
 8. Pipe spool material: 316 Stainless Steel
 9. Meter enclosure material: Carbon steel welded
 10. Meter enclosure classification: Nema 4

11. Cable entries: ½ inch
12. Flow Range: 0 GPM – 100 GPM

PART 3 EXECUTION

3.1 EXAMINATION

- A. Verify that building service connection and municipal utility water main size, location, and invert are as indicated.

3.2 PREPARATION

- A. Cut pipe ends square, ream pipe and tube ends to full pipe diameter, remove burrs.
- B. Remove scale and dirt on inside and outside before assembly.
- C. Prepare pipe connections to equipment with flanges or unions.
- D. All PVC or Polyethylene Pipe shall not be left exposed to the sun and shall be covered. The contractor shall also provide documentation to the Construction Inspector as to the pipe's age and handling prior to being delivered to the site. This is to prevent pipe that has been left exposed to the sun at a storage yard from being used on the project. Lack of documentation of PVC or HDPE history will cause said pipe to be rejected.

3.3 BEDDING

- A. Place bedding material according to the trench details provided on the Engineering Drawings.
- B. Backfill around sides and to top of pipe with cover fill, tamp in place and compact to 95 percent.
- C. Tracer Wire:
 1. Install continuous length of tracer wire for the full length of each run of nonmetallic pipe.
 2. Attach wire to top of pipe securely to prevent displacement during installation.
- D. Place fill material in accordance with Section 31 23 23.

3.4 INSTALLATION - PIPE

- A. Maintain separation of water main from sewer piping in accordance with New York State DEC code.
- B. HDPE Pipe:
 1. The Manufacturer shall supply an Installation Manual to the Engineer which outlines guidelines for handling, joining, installing, embedding and testing of polyethylene pipeline. These guidelines shall be used as reference material for the Engineer in his determination of the required procedures.
 2. Joints between plain ends of polyethylene pipe shall be made by butt fusion when possible. The Pipe Manufacturer's fusion procedures shall be followed at all times as well as the recommendations of the Fusion Machine Manufacturer. The wall thickness of the adjoining pipes shall have the same DR at the point of fusion.
 3. If mechanical fittings (which are designed for, or tested and found acceptable for use with polyethylene pipe) are utilized for transitions between pipe materials, repairs, jointing pipe sections, saddle connections, or at other locations; the recommendation of the Mechanical Fitting Manufacturer must be followed. These procedures may differ from other pipe materials.
 4. Where connecting to newly laid ductile iron pipe, the next two ductile iron pipe joints adjacent to the HDPE/DIP connection shall utilize boltless restraining gaskets as approved by the engineer.

5. On each day butt fusions are to be made, the first fusion of the day shall be a trial fusion.
The trial fusion shall be allowed to cool completely, then fusion test straps shall be cut out.
The test strap shall be 12" or 30 times the wall thickness in length (minimum) and 1" or 1.5 times the wall thickness in width (minimum). Bend the test strap until the ends of the strap touch. If the fusion fails at the joint, a new trial fusion shall be made, cooled completely and tested. Butt fusion of pipe to be installed shall not commence until a trial fusion has passed the bent strap test.
6. Pressure testing shall be conducted in accordance with the Manufacturer's recommended procedure. Pressure testing shall use water as the test media. Pneumatic (air) testing is prohibited.
7. Consideration should be given by the Contractor that the length of the HDPE piping expands and contracts considerably with changes in temperature. As such, the Contractor should take appropriate measures to ensure that undue stresses do not occur with fused pipe and its associated fittings.
8. The manufacturer shall have manufacturing and quality control facilities capable of producing and assuring the quality of the pipe and fittings required by these specifications and as shown on the contract drawings.
9. Caution shall be exercised at all times to avoid compression, damage or deformation to the pipe. Pipe shall be inspected before installation for cracks, defects, and chips and any pipe or fittings containing harmful imperfections shall be rejected and removed from the job site. HDPE pipe shall be supported by racks during storage to prevent damage to the bottom. Pipe stored outside shall be covered with opaque material while permitting air circulation around the pipe to prevent excessive heat accumulation. The interior as well as all sealing surfaces or pipe, fittings and other accessories shall be kept free from dirt and foreign matter. Severe impact blows, abrasion damage, and gouging or cutting by metal surfaces or rocks shall be avoided.

3.5 INSTALLATION - VALVES, HYDRANTS, BACKFLOW PREVENTERS

- A. Set valves on solid bearing.
- B. Center and plumb valve box over valve. Set box cover flush with finished grade.

3.6 FIELD QUALITY CONTROL

- A. See Section 01 40 00 - Quality Requirements, for additional requirements.
- B. Perform field inspection and testing in accordance with Section 01 40 00.
- C. Pressure test water piping:
 1. After completion of pipeline installation, including backfill, but prior to final connection to existing system, conduct, in presence of Architect/Engineer, concurrent hydrostatic pressure and leakage tests in accordance with AWWA C600.
 2. Provide equipment required to perform leakage and hydrostatic pressure tests.
 3. Test Pressure: Not less than 200 psi or 50 psi in excess of maximum static pressure, whichever is greater.
 4. Conduct hydrostatic test for at least two-hour duration.
 5. No pipeline installation will be approved when pressure varies by more than 5 psi at completion of hydrostatic pressure test.
 6. Before applying test pressure, completely expel air from section of piping under test. Provide corporation cocks so air can be expelled as pipeline is filled with water. After air has been expelled, close corporation cocks and apply test pressure. At conclusion of tests, remove corporation cocks removed and plug resulting piping openings.
 7. Slowly bring piping to test pressure and allow system to stabilize prior to conducting leakage test. Do not open or close valves at differential pressures above rated pressure.
 8. Examine exposed piping, fittings, valves, hydrants, and joints carefully during hydrostatic pressure test. Repair or replace damage or defective pipe, fittings, valves, hydrants, or joints discovered, following pressure test.

9. No pipeline installation will be approved when leakage is greater than that determined by the following formula:

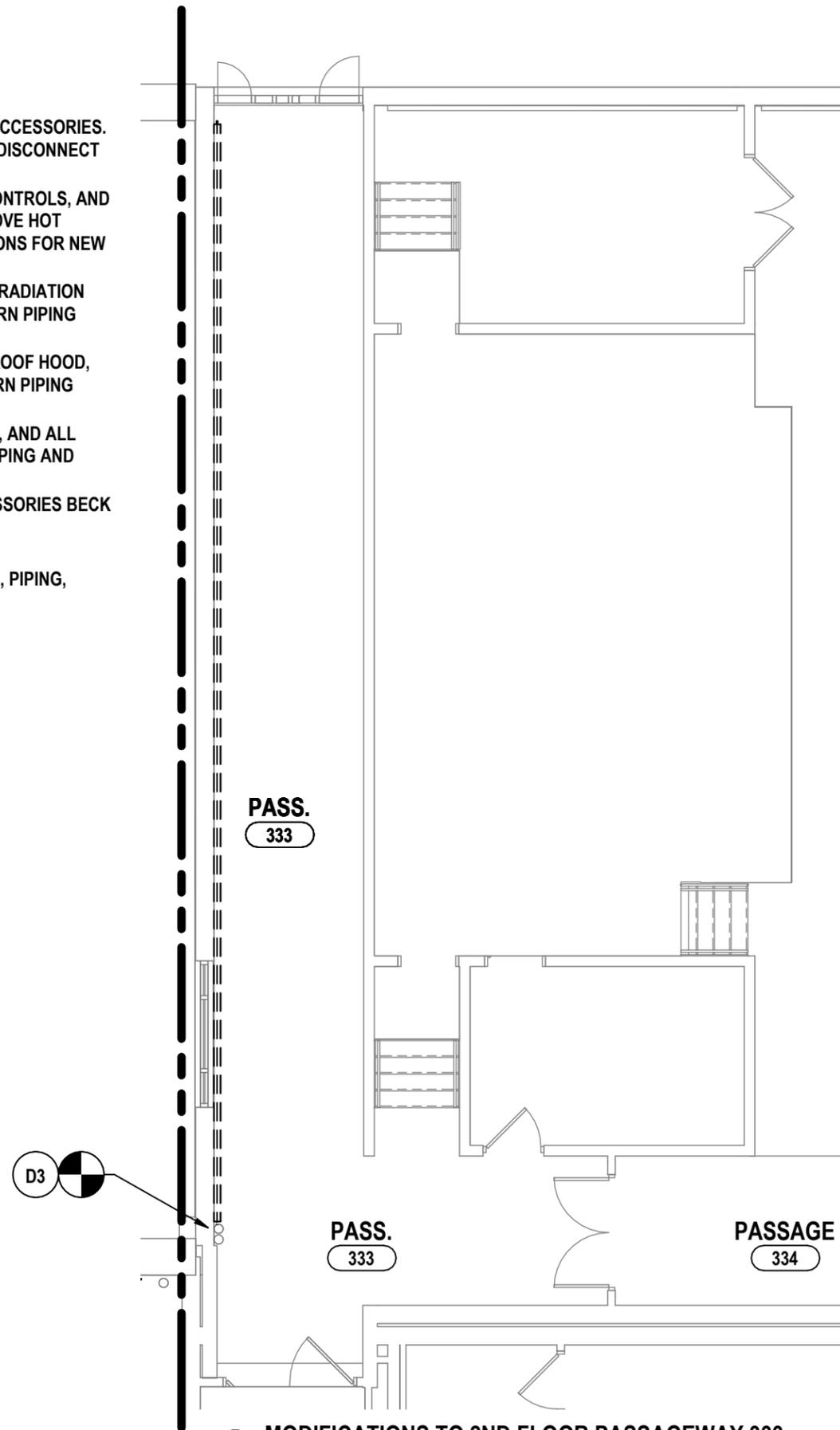
$L = (S \cdot D \cdot \sqrt{P}) / 133,200$
L = allowable leakage, in gallons per hour
S = length of pipe tested, in feet
D = nominal diameter of pipe, in inches
p = average test pressure during leakage test, in pounds per square inch (gauge)

10. When leakage exceeds specified acceptable rate, locate source and make repairs. Repeat test until specified leakage requirements are met.
- D. Perform backflow preventer testing in accordance with ASSE 5013, by State certified backflow prevention device tester.
1. Provide test results and Certification of tester.
- E. If tests indicate Work does not meet specified requirements, remove Work, replace and retest at no cost to Owner.

END OF SECTION

DEMOLITION NOTES - MECHANICAL

- D1 REMOVE AIR HANDLING UNIT, DUCTWORK, CONTROLS, AND ANY ASSOCIATED ACCESSORIES. REMOVE SUPPLY DUCTWORK TO POINTS INDICATED ON SECOND FLOOR PLAN. DISCONNECT HOT WATER SUPPLY AND RETURN AND PREPARE FOR NEW CONNECTION.
- D2 DISCONNECT HOT WATER SUPPLY AND RETURN. REMOVE UNIT VENTILATOR, CONTROLS, AND ALL ASSOCIATED ACCESSORIES. WALL SLEEVE AND LOUVER TO REMAIN. REMOVE HOT WATER SUPPLY AND RETURN TO POINTS INDACATED AND PREPARE CONNECTIONS FOR NEW WORK.
- D3 DISCONNECT HOT WATER SUPPLY AND RETURN PIPING AND REMOVE FIN TUBE RADIATION AND ANY ASSOCIATED ACCESSORIES. REMOVE HOT WATER SUPPLY AND RETURN PIPING BACK TO POINTS INDICATED AND PREPARE FOR NEW CONNECTIONS
- D4 DISCONNECT AND REMOVE FAN POWERED VAV BOX, CONTROLS, DUCTWORK, ROOF HOOD, AND ALL ASSOCIATED ACCESSORIES. REMOVE HOT WATER SUPPLY AND RETURN PIPING BACK TO POINTS INDICATED AND CAP.
- D5 DISCONNECT AND REMOVE WALL MOUNTED CABINET UNIT HEATER, CONTROLS, AND ALL ASSOCIATED ACCESSORIES. DISCONNECT HOT WATER SUPPLY AND RETURN PIPING AND PREPARE FOR NEW CONNECTIONS.
- D6 DISCONNECT AND REMOVE GRILLES, DUCTWORK, AND ALL ASSOCIATED ACCESSORIES BECK TO POINT INDICATED.
- D7 REMOVE & RETAIN SPACE SENSOR & PREPARE FOR NEW WORK.
- D8 DISCONNECT AND REMOVE INDOOR EVAPORATOR, CONDENSING UNIT ON ROOF, PIPING, CONTROLS AND ALL ASSOCIATED ACCESSORIES IN THEIR ENTIRETY.



1 MODIFICATIONS TO 2ND FLOOR PASSAGEWAY 333
1/8" = 1'-0"

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MODIFICATIONS TO 2ND FLOOR - AREA C
 2025 CAPITAL IMPROVEMENT PROJECT
 PRATTSBURGH CENTRAL SCHOOL DISTRICT
 1 ACADEMY STREET PRATTSBURGH, NY 148473

AD4-H1
 PROJECT NO: 2716-043

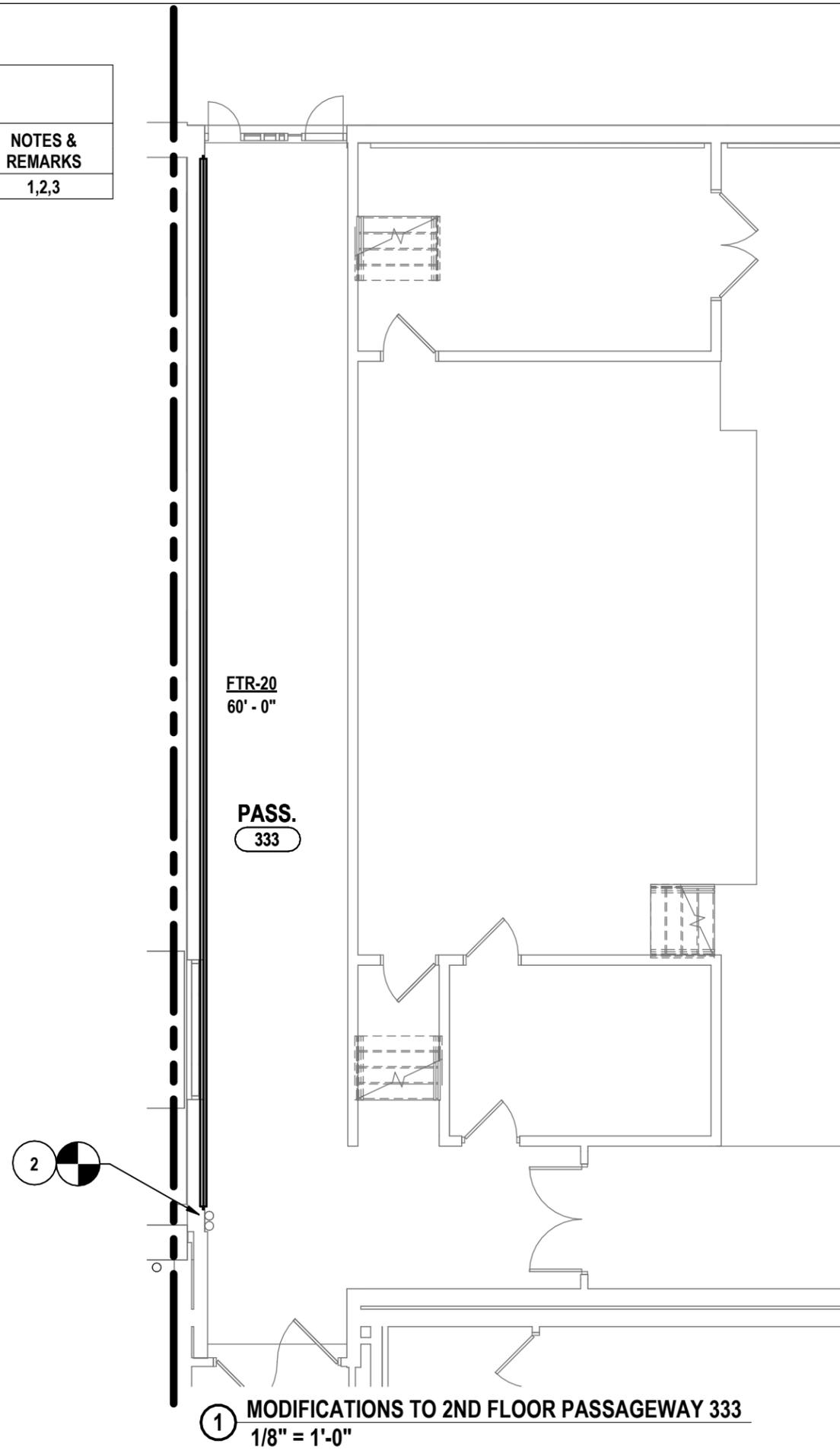
FTR-20 SCHEDULE

UNIT #	LOCATION	ELEMENT			EWT	GPM	CAP BTU/HR/LF	MANUFACTURE / MODEL #	NOTES & REMARKS
		MTG. HEIGHT	ROWS	LENGTH (FT)					
FTR-20	AD4-H2	4"	2	60' - 0"	160	3.5	580	ZEHNDER RITTLING / SBG3	1,2,3

- NOTES: 1. PROVIDE FIELD WIRED CONTROLS.
 2. LENGTH GIVEN IS ACTIVE ELEMENT LENGTH. COVER IS TO BE WALL TO WALL
 3. COLOR TO BE SELECTED BY ARCHITECT.

CONSTRUCTION NOTES MECHANICAL

- 1 PROVIDE UNIT VENTILATOR AND CONTROLS AS SCHEDULE. EXTEND AND RECONNECT HOT WATER PIPING.
- 2 PROVIDE FIN TUBE RADIATION AS SCHEDULED. EXTEND AND RECONNECT HOT WATER PIPING.



① MODIFICATIONS TO 2ND FLOOR PASSAGEWAY 333
 1/8" = 1'-0"

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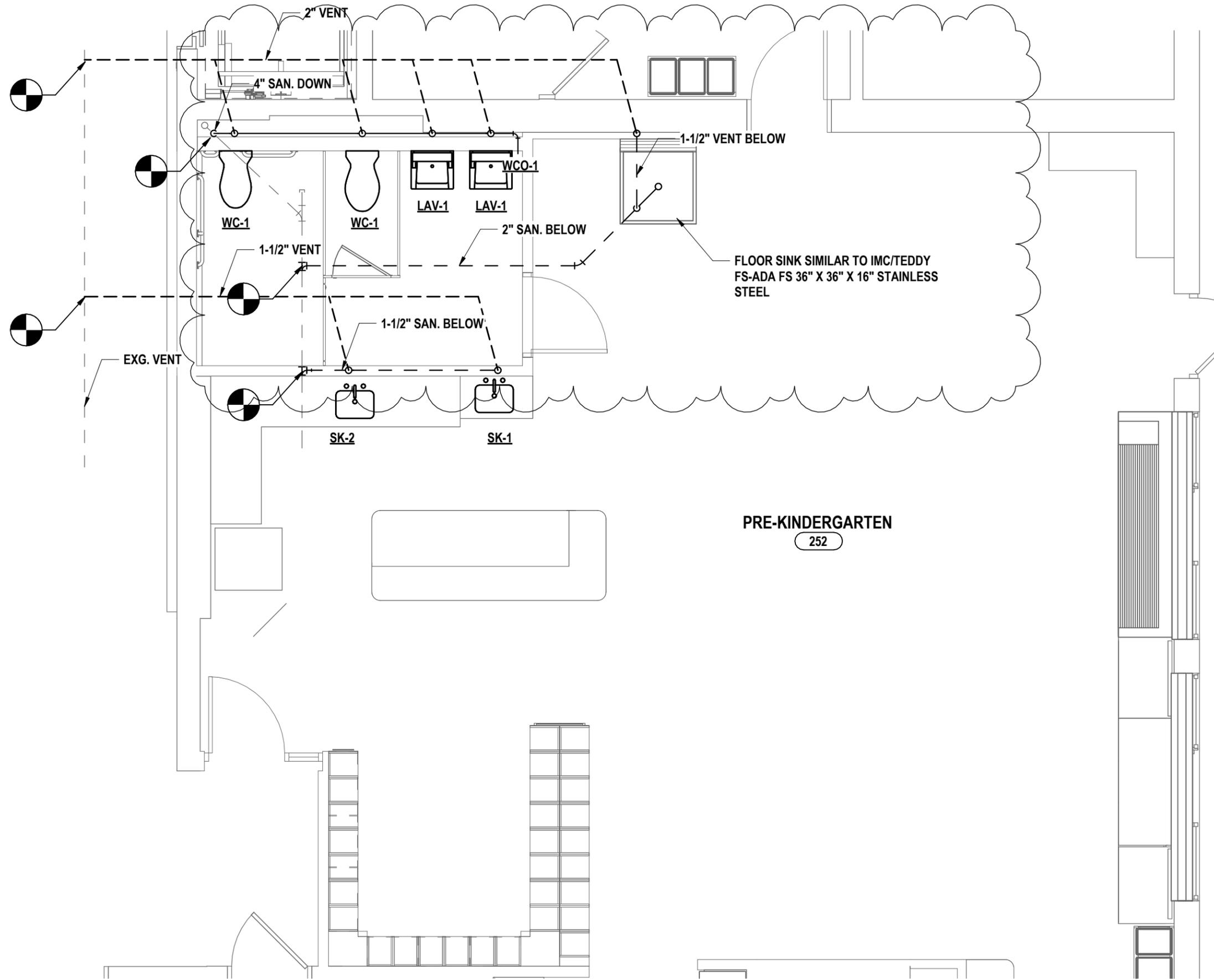
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MODIFICATIONS TO 2ND FLOOR - AREA C
 2025 CAPITAL IMPROVEMENT PROJECT
 PRATTSBURGH CENTRAL SCHOOL DISTRICT
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AD4-H2

PROJECT NO: 2716-043



1 REVISED FIRST FLOOR SANITARY PLAN - AREA C
 1/4" = 1'-0"

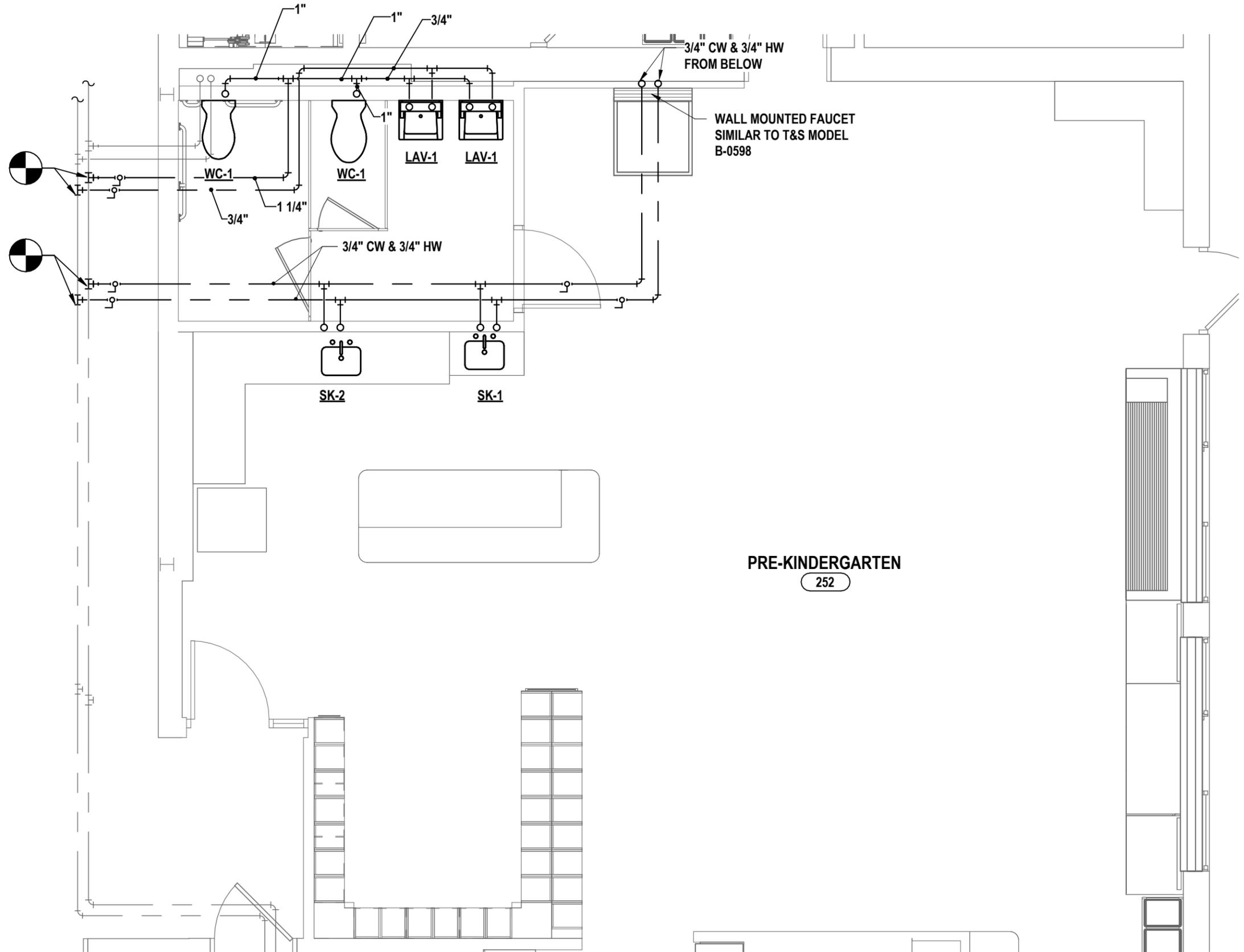
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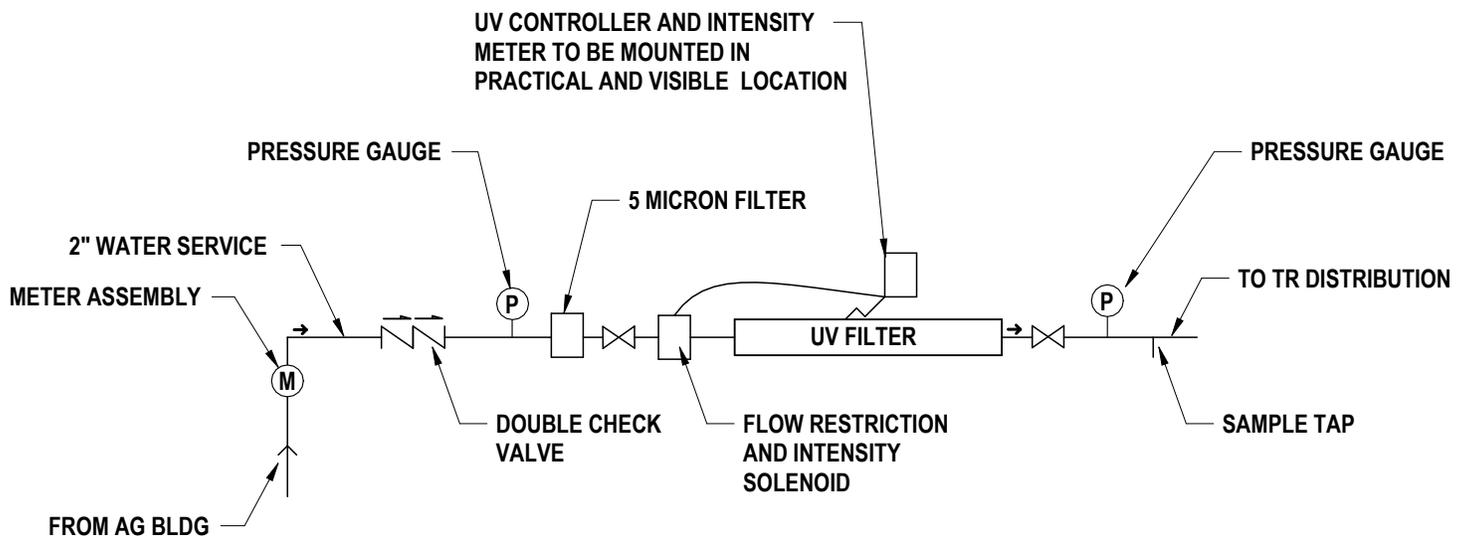
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REVISED FIRST FLOOR DOMESTIC PLAN - AREA C
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AD4-P2
 PROJECT NO: 2716-043

1 REVISED FIRST FLOOR DOMESTIC PLAN - AREA C
 1/4" = 1'-0"



1 UV SCHEMATIC
 1/4" = 1'-0"

UV SCHEMATIC

**2025 CAPITAL IMPROVEMENT PROJECT
 PRATTSBURGH CENTRAL SCHOOL DISTRICT**

1 ACADEMY STREET, PRATTSBURGH NY

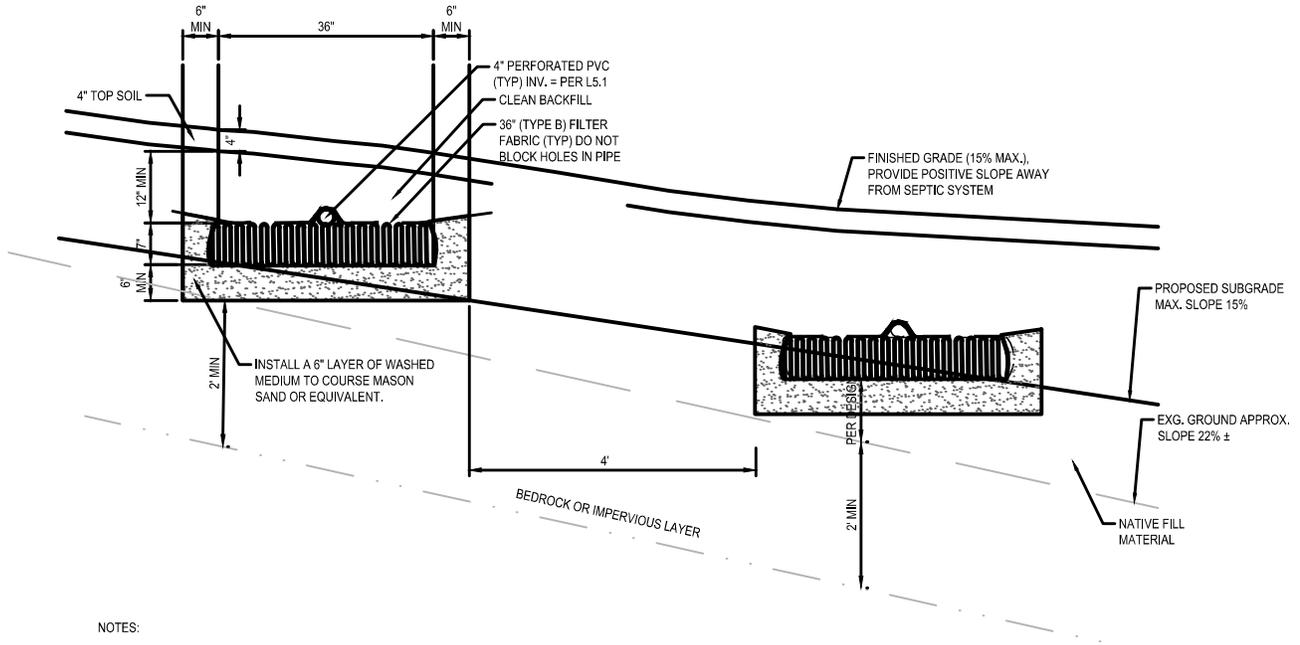
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DATE:
03/12/26

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NOTES:

1. ELJEN IN-DRAIN SHALL BE INSTALLED LEVEL.
2. THE ABSORPTION AREA SHALL BE STAKED AND CLEARLY IDENTIFIED, NO CONSTRUCTION EQUIPMENT SHALL BE DRIVEN INSTALLED DISTRIBUTION LATERALS.
3. DURING CONSTRUCTION, ONLY LIGHTWEIGHT TRACKED OR WIDE TRACKED CONSTRUCTION EQUIPMENT SHALL BE USED FOR MOVING MATERIALS IN AND ABOUT THE ABSORPTION SYSTEM. CONSTRUCTION SHALL OCCUR FROM THE SIDES OF THE ABSORPTION SYSTEM ONLY, ANY CONSTRUCTION EQUIPMENT ACTIVITY WITHIN THE ABSORPTION SYSTEM, HOWEVER SLIGHT, SHOULD BE FOLLOWED BY A THOROUGH ROUGHING OF THE TRENCH SYSTEM PRIOR TO PLACING OF THE FILL MATERIAL..
4. THE PREPARED ABSORPTION ARE SHALL NOT REMAIN EXPOSED TO THE ELEMENTS, PARTICULARLY RAIN, SAND, ELJEN IN-DRAIN SYSTEM, DISTRIBUTION LATERALS, GEOTEXTILE FABRIC, BACKFILL AND TOPSOIL MUST BE PLACED AS SOON AS POSSIBLE.
5. CONTRACTOR SHALL VERIFY DEPTH TO BEDROCK AND PROVIDE A MINIMUM SOIL DEPTH OF 23 INCHES FROM BOTTOM OF TRENCH TO IMPERVIOUS LAYER.
6. THE TOILET ROOM FACILITY SHALL NOT CONTAIN A GARBAGE DISPOSAL UNIT.
7. IT IS RECOMMENDED THAT ALL KITCHEN EFFLUENT BE DISCHARGED TO A GREASE/OIL SEPARATOR PRIOR TO DISCHARGING TO THE SEPTIC TANK.
8. IF A WATER SOFTENER IS UTILIZED WITHIN THE PROPOSED RESIDENCE, THE BACKWASH DISCHARGE PIPING SHALL NOT BE CONNECTED TO THE PROPOSED SEPTIC SYSTEM.
9. IF A GROUNDWATER WELL IS INSTALLED IN LIEU OF UTILIZATION OF THE POTABLE WATER SUPPLY, THE GROUNDWATER WELL MUST HAVE THE FOLLOWING SEPARATION DISTANCES FROM THE PROPOSED SEPTIC SYSTEM (SEE SHADED AREA):

SEPARATION FROM ALL ABSORPTION FIELDS (INC. NEIGHBOR): 100 FT.
 SEPARATION FROM ALL SEPTIC TANKS (INC. NEIGHBOR): 50 FT.

11 ELJEN IN-DRAIN SYSTEM - TYP SECTION
 SCALE: 3/8" = 1'-0"

ELJEN PROFILE DETAIL

**2025 CAPITAL IMPROVEMENT PROJECT
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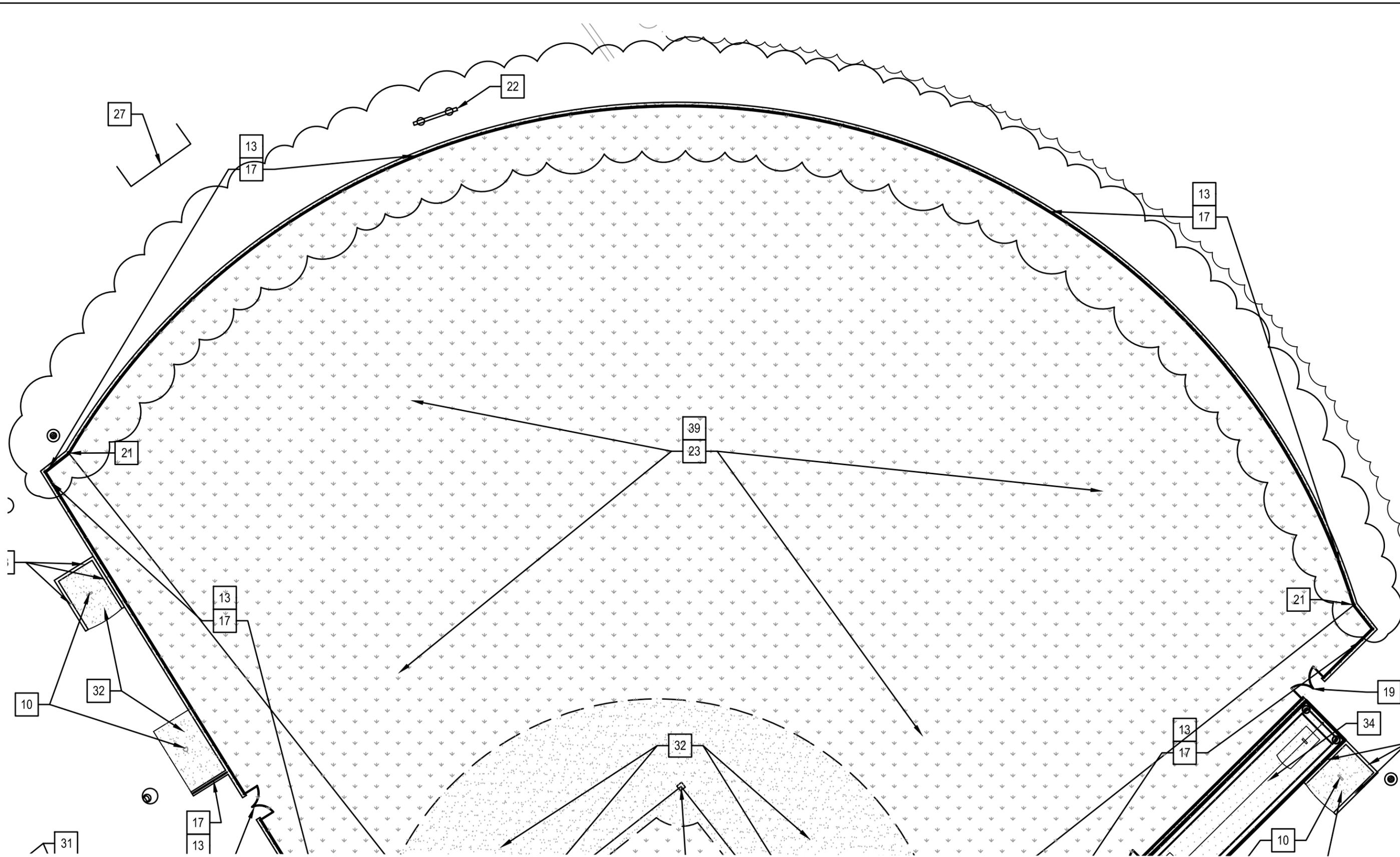
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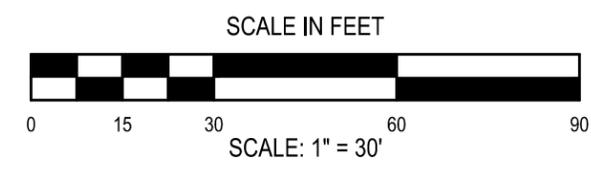
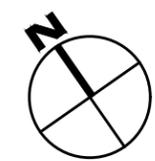
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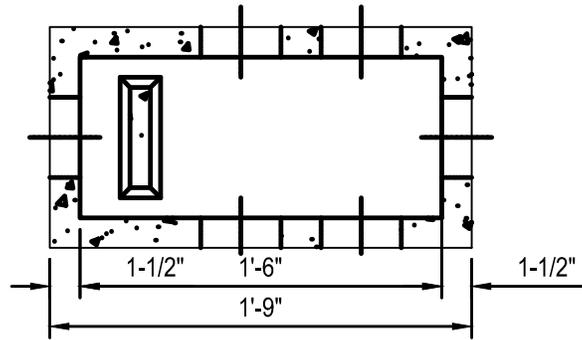
BASEBALL FENCE PERIMETER
 2025 CAPITAL IMPROVEMENT PROJECT
 PRATTSBURGH CSD
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AD4-L2
 PROJECT NO: 2716-043



1 BASEBALL FENCE PERIMETER - AD4
 SCALE: 1" = 30'

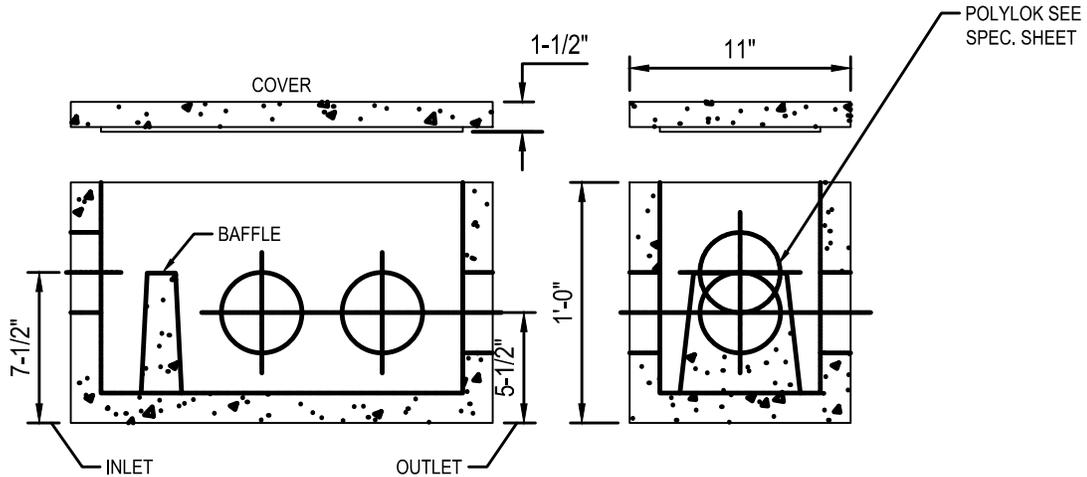




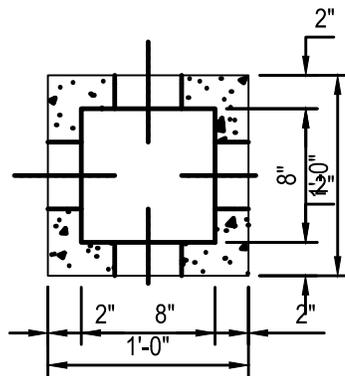
DISTRIBUTION BOX

1 - INLET
5 - OUTLETS

NOTES:
CONCRETE: 5000 PSI @ 28 DAYS
W/ 4x4 12.5 MESH
REF.: #3 REBAR IN COVER
PIPE CONNECTS: POLYLOK
BOX TO BE INSTALLED ON LEVEL GROUND
WITH 12" BED OF SAND AND PEA GRAVEL
12" COVER



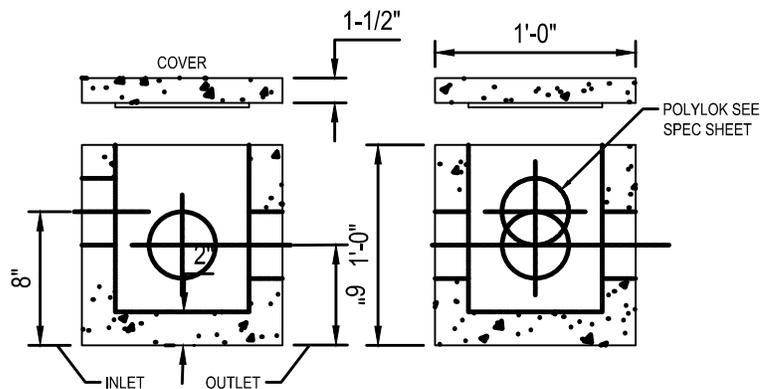
9 5 HOLE DISTRIBUTION BOX DETAIL
SCALE: N.T.S.



DISTRIBUTION BOX

1 - INLET
3 - OUTLETS

NOTES:
ASTM C-478
CONCRETE: 5000 PSI @ 28 DAYS
W/ 4x4 12.5 MESH
REF.: #3 REBAR IN COVER
PIPE CONNECTS: POLYLOK
WITH 12" BED OF SAND AND PEA GRAVEL
12" COVER



7 3 HOLE DISTRIBUTION BOX DETAIL
SCALE: N.T.S.

DISTRIBUTION BOX DETAILS

**2025 CAPITAL IMPROVEMENT PROJECT
PRATTSBURGH CSD**

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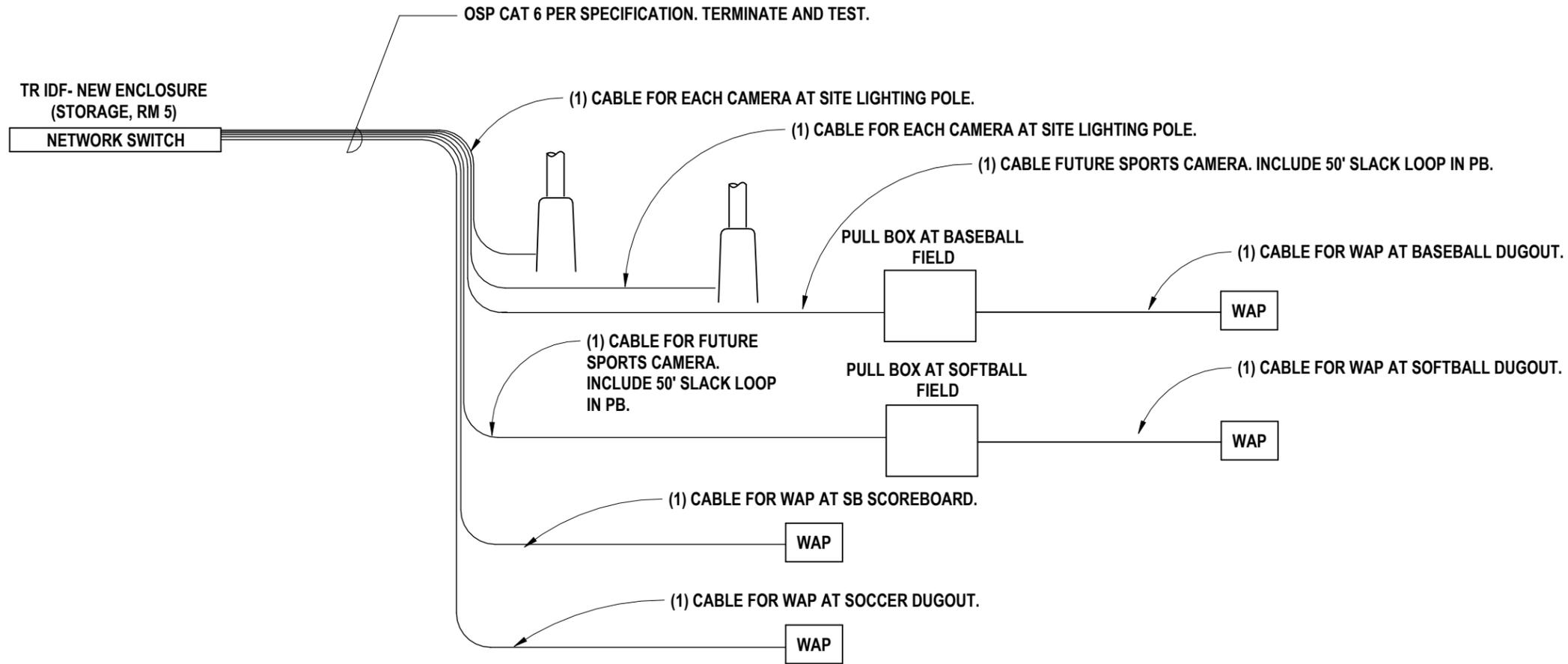
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1 TR OSP DISTRIBUTION DIAGRAM
NTS

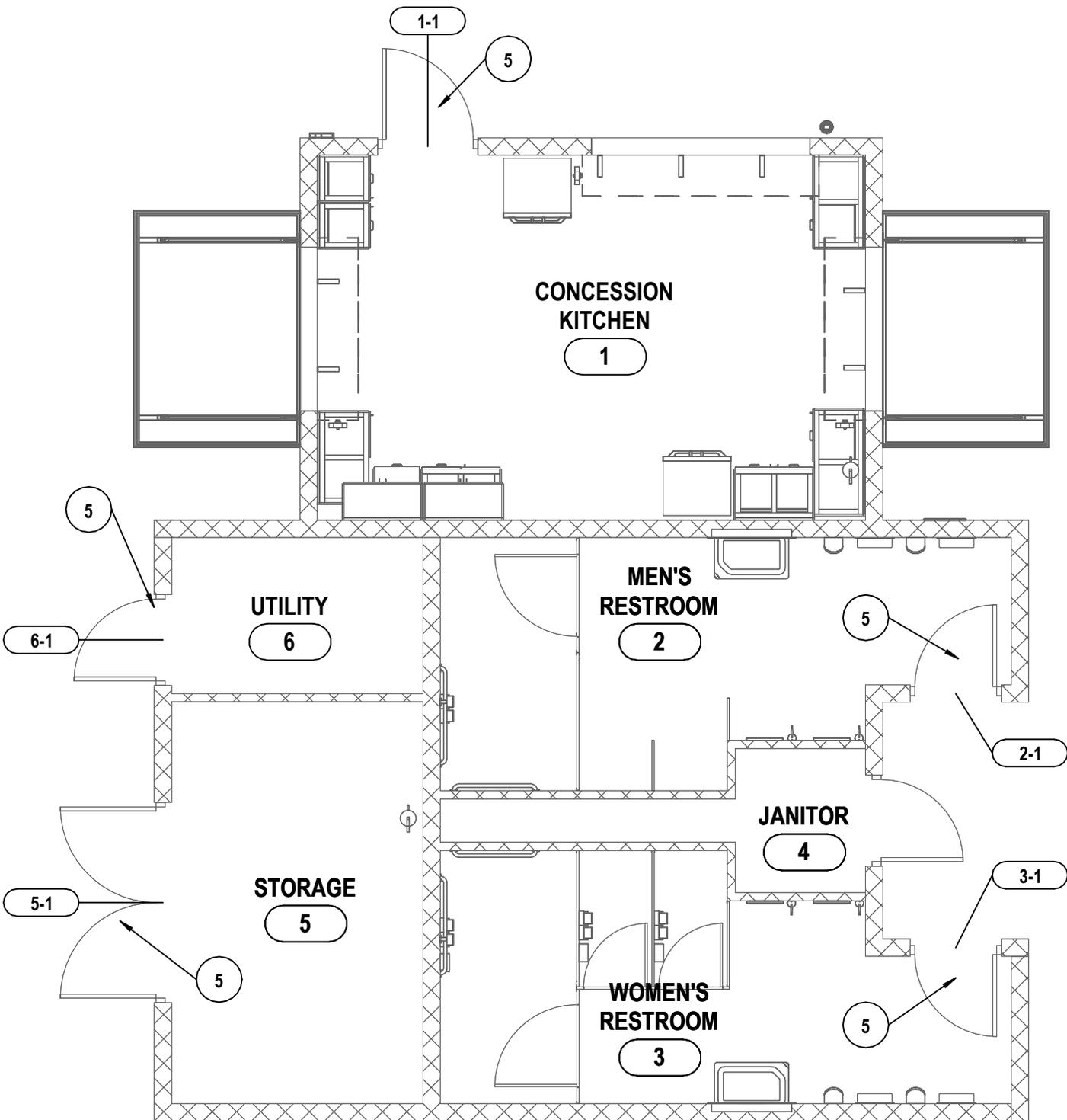
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TR OSP DISTRIBUTION DIAGRAM
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PRATTSBURGH CENTRAL SCHOOL DISTRICT
1 ACADEMY STREET, PRATTSBURGH NY

AD4-T1
PROJECT NO: 2716-043



1 TR ACCESS CONTROL PLAN
 3/16" = 1'-0"

<p>TR ACCESS CONTROL PLAN 2025 CAPITAL IMPROVEMENT PROJECT PRATTSBURGH CENTRAL SCHOOL DISTRICT 1 ACADEMY STREET, PRATTSBURGH NY</p>	<p>HUNT ENGINEERS ARCHITECTS SURVEYORS HORSEHEADS, NY 607 - 358 - 1000 ROCHESTER, NY 585 - 327 - 7949 TOWANDA, PA 570 - 265 - 4868</p>	<p>AD4-T2</p> <p>DATE: 03/13/26</p> <p>PROJECT NO: 2716-043</p>
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GENERAL ROOF NOTES:

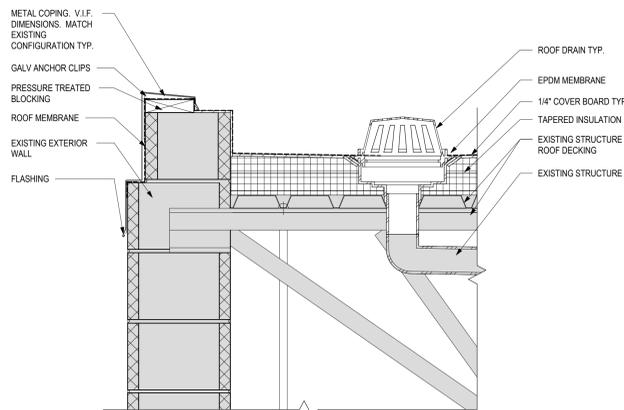
- A ALL WORK SHALL CONFORM TO THE NEW YORK STATE BUILDING CODE AND ADOPTED REFERENCE STANDARDS.
- B ALL WORK ON OR NEAR EXISTING ROOFING SCHEDULED TO REMAIN IN PLACE, SHALL BE REFORMED IN SUCH A MANNER TO MAINTAIN ALL EXISTING WARRANTIES.
- C THE GENERAL TRADES CONTRACTOR SHALL PROTECT EXISTING CONSTRUCTION TO REMAIN AS REQUIRED DURING DEMOLITION. ALL EXISTING CONSTRUCTION TO REMAIN SHALL BE RESTORED TO ITS ORIGINAL CONDITION, UNLESS NOTED OTHERWISE.
- D THE GENERAL TRADES CONTRACTOR SHALL INSTALL AND MAINTAIN TEMPORARY ROOFING TO ENSURE WEATHER TIGHT CONDITIONS UNTIL SUCH TIME THE PERMANENT ROOFING SYSTEM IS INSTALLED.
- E AN ACCEPTABLE TAPERED INSULATION SHOP DRAWING SUBMISSION IS REQUIRED TO IDENTIFY INSULATION LAYOUT, DRAINAGE PATTERN, SLOPE AND AVERAGE "R" VALUE.
- F THE CONTRACTOR SHALL PROVIDE AND INSTALL ALL MATERIALS TO MAKE SMOOTH TRANSITIONS AT ROOF EDGES AND INTERSECTIONS. TAPERED EDGE STRIPS ARE REQUIRED AT PERIMETER.
- G THE GENERAL TRADES CONTRACTOR SHALL REFER TO THE HVAC DRAWINGS AND COORDINATE WITH THE HVAC CONTRACTOR FOR QUANTITY AND LOCATIONS OF PENETRATIONS THROUGH ROOF FOR EQUIPMENT RAILS, CURBS AND BOOTS.
- H THE GENERAL TRADES CONTRACTOR SHALL PROVIDE FLASHING AT ALL PENETRATIONS.
- I THE HVAC CONTRACTOR SHALL PROVIDE EQUIPMENT RAILS AND CURBS FOR MECH. EQUIP. COORDINATE EXACT LOCATION WITH HVAC CONTRACTOR. THE GENERAL TRADES CONTRACTOR SHALL PROVIDE AND INSTALL PIPE CURBS OF REQUIRED SIZE.
- J THE GENERAL TRADES CONTRACTOR SHALL PROVIDE ADDITIONAL BLOCKING AT ALL ROOF PENETRATIONS WHERE REQUIRED TO PROVIDE A MINIMUM HEIGHT OF CURB TO BE 12" ABOVE THE ROOF SURFACE. THE GENERAL TRADES CONTRACTOR SHALL INSTALL NEW CURBS AND BOOT'S PROVIDED BY HVAC CONTRACTOR. REFER TO HVAC DRAWINGS.
- K REFER TO THE DRAWINGS FOR WORK BY HVAC AND ELECTRICAL CONTRACTORS ASSOCIATED WITH EXISTING EQUIPMENT REMOVALS.
- L G.C. TO PROVIDE ANY ADDITIONAL BLOCKING AND ANY ASSOCIATED ACCESSORIES REQUIRED BASED ON INSULATION LAYOUT.
- M PREVENT DIRT AND ROOFING DEBRIS FROM ENTERING THE ROOF DRAININGS AND DRAIN LINES (LEADERS) DURING CONSTRUCTION. THE CONTRACTOR SHALL SNAKE DRAIN LINES AT COMPLETION OF WORK.
- N ALL ROOF OF MECHANICAL EQUIPMENT SHALL BE TESTED PRIOR TO REMOVAL. IN THE PRESENCE OF THE OWNERS REPRESENTATIVE. FOLLOWING REINSTALLATION, EQUIPMENT WILL AGAIN BE TESTED IN THE PRESENCE OF THE OWNERS REPRESENTATIVE.
- O IF EXISTING ROOF DECK OR STRUCTURAL SYSTEM DAMAGE IS SUSPECTED, NOTIFY THE OWNER'S REPRESENTATIVE IMMEDIATELY.
- P MECHANICAL UNITS THAT REQUIRED MULTIPLE PIPE AND/OR WIRE TO USE DETAIL 18HS-A1.16

ROOF DRAWING NOTES:

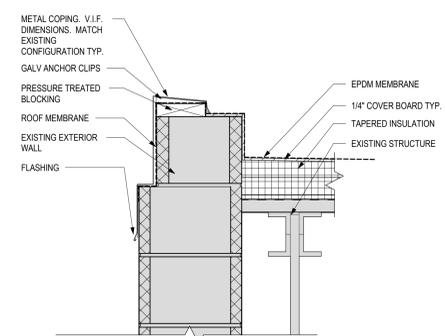
- 1 REMOVE ALL EDGE METAL COPING IN ITS ENTIRETY. REMOVE ALL BLOCKING AND REPLACE WITH NEW PRESSURE TREATED BLOCKING AS REQUIRED. BLOCKING MAY REMAIN IF IN SUITABLE CONDITION. PROVIDE NEW EDGE METAL COPING AT ENTIRE PERIMETER TYP. NEW COPING TO MATCH EXISTING CONFIGURATION TYP.
- 2 COMPLETE RE-ROOF REFER TO THE ROOF LEGEND FOR ROOFING SYSTEM DETAILS. VERIFY EXISTING ROOF SLOPE IN FIELD.
- 3 EXISTING ROOF SKYLIGHT TO REMAIN. PARTIALLY REMOVE AND REINSTALL AS REQUIRED FOR ROOF SCOPE.
- 4 EXISTING ROOF VENTS TO REMAIN. EXTEND TO 18" ABOVE NEW ROOFING MATERIAL AS REQUIRED. V.I.F. FOR EXACT LOCATION.
- 5 PARTIALLY REMOVE AND REINSTALL ROOF DRAIN AS REQUIRED FOR ROOFING SCOPE COMPLETION. V.I.F. FOR EXACT LOCATION.
- 6 MECHANICAL CONTRACTOR TO REMOVE AND REINSTALL MECHANICAL EQUIPMENT. DISCONNECT, EXTEND AND RECONNECT DUCTWORK, GAS, AND CONTROLS TO ACCOMMODATE ROOF SCOPE. ROOF CURB TO REMAIN.
- 7 MECHANICAL CONTRACTOR TO REMOVE AND REINSTALL EXHAUST FAN. DISCONNECT, EXTEND AND RECONNECT DUCTWORK, AND CONTROLS TO ACCOMMODATE ROOF SCOPE. ROOF CURB TO REMAIN.
- 8 EC TO DISCONNECT AND RECONNECT AS REQUIRED FOR ROOFING SCOPE COMPLETION. V.I.F. FOR EXACT LOCATION.
- 9 MC TO DISCONNECT AND RECONNECT AS REQUIRED FOR ROOFING SCOPE COMPLETION. V.I.F. FOR EXACT LOCATION.
- 10 PROVIDE TAPERED INSULATION TO ACHIEVE MIN. 1/4" PER FOOT SLOPE AT HIGH SIDE OF ROOF CURBS AND HVAC EQUIPMENT AS SHOWN ON PLAN.

ROOF LEGEND:

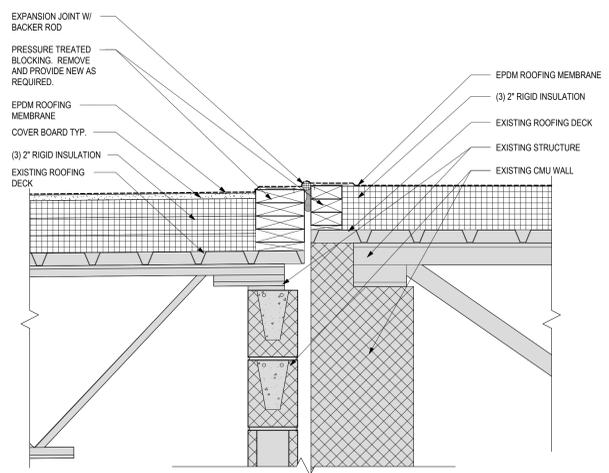
-  FULLY ADHERED EPDM MEMBRANE ON 1/4" COVERBOARD OVER THREE (3) LAYERS OF 2" RIGID INSULATION. INSULATION TO BE FULLY ADHERED TO ONE LAYER OF A 40 MIL RUBBERIZED ASPHALTIC VAPOR RETARDER OVER 1/2" ROOF UNDERLAYMENT MECHANICALLY FASTENED TO METAL DECK. SYSTEM TO ACHIEVE AN AVERAGE R VALUE OF 36; NO LESS THAN R20 MIN PER IECC. SLOPE STRUCTURE BELOW TO ACHIEVE POSITIVE FLOW TO ROOF DRAINS AT A MINIMUM SLOPE OF 1/8" PER FOOT.
-  FULLY ADHERED EPDM MEMBRANE ON 1/4" COVERBOARD OVER THREE (3) LAYERS OF 2" RIGID INSULATION. INSULATION TO BE FULLY ADHERED TO ONE LAYER OF A 40 MIL RUBBERIZED ASPHALTIC VAPOR RETARDER OVER 1/2" ROOF UNDERLAYMENT MECHANICALLY FASTENED TO METAL DECK. SYSTEM TO ACHIEVE AN AVERAGE R VALUE OF 36; NO LESS THAN R20 MIN PER IECC. PROVIDE TAPERED INSULATION OVER THE BASE MINIMUM INSULATION THICKNESS TO ACHIEVE POSITIVE FLOW TO ROOF DRAINS AT A MINIMUM SLOPE OF 1/4" PER FOOT.
-  EXISTING ROOF SYSTEM TO REMAIN
-  NEW ROOF DRAIN
-  VENT PIPE
-  ARROW INDICATES DIRECTION OF SLOPE FOR THE ROOF STRUCTURE OR TAPERED INSULATION (SEE STRUCTURAL DRAWINGS)
-  +8" HP INSULATION THK @ HIGH POINT
-  +2" LP INSULATION THK @ LOW POINT



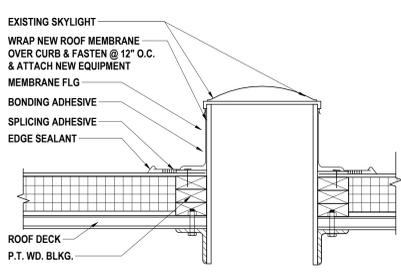
6 ROOF EDGE @ ROOF DRAIN
1 1/2" = 1'-0"



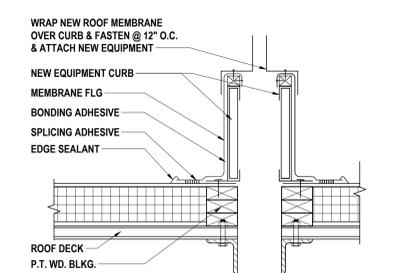
5 ROOF EDGE TYPICAL
1 1/2" = 1'-0"



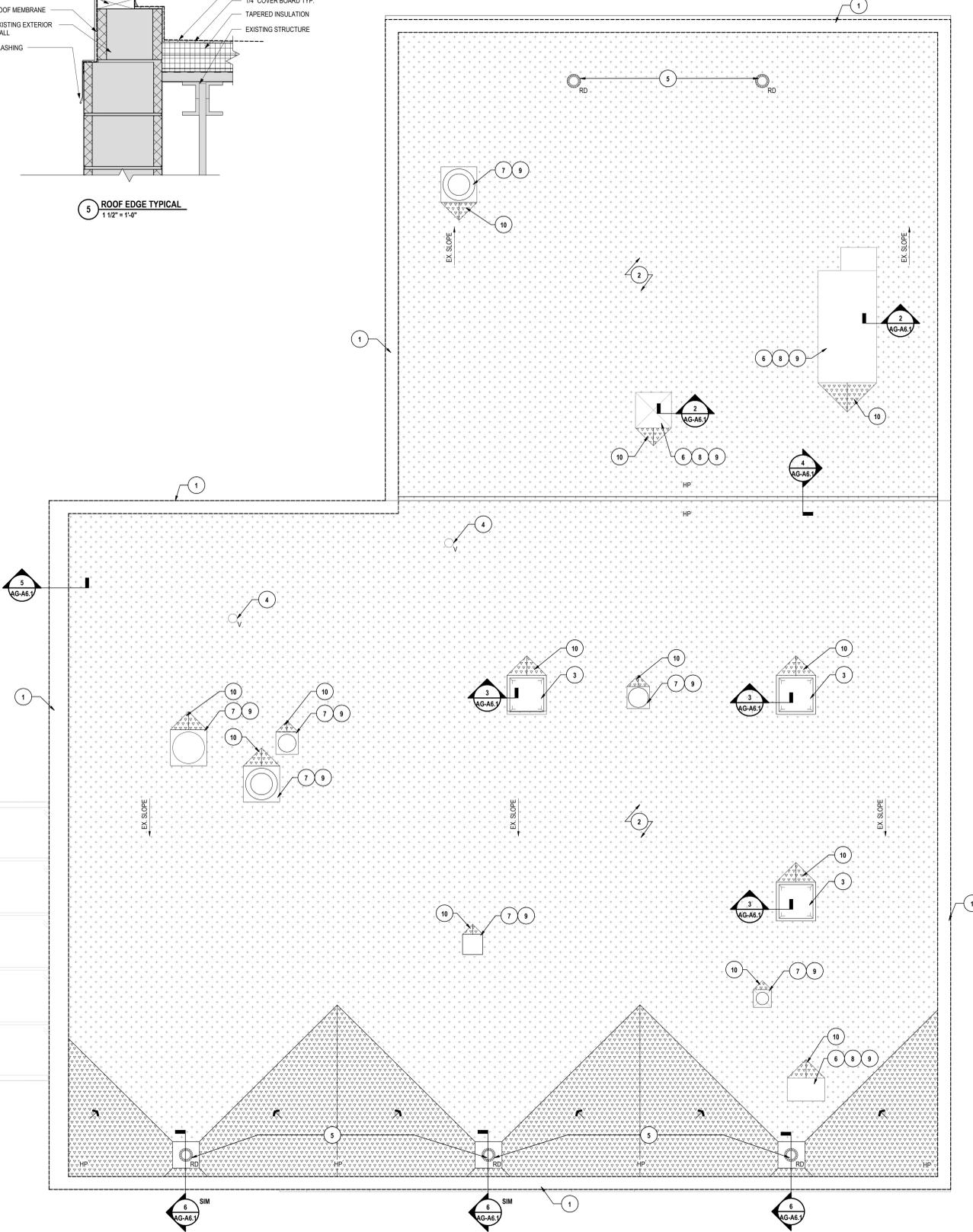
4 EXPANSION JOINT DETAIL
1 1/2" = 1'-0"



3 SKYLIGHT CURB DETAIL
1 1/2" = 1'-0"



2 TYPICAL CURB DETAIL
1 1/2" = 1'-0"



1 ROOF PLAN
1/4" = 1'-0"

HUNT ENGINEERS ARCHITECTS | SURVEYORS
 HORSEHEADS, NY 607-568-1088 | ROCHESTER, NY 585-637-7668 | TOWANDA, PA 570-265-4668
 BINGHAMTON, NY 607-798-8881 | ALBANY, NY 607-798-4801
 WWW.HUNTEAS.COM | NY CERTIFICATE NO. 0016250 | PA CERTIFICATE NO. TSC220313464-1

ROOF PLAN
 2025 CAPITAL IMPROVEMENT PROJECT
 PRATTSBURGH CENTRAL SCHOOL DISTRICT
 PRATTSBURGH, NEW YORK

AG-A6.1
 PROJECT NO: 2716-043

DESCRIPTION OF REVISION:
 1 ISSUED FOR BID
 2 AUGUST 14

DATE: 12/18/2025
PHASE: CD

DRAWN BY: TSK
CHECKED BY: JDZ
DATE: 12/18/2025
PHASE: CD

THIS IS A MODIFICATION OF THE LAW FOR ANY PERSON TO MAKE UNAUTHORIZED ALTERATIONS OR ADDITIONS TO PLANS DRAWING, CALCULATIONS, SPECIFICATIONS, INSTRUMENTS OR SURVEYORS' SEAL.