

DICK VAN DYKE ADDICTION TREATMENT CENTER – KITCHEN & DINING ROOM ADDITION INCLUDING ASBESTOS ABATEMENT PROJECT

1330 County Rd. #132
Ovid, New York, 14521

Prepared for: Dormitory Authority State of New York (DASNY)
Prepared by: Colliers Engineering & Design
280 East Broad Street - Suite 200
Rochester, New York 14604

ADDENDUM # 002

Date Issued: September 23, 2026

The following additions, changes, and clarification modify the original bidding documents, and receipt of this addendum shall be acknowledged in the space provided on the proposal form. This addendum contains changes to the requirements of the Contract Documents - Project Manual/ Specifications and Construction Document Drawings - which have been issued to date. Such changes are to be incorporated into the Contract Documents and shall apply to the Work with the same meaning and force as if they had been included in the original documents. Wherever this Addendum modified a portion of a paragraph of the Project Manual, or portion of any Drawings, the remainder of the paragraph or Drawing shall remain in force.

RFI Questions/Responses:

Item No. 001 – 09/17/26

1. Need some clarification on stud walls and wall type.
 - a. M21 wall type is detailed as 1-5/8" metal stud and 5/8" gyp and is being called 3". Should the stud be 2-1/2"?

RESPONSE: Correct, wall type M21 should consist of 2-1/2" metal studs and 5/8" gyp. board.

- b. Detail 6 on A501, the metal framing is being called out as 3-5/8" stud. On A401, this wall is being called for an M21,2 wall type which is 1-5/8", Please advise

RESPONSE: A401 is correct, wall type should be M21 with 2-1/2" metal studs.

2. Which contract is responsible for excavation & backfill for division 22 sanitary piping & grease interceptor?
 - **RESPONSE: The General Construction contract is responsible.**
3. Need verified the size of gas line to existing combi oven?
 - **RESPONSE: A 1-1/4" LPG line for the oven has been provided per the contract drawings.**
4. Need verified additional utilities to existing hot food table?
 - **RESPONSE: A floor sink for the indirect drain (P101, keynote 4) has been provided per the contract drawings. Refer to FSE-3 for electrical requirements.**
5. Need verified additional utilities to existing icemaker in regards to if it needs a drain line or not?
 - **RESPONSE: A floor sink for the indirect drain line the icemaker required (P101, keynote 7) and a cold water connection (P121, keynote 6) have been provided. Refer to FSE-3 for electrical requirements.**
6. Are you able to tell us a list of changes that were made between the EOI drawing set and current bid drawings?
 - **RESPONSE: DASNY advertised for solicitation of Expressions of Interest from Qualified Construction Firms for the General Construction contract. The EOI packages were due on 8/17/2026. DASNY did not receive the required number of EOI's to proceed with this procurement method. To review the changes, bidders shall obtain new bid documents.**

Item No. 002 – 09/18/26

1. Ask for loading dock and foundation wall thicknesses and slab thicknesses at areas being removed.
 - **RESPONSE: There are no existing drawings of the loading dock foundation and slab to be removed. Include removal of 50 LF of 2'-0" wide x 1'-0" thick concrete footing, 50 LF of 8'-0" wide x 4'-0" tall concrete wall, and 175 square feet of 6" slab on grade. Unit prices should be included for each of these items for any additional removals required, these quantities are assumptions.**

Item No. 003 – 09/21/26

1. During the prebid meeting, it was mentioned to pay close attention to the Geotechnical report. Please advise if the Geotech report was released in the specifications. I could not find it. If it was not released, please provide the geo report.
 - **RESPONSE: Please see attached Geotechnical Sections at end of addendum:**
 - FD Geotechnical Evaluation Dick Van Dyke Ctr 25.05826
 - FD Addendum to Geotech Eval 25.05826-1
 - FD Addendum #2 for Geotechnical Evaluation Dick Van Dyke Ctr 25.05826

2. Provide information on software required by owner for project scheduling.
 - **RESPONSE:** Utilization of the Primavera P6 or P3 Primavera Project Planner operating system is recommended. The contractor can propose another critical path method scheduling system for review/approval by owner

Item No. 004 – 09/22/26

1. Is there a BMS controls system currently in the facility? If so, who is the contact for that?
 - **RESPONSE:** The current BMS system is Daikin and the contact is Modular Comfort Systems.
2. On P-100 and P-101, no vents are shown for Grease Interceptor or Hand Sinks. Is this required?
 - **RESPONSE:** The drainage system serving the hand sinks are part of the combination waste & vent system. The vents are shown on drawings P100 & P101. The grease interceptor is located on the exterior of the building. Venting per manufacturer's instructions is required.
3. We would like to confirm that the Site Contract is responsible for SAN/GW/Sewer piping after the grease interceptor shown on C110 & P100.
 - **RESPONSE:** Per drawing P100, site contract is responsible for SAN/GW/Sewer piping after grease interceptor.

Item 05: Refer to Contract Document Project Manuals – Substitute the following specifications:

1. Section "Bidding Requirements for Construction – Information for Bidders"
 - a. **MODIFY:** "A mandatory Pre-Award Meeting for the apparent low bidder will be held on October 21st, 2026 at DASNY's Rochester Office..."

END OF ADDENDUM #002



Foundation Design, P.C.

SOIL • BEDROCK • GROUNDWATER

March 19, 2025

Colliers Engineering & Design
280 East Broad Street, Suite 200
Rochester, New York 14604

Attention: Colleen R. Braun, RA, NCARB, AIA
Project Architect

Reference: Dick Van Dyke Treatment Center – Kitchen Expansion
1330 County Route 132, Ovid, New York
Geotechnical Evaluation, 25.05826

Dear Ms. Braun,

This letter report outlines our Geotechnical Evaluation for the referenced project. We understand that the treatment center is planning a building addition and site improvements to the existing Dick Van Dyke Treatment Center in Ovid, New York. The building improvements include a 500 square foot addition to the northeast corner of the structure; we have assumed that this will be a single story, steel-framed structure with a full basement. The site improvements include expanding the parking lot by 15 feet and reconstructing the existing parking lot to support tractor trailer deliveries. We base this evaluation on our review of U.S.G.S. and N.Y.S.D.O.T. topographic and geologic mapping; soil boring exploration; previous work on site; laboratory testing; and consultation with the design team. We intend the design recommendations contained in this report to be used exclusively for this project.

Site Location

The Dick Van Dyke Treatment Center is located at 1330 County Road 132 in Ovid, New York. The Dick Van Dyke Treatment center is located within the former Willard Drug Treatment Campus. Seneca Lake is located to the west with tennis courts to the north. Hardwoods border the property to the east and south. Grades decrease across the site, sloping from the east to the west by approximately 10 feet, with minimal elevation change from north to south. A *General Location Plan* on 2023 U.S.G.S topographic mapping showing the site location is attached.

Exploration

We retained Northeast Specialized Drilling, Inc. to perform soil borings B25-1 through B25-6 on March 10, 2025. Northeast Specialized Drilling provided a truck mounted CME 55 drill rig for the exploration work. The soil borings were advanced using hollow stem auger casing. The drillers recovered split spoon soil samples continuously to ten feet (twelve feet at B25-2 only) using the Standard Penetration Test (SPT) method and then at five-foot intervals thereafter until completion. We terminated the soil borings at depths ranging from 6.0 feet to 20.5 feet below grade. We located the boring locations, measuring off existing site features. Boring locations within the asphalt parking lot were patched with cold patch upon completion. Our staff logged the subsurface profiles and collected representative soil samples. The *Boring Location Plan* and the soil boring logs are enclosed.

We performed lab testing consisting of five moisture content tests and five sieve analyses on representative samples. The laboratory test results are attached to this report.

Site Geology

The following interpretations of the soil, bedrock, and groundwater conditions are based on the new soil borings, our site observations, past experience in the area, and consulting with the design team. See the attached soil boring logs for soil descriptions at the test locations. Variations from the inferred profile are possible. Contact us immediately if variations are found during construction so we may evaluate the impact on our recommendations.

A typical soil profile consists of asphalt/topsoil then loamy till with shale fragments. The asphalt thickness was recorded as 5.0 inches at each location. Beneath the asphalt was subbase consisting of crushed stone with little sand. A sieve analysis of a subbase sample within the parking lot classified as GM (silty gravel with sand) in the Unified Soil Classification System (USCS). The moisture content of this sample was found as 2.2 percent, slightly dry of optimum for compaction. Firm, possible fill material was encountered beneath the subbase at boring location B25-1. Sieve analysis of the possible fill material classified as ML (sandy silt with gravel) in the USCS. The moisture content of this fill was recorded as 14.3 percent, wet of optimum for compaction. The loamy till beneath the subbase and earth fill consisted of silt with various amounts of sand and gravel. Sieve analyses of representative samples of the loamy till classified as ML (sandy silt) and SM (silty sand with gravel) in the USCS. Moisture contents of the loamy till were between 8.5 and 11.2 percent, near optimum for compaction. Representative N-values of the till material classified it as firm to very dense.

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We did not encounter bedrock during our recent or past exploration. Geologic mapping indicates bedrock is of the Sonyea Formation consisting of shales that are often highly weathered and fractured in the upper 5 to 10 feet. The bedrock is over 25 feet below grade and will not impact the project.

Groundwater was not encountered during our recent or past exploration. We expect groundwater will tend to 'perch' above the dense glacial till soils. Expect groundwater levels to fluctuate with seasonal changes.

Conclusions

We conclude that the proposed addition can be supported on a spread footing foundation system. We recommend designing the new foundations to bear on the native soil or new structural fill.

Based on the above, we make the following specific recommendations:

1. Clear, grub, and remove any asphalt and unsuitable soil from the proposed kitchen addition footprint. Relocate any existing utilities in the area. Foundation Design, P.C. should observe a proof-roll of the subgrade. The contractor shall supply a loaded ten-wheel truck for this operation. Rework or replace as directed areas that rut, weave, quake, or are otherwise deemed unsuitable by Foundation Design, P.C.
2. The on-site native soil, free of organics and debris, can be used for backfilling of the addition. The native soil is silty and moisture sensitive, it may be wet of optimum for compaction and require drying to meet compaction requirements. Alternatively, use an imported fill conforming to the requirements of N.Y.S.D.O.T. Item 203.07, Select Granular Fill, to balance the site. Submit imported material sources to Foundation Design, P.C. for review and approval on a case-by-case basis. Maintain good surface drainage.

We define structural fill as fill under or around the proposed footings, floor slabs, pavements, and for sidewalks. Place and moisture condition structural fill to within two percent of optimum moisture for compaction. Compact structural fill to at least 95 percent of maximum dry density within two percent of the optimum moisture as determined by ASTM D1557, the Modified Proctor Method. Compact other fill to 90 percent of Modified Proctor or as otherwise determined by the site engineer or landscape architect. Place fill in lifts not-to-exceed eight inches in loose thickness.

3. Support the addition on spread footings bearing on the undisturbed native soil or the new structural fill material. Design the new footings based on a net new allowable bearing pressure of 3,000 psf. This bearing pressure is intended to limit settlements to under half an inch, a reasonable criterion for additions next to existing structures. Design exterior footings at a frost free depth of 48 inches. Design footings based on a minimum size of 24 inches (square or wide). New foundations that abut existing footings should bear at the same elevation. Hand clean all loose soil and from the bearing surface. We suggest using a shovel equipped with a ditching bucket for the foundation excavations. The intent is to limit

disturbance of the bearing surface during excavation. Install sumps and pumps to control water as needed to prevent destabilization of the bearing surface. Consider placing 2 to 3 inches of crushed stone for a working surface in foundation trenches where persistent water seepage occurs.

4. Place six inches of N.Y.S.D.O.T. Item 304.12 (crusher-run stone) under the slab-on-grade. Proof-roll the slab subgrade prior to subbase placement to check for loose pockets of fill and replace pockets with structural material. Design the floor slabs bearing over the new structural fill based on a Modulus of Subgrade Reaction, K_{vi} , of 175 psi/in at the bottom of slab/top of stone. The architect and/or structural engineer should review the proposed interior floor finishes and humidity control requirements to determine whether a vapor barrier is appropriate under the floor slab and if so, where it should be installed. See the American Concrete Institute Document 302.1R, *Concrete Floor and Slab Construction*, for more information.
5. NYS Building Code identifies various seismic design criteria for this project. We identify the site as having a Site Classification of C (Very Dense Soil) per ASCE-7-16. We recommend using the following seismic design parameters.

Table No. 1 – Seismic Design Parameters					
Spectral Response Acceleration		MCER Response Spectrum		Design Spectral Response Acceleration	
S_s	S_1	S_{MS}	S_{M1}	S_{Ds}	S_{D1}
0.132g	0.046g	0.172g	0.070g	0.114g	0.046g

6. For the parking lot expansion, the pavement subgrade will be the in-place native soil or structural fill. With the poor pavement conditions and contaminated subbase, we recommend a complete box out of the pavement section. We recommend that the new pavement areas be boxed-out to the subgrade, with the top eight-inches re-compacted to the structural standards and the subgrade proof-rolled under the observation of Foundation Design, P.C. Re-compact or replace areas that rut, weave, quake, or are otherwise deemed unstable under proof-rolling traffic.

The tested subbase sample from B25-4, was found to have 13.0 percent fines. N.Y.S.D.O.T. regulations suggest that subbase material with more than 10.0 percent fines are unsuitable for reuse as subbase material, but that it can be used as structural fill. Plan to install a new subbase section depending on the provided pavement sections below.

Slope both the pavement surface and subgrade at slopes of at least 2.0 percent to facilitate water flow toward the stormwater system and to prolong the pavement life. Install weeps to allow water out of the subbase and into the drainage system (or to daylight).

Install the Standard and Heavy Duty pavement sections tabulated below. Use the Standard Duty Section in car parking spaces and the Heavy Duty Section in the access driveway and drive lanes that receive delivery truck traffic.

Table No. 2 – Standard Duty Pavement Section		
1.5"	Asphalt Top	NYSDOT Item 403.198902
2.5"	Asphalt Binder	NYSDOT Item 403.138902
9.0"	Crusher-run Stone Subbase	NYSDOT Item 304.12
	Subgrade	Approved Proof-Roll

Table No. 3 – Heavy Duty Pavement Section		
1.5"	Asphalt Top	NYSDOT Item 403.198902
2.5"	Asphalt Binder	NYSDOT Item 403.138902
3.0"	Asphalt Base	NYSDOT Item 403.118902
12.0"	Crusher-run Stone Subbase	NYSDOT Item 304.12
	Subgrade	Approved Proof-Roll

It is our experience that flexible pavements do not perform well in locations where repetitive traffic occurs (i.e., loading docks, dumpster locations, etc.). We recommend installing rigid concrete pavement in these areas.

7. Perform the trenching and excavating work in accordance with NYS Building Code and OSHA safety standards. The contractor is responsible for determining what measures are required to meet these standards. Under no circumstances should excavation slopes be steeper than 1 horizontal on 1 vertical. It is our opinion that the foundation and utility excavation work can be achieved with 'normal' excavating equipment capable of achieving the desired depths. Remove water that accumulates in open excavations using sumps and pumps.
8. The NYS Building Code requires special inspection services. We recommend the following inspections and ask that you include them on the List of Special Inspections developed as part of the Building Permitting Process:
 - We recommend that Foundation Design, P.C. make periodic site visits to confirm that the actual soil conditions are as expected and provide recommendations to address areas where conditions differ. Specifically, we should observe the following:
 - A. IBC Table 1705.6 – 5: Observe/approve proof-rolling of addition subgrade and parking lot areas prior to new fill placement.
 - B. IBC Table 1705.6 – 1: Observe/verify that the materials below the shallow spread footings are adequate to achieve the design bearing pressure.
 - C. Review the soils-related test results and consult on compaction-related issues as they arise. Review and approve of structural fill materials.
 - The owner should retain an independent test agency to perform full-time observation of mass fill placement and part-time observation of the backfilling operations.

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Specific testing requirements follow:

- A. Conduct at least one density test on each 2,500 square feet of mass fill placed, with at least one test for each partial lift.
- B. Conduct at least one density test on each 50 lineal feet of foundation/utility trench, with at least one test for each partial lift.
- C. Place fill in lifts not-to-exceed eight inches in loose thickness.
- D. Compact structural fill to at least 95 percent of the maximum dry density as determined by the Modified Proctor Test (ASTM D-1557).

Attached to the end of this report is a Geoprofessional Business Association paper entitled *Important Information about This Geotechnical Engineering Report*. It describes how we intend this report to be used. We will continue to work cooperatively with you, other project principals, and interested parties to achieve win/win solutions that benefit all.

As part of our design phase services, a test pit along the existing foundation to determine its depth and type is required. At the time of this report submission, we are still waiting for asphalt plants to open for the year so that proper repair of the test pit excavation area can be conducted. We will continue to work with you, the client, to complete our design phase and supply an addendum upon completion of the test pit. Forward a copy of the near final plans and specifications for our review, comment, and use during construction.

It has been a pleasure to work with you on this project and we look forward to hearing from you as it proceeds towards construction.

Sincerely,

FOUNDATION DESIGN, P.C.



Brennan Valentino, P.E.
Staff Engineer



Ryan J. Radford, P.E.
Vice President

Important Information about This Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

The Geoprofessional Business Association (GBA) has prepared this advisory to help you – assumedly a client representative – interpret and apply this geotechnical-engineering report as effectively as possible. In that way, you can benefit from a lowered exposure to problems associated with subsurface conditions at project sites and development of them that, for decades, have been a principal cause of construction delays, cost overruns, claims, and disputes. If you have questions or want more information about any of the issues discussed herein, contact your GBA-member geotechnical engineer. Active engagement in GBA exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project.

Understand the Geotechnical-Engineering Services Provided for this Report

Geotechnical-engineering services typically include the planning, collection, interpretation, and analysis of exploratory data from widely spaced borings and/or test pits. Field data are combined with results from laboratory tests of soil and rock samples obtained from field exploration (if applicable), observations made during site reconnaissance, and historical information to form one or more models of the expected subsurface conditions beneath the site. Local geology and alterations of the site surface and subsurface by previous and proposed construction are also important considerations. Geotechnical engineers apply their engineering training, experience, and judgment to adapt the requirements of the prospective project to the subsurface model(s). Estimates are made of the subsurface conditions that will likely be exposed during construction as well as the expected performance of foundations and other structures being planned and/or affected by construction activities.

The culmination of these geotechnical-engineering services is typically a geotechnical-engineering report providing the data obtained, a discussion of the subsurface model(s), the engineering and geologic engineering assessments and analyses made, and the recommendations developed to satisfy the given requirements of the project. These reports may be titled investigations, explorations, studies, assessments, or evaluations. Regardless of the title used, the geotechnical-engineering report is an engineering interpretation of the subsurface conditions within the context of the project and does not represent a close examination, systematic inquiry, or thorough investigation of all site and subsurface conditions.

Geotechnical-Engineering Services are Performed for Specific Purposes, Persons, and Projects, and At Specific Times

Geotechnical engineers structure their services to meet the specific needs, goals, and risk management preferences of their clients. A geotechnical-engineering study conducted for a given civil engineer

will not likely meet the needs of a civil-works constructor or even a different civil engineer. Because each geotechnical-engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client.

Likewise, geotechnical-engineering services are performed for a specific project and purpose. For example, it is unlikely that a geotechnical-engineering study for a refrigerated warehouse will be the same as one prepared for a parking garage; and a few borings drilled during a preliminary study to evaluate site feasibility will not be adequate to develop geotechnical design recommendations for the project.

Do not rely on this report if your geotechnical engineer prepared it:

- for a different client;
- for a different project or purpose;
- for a different site (that may or may not include all or a portion of the original site); or
- before important events occurred at the site or adjacent to it; e.g., man-made events like construction or environmental remediation, or natural events like floods, droughts, earthquakes, or groundwater fluctuations.

Note, too, the reliability of a geotechnical-engineering report can be affected by the passage of time, because of factors like changed subsurface conditions; new or modified codes, standards, or regulations; or new techniques or tools. *If you are the least bit uncertain about the continued reliability of this report, contact your geotechnical engineer before applying the recommendations in it. A minor amount of additional testing or analysis after the passage of time – if any is required at all – could prevent major problems.*

Read this Report in Full

Costly problems have occurred because those relying on a geotechnical-engineering report did not read the report in its entirety. Do not rely on an executive summary. Do not read selective elements only. *Read and refer to the report in full.*

You Need to Inform Your Geotechnical Engineer About Change

Your geotechnical engineer considered unique, project-specific factors when developing the scope of study behind this report and developing the confirmation-dependent recommendations the report conveys. Typical changes that could erode the reliability of this report include those that affect:

- the site's size or shape;
- the elevation, configuration, location, orientation, function or weight of the proposed structure and the desired performance criteria;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project or site changes – even minor ones – and request an assessment of their impact. *The geotechnical engineer who prepared this report cannot accept*

responsibility or liability for problems that arise because the geotechnical engineer was not informed about developments the engineer otherwise would have considered.

Most of the “Findings” Related in This Report Are Professional Opinions

Before construction begins, geotechnical engineers explore a site’s subsurface using various sampling and testing procedures. *Geotechnical engineers can observe actual subsurface conditions only at those specific locations where sampling and testing is performed.* The data derived from that sampling and testing were reviewed by your geotechnical engineer, who then applied professional judgement to form opinions about subsurface conditions throughout the site. Actual site-wide subsurface conditions may differ – maybe significantly – from those indicated in this report. Confront that risk by retaining your geotechnical engineer to serve on the design team through project completion to obtain informed guidance quickly, whenever needed.

This Report’s Recommendations Are Confirmation-Dependent

The recommendations included in this report – including any options or alternatives – are confirmation-dependent. In other words, they are not final, because the geotechnical engineer who developed them relied heavily on judgement and opinion to do so. Your geotechnical engineer can finalize the recommendations *only after observing actual subsurface conditions* exposed during construction. If through observation your geotechnical engineer confirms that the conditions assumed to exist actually do exist, the recommendations can be relied upon, assuming no other changes have occurred. *The geotechnical engineer who prepared this report cannot assume responsibility or liability for confirmation-dependent recommendations if you fail to retain that engineer to perform construction observation.*

This Report Could Be Misinterpreted

Other design professionals’ misinterpretation of geotechnical-engineering reports has resulted in costly problems. Confront that risk by having your geotechnical engineer serve as a continuing member of the design team, to:

- confer with other design-team members;
- help develop specifications;
- review pertinent elements of other design professionals’ plans and specifications; and
- be available whenever geotechnical-engineering guidance is needed.

You should also confront the risk of constructors misinterpreting this report. Do so by retaining your geotechnical engineer to participate in prebid and preconstruction conferences and to perform construction-phase observations.

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can shift unanticipated-subsurface-conditions liability to constructors by limiting the information they provide for bid preparation. To help prevent the costly, contentious problems this practice has caused, include the complete geotechnical-engineering report, along with any attachments or appendices, with your contract documents, *but be certain to note*

conspicuously that you’ve included the material for information purposes only. To avoid misunderstanding, you may also want to note that “informational purposes” means constructors have no right to rely on the interpretations, opinions, conclusions, or recommendations in the report. Be certain that constructors know they may learn about specific project requirements, including options selected from the report, *only* from the design drawings and specifications. Remind constructors that they may perform their own studies if they want to, and *be sure to allow enough time* to permit them to do so. Only then might you be in a position to give constructors the information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions. Conducting prebid and preconstruction conferences can also be valuable in this respect.

Read Responsibility Provisions Closely

Some client representatives, design professionals, and constructors do not realize that geotechnical engineering is far less exact than other engineering disciplines. This happens in part because soil and rock on project sites are typically heterogeneous and not manufactured materials with well-defined engineering properties like steel and concrete. That lack of understanding has nurtured unrealistic expectations that have resulted in disappointments, delays, cost overruns, claims, and disputes. To confront that risk, geotechnical engineers commonly include explanatory provisions in their reports. Sometimes labeled “limitations,” many of these provisions indicate where geotechnical engineers’ responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely.* Ask questions. Your geotechnical engineer should respond fully and frankly.

Geoenvironmental Concerns Are Not Covered

The personnel, equipment, and techniques used to perform an environmental study – e.g., a “phase-one” or “phase-two” environmental site assessment – differ significantly from those used to perform a geotechnical-engineering study. For that reason, a geotechnical-engineering report does not usually provide environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated subsurface environmental problems have led to project failures.* If you have not obtained your own environmental information about the project site, ask your geotechnical consultant for a recommendation on how to find environmental risk-management guidance.

Obtain Professional Assistance to Deal with Moisture Infiltration and Mold

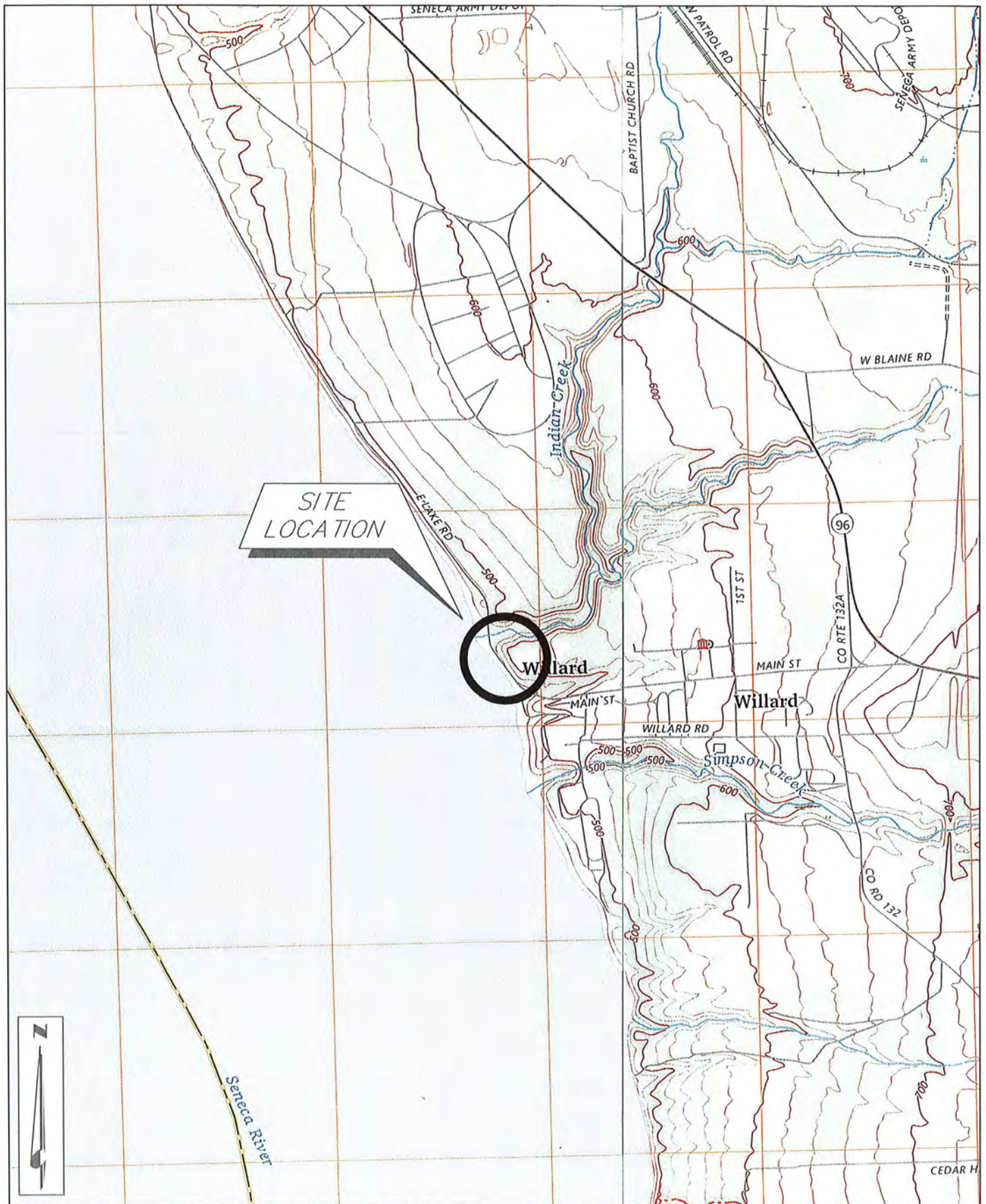
While your geotechnical engineer may have addressed groundwater, water infiltration, or similar issues in this report, the engineer’s services were not designed, conducted, or intended to prevent migration of moisture – including water vapor – from the soil through building slabs and walls and into the building interior, where it can cause mold growth and material-performance deficiencies. Accordingly, *proper implementation of the geotechnical engineer’s recommendations will not of itself be sufficient to prevent moisture infiltration.* Confront the risk of moisture infiltration by including building-envelope or mold specialists on the design team. *Geotechnical engineers are not building-envelope or mold specialists.*



GEOPROFESSIONAL
BUSINESS
ASSOCIATION

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**Foundation
Design, P.C.**

46A Sager Dr, Rochester, NY 14607

Dick Van Dyke Treatment Center - Kitchen Expansion

1330 County Route 132, Ovid, New York

General Location Plan

Adapted from: USGS Topographic Mapping 2023 Dresden & Ovid Quadrangles

CHECKED BY: RJR

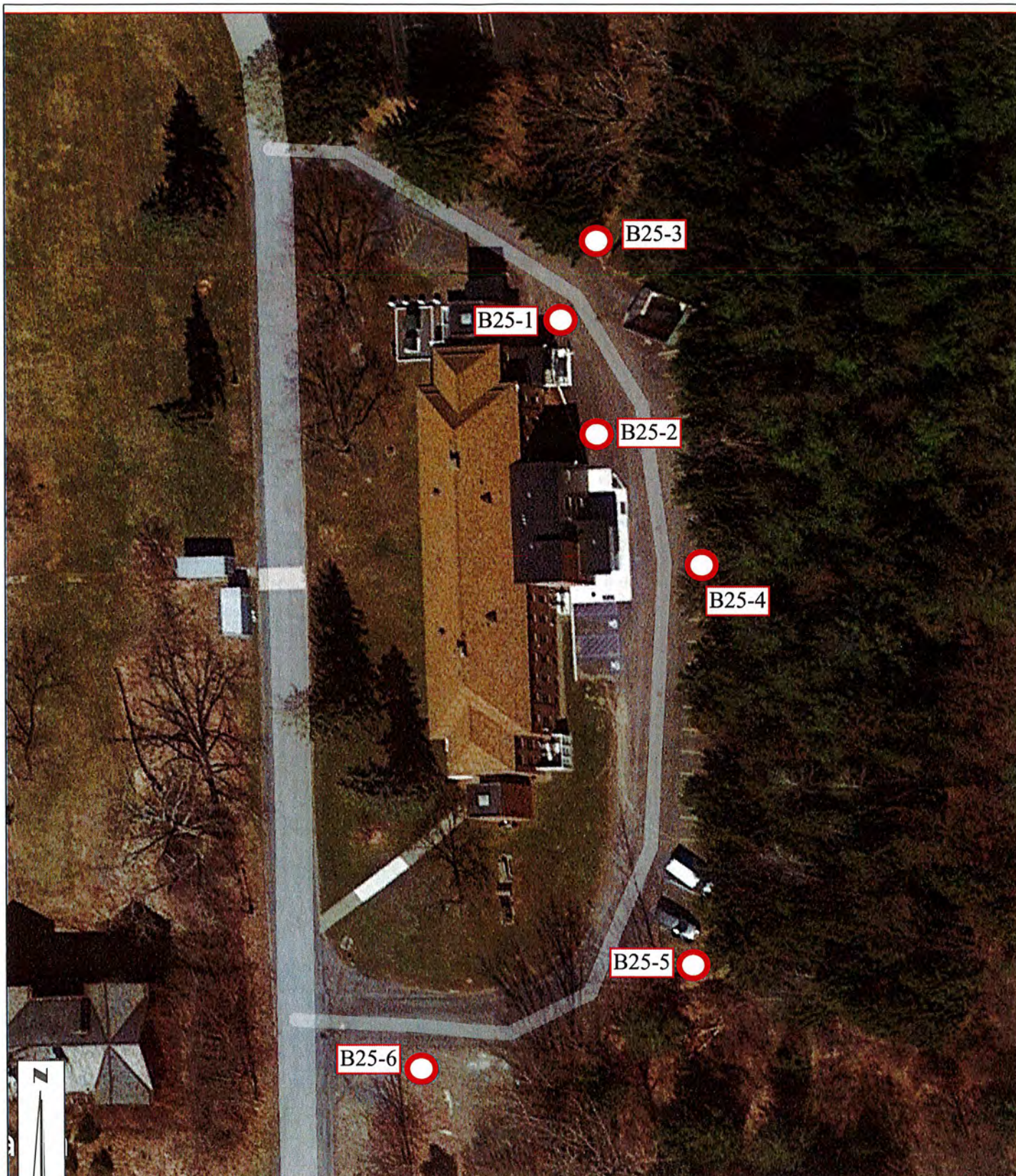
DRAWN BY: LMB

JOB NO.

25.05826

DATE: 2/28/25

SCALE: 1"=2000'



**Foundation
Design, P.C.**

46A Sager Dr, Rochester, NY 14607

Dick Van Dyke Treatment Center - Kitchen Expansion

1330 County Road #132, Ovid, New York

Boring Location Plan

Adapted from: Foundation Design, P.C., Google Earth Image

CHECKED BY: RJR

DRAWN BY: LSK

JOB NO.
25.05826

DATE: 3/11/25

SCALE: NTS

SOIL DESCRIPTIONS

COHESIVE SOIL

Very fine grained soils. Plastic soils that can be rolled into a thin thread if moist. Clays and silty clays show cohesion.

NON-COHESIVE SOIL

Soils composed of silt, sand and gravel, showing no cohesion or very slight cohesion

<u>DESCRIPTION</u>	<u>SPT –BLOWS/FOOT</u>	<u>DESCRIPTION</u>	<u>SPT –BLOWS/FOOT</u>
Very Soft	0-2	Loose	0-10
Soft	3-5	Firm	11-25
Medium	6-15	Compact	26-40
Stiff	16-25	Dense	41-50
Hard	26 or more	Very Dense	51 or more

<u>SOIL COMPOSITION</u>	<u>DESCRIPTION</u>	<u>ESTIMATED PERCENTAGE</u>
	and	50
	some	30-49
	little	11-29
	trace	0-10

<u>MOISTURE CONDITIONS</u>	Dry, Damp, Moist, Wet, Saturated Groundwater measured in the boring or test pit may not have reached equilibrium
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<u>SOIL STRATA:</u>	<u>TERM</u>	<u>DESCRIPTION</u>
	layer	Soil deposit more than 6" thick
	seam	Soil deposit less than 6" thick
	parting	Soil deposit less than 1/8" thick
	varved	Horizontal uniform layers or seams of soil

GRAIN SIZE

<u>MATERIAL</u>	<u>SIEVE SIZE</u>
Boulder	Larger than 12 inches
Cobble	3 inches to 12 inches
Gravel - coarse	1 inch to 3 inches
- medium	3/8 inch to 1 inch
- fine	No. 4 to 3/8 inch
Sand - coarse	No. 10 to No. 4
- medium	No. 40 to No. 10
- fine	No. 200 to No. 40
Silt and Clay	Less than No. 200

Standard Penetration Test: The number of blows required to drive a split spoon sampler into the soil with a 140 pound hammer dropped 30 inches. The number of blows required for each 6-inches of penetration is recorded. The total number of blows required for the second and third 6-inches of penetration is termed the penetration resistance, or the "N" value.

Split Spoon Sampler: Typically a 2-foot long, 2-inch diameter hollow steel tube that breaks apart or splits in two down the tube length.

Refusal: Depth in the boring where more than 100 blows per 5-inches are needed to advance the sample spoon.

Core Recovery (%): The total length of rock core recovered divided by the total core run.

RQD (%): Rock Quality Designation – the total length of all the pieces of the rock core longer than 4-inches divided by the total length of the rock core run.

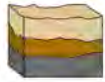


Boring Log

Project No.	25.05826	Page	1	of	1	Soil Boring No.	B25-1
Project Name	Dick Van Dyke Treatment Center – Kitchen Expansion, 1330 County Route 132, Ovid, NY						
Client	Colliers Engineering & Design, 280 East Broad Street, Rochester, NY						
Elevation		Weather	Sunny, 40s		Engineer	L. Kosciol	
Date Started	3/10/2025	Completed	3/10/2025		Driller	M. Cheney	
Drilling Company:	Northeast Specialized Drilling				Drilling Equipment:	CME 55 Truck Rig	

Ft.	Blows Per Six Inches				N Value	Sample No.	Depth	Rec	Visual Soil and Rock Classifications
	0"/6"	6"/12"	12"/18"	18"/24"					Remarks
	-	10							ASPHALT 0'5"
			11	6	21	S-1	6'-2'	12"	SUBBASE: Firm gray/black dry CRUSHED STONE, little sand 2'0"
	5	6							Firm gray/brown damp SILT, little sand, trace clay (Possible Fill) 4'0"
			9	14	15	S-2	2'-4'	16"	Compact brown damp SILT, little sand, trace gravel, trace clay
5	13	18							
			19	22	37	S-3	4'-6'	15"	
	12	13							S-4: Trace cobble fragments
			19	19	32	S-4	6'-8'	21"	S-5: Dense, trace to little gravel, trace cobble fragments, no clay
	14	19							
10			26	31	45	S-5	8'-10'	20"	
									13'0"
	15	19							Dense gray damp SILT, little gravel, little clay, trace cobble fragments, trace sand
15			31	31	50	S-6	13'-15'	21"	
	15	19							18'0"
20			50	50	69	S-7	18'-20'	17"	Hard gray damp CLAY, little silt, little cobble fragments, trace gravel
									20'6"
									Boring Terminated at 20'7" Auger Refusal
25									
30									Notes: 1. Water at 12'10" upon completion. 2. Advanced hole using hollow stem augers. 3. Bore hole backfilled using auger spoils, surface patched with concrete.

N=No. of blows to Drive 2" Spoon 12" with 140 lb. Wt. 30" Ea. Blow Hammer: Safety Size Rod: 2"



Boring Log

Project No.	25.05826	Page	1	of	1	Soil Boring No.	B25-2
Project Name	Dick Van Dyke Treatment Center – Kitchen Expansion, 1330 County Route 132, Ovid, NY						
Client	Colliers Engineering & Design, 280 East Broad Street, Rochester, NY						
Elevation		Weather	Sunny, 40s		Engineer	L. Kosciol	
Date Started	3/10/2025	Completed	3/10/2025		Driller	M. Cheney	
Drilling Company:	Northeast Specialized Drilling				Drilling Equipment:	CME 55 Truck Rig	

Ft.	Blows Per Six Inches				N Value	Sample No.	Depth	Rec	Visual Soil and Rock Classifications
	0"/6"	6"/12"	12"/18"	18"/24"					Remarks
	-	13							ASPHALT 0'5"
			23	15	36	S-1	6'-2'	10"	SUBBASE: Compact gray/brown dry CRUSHED STONE, little sand 2'0"
	13	11							Firm brown/gray damp SILT, little gravel, trace cobble fragments
			13	16	24	S-2	2'-4'	16"	
5	13	16							S-3: Compact, trace sand
			23	31	39	S-3	4'-6'	18"	
	19	29							S-4: Very dense, little sand, no cobble fragments
			42	43	71	S-4	6'-8'	21"	
	22	23							S-5: Very dense, little sand, trace clay, no cobble fragments
10			50/5"	-	73/11"	S-5	8'-9'5"	8"	
	33	41							S-6: Very dense, little cobble fragments, trace clay, trace sand 12'0"
			33	33	74	S-6	10'-12'	20"	Boring Terminated at 12'0"
15									
20									
25									
30									

Notes:

1. Dry upon completion.
2. Advanced hole using hollow stem augers.
3. Bore hole backfilled using auger spoils, surface patched with concrete.

N=No. of blows to Drive 2" Spoon 12" with 140 lb. Wt. 30" Ea. Blow Hammer: Safety Size Rod: 2"



Boring Log

Project No.	25.05826	Page	1	of	1	Soil Boring No.	B25-3
Project Name	Dick Van Dyke Treatment Center – Kitchen Expansion, 1330 County Route 132, Ovid, NY						
Client	Colliers Engineering & Design, 280 East Broad Street, Rochester, NY						
Elevation		Weather	Sunny, 40s		Engineer	L. Kosciol	
Date Started	3/10/2025	Completed	3/10/2025		Driller	M. Cheney	
Drilling Company:	Northeast Specialized Drilling				Drilling Equipment:	CME 55 Truck Rig	

Ft.	Blows Per Six Inches				N Value	Sample No.	Depth	Rec	Visual Soil and Rock Classifications
	0"/6"	6"/12"	12"/18"	18"/24"					Remarks
	5	4							Loose brown/black SILT, little sand, little asphalt millings 0'6" Loose brown damp SILT, little gravel, little sand S-2: Firm, moist, trace shale fragments S-3: Compact 6'0" Boring Terminated at 6'0"
			4	5	8	S-1	0'-2'	13"	
	3	7							
			12	14	19	S-2	2'-4'	18"	
5	12	16							
			23	24	39	S-3	4'-6'	20"	
10									
15									
									Notes: 1. Dry upon completion. 2. Advanced hole using hollow stem augers. 3. Bore hole backfilled using auger spoils.
20									
25									
30									

N=No. of blows to Drive 2" Spoon 12" with 140 lb. Wt. 30" Ea. Blow Hammer: Safety Size Rod: 2"



Boring Log

Project No.	25.05826	Page	1	of	1	Soil Boring No.	B25-4
Project Name	Dick Van Dyke Treatment Center – Kitchen Expansion, 1330 County Route 132, Ovid, NY						
Client	Colliers Engineering & Design, 280 East Broad Street, Rochester, NY						
Elevation		Weather	Sunny, 50s		Engineer	L. Kosciol	
Date Started	3/10/2025	Completed	3/10/2025		Driller	M. Cheney	
Drilling Company:	Northeast Specialized Drilling				Drilling Equipment:	CME 55 Truck Rig	

Ft.	Blows Per Six Inches				N Value	Sample No.	Depth	Rec	Visual Soil and Rock Classifications
	0"/6"	6"/12"	12"/18"	18"/24"					Remarks
	-	13							ASPHALT 0'5"
			14	12	27	S-1	6"-2'	13"	SUBBASE: Compact brown/gray/black damp CRUSHED STONE, some sand, little silt 2'0"
	10	13							Compact brown/gray moist SILT, little gravel, little sand
			16	13	29	S-2	2'-4'	19"	
5	11	13							S-3: Little shale fragments, trace cobble fragments, trace clay 6'0"
			26	35	39	S-3	4'-6'	20"	Boring Terminated at 6'0"
10									
15									
20									
25									
30									

Notes:

1. Dry upon completion.
2. Advanced hole using hollow stem augers.
3. Bore hole backfilled using auger spoils, surface patched with concrete.

N=No. of blows to Drive 2" Spoon 12" with 140 lb. Wt. 30" Ea. Blow Hammer: Safety Size Rod: 2"



Boring Log

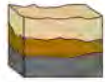
Project No.	25.05826	Page	1	of	1	Soil Boring No.	B25-5
Project Name	Dick Van Dyke Treatment Center – Kitchen Expansion, 1330 County Route 132, Ovid, NY						
Client	Colliers Engineering & Design, 280 East Broad Street, Rochester, NY						
Elevation		Weather	Sunny, 50s		Engineer	L. Kosciol	
Date Started	3/10/2025	Completed	3/10/2025		Driller	M. Cheney	
Drilling Company:	Northeast Specialized Drilling				Drilling Equipment:	CME 55 Truck Rig	

Ft.	Blows Per Six Inches				N Value	Sample No.	Depth	Rec	Visual Soil and Rock Classifications
	0"/6"	6"/12"	12"/18"	18"/24"					Remarks
	2	3							TOPSOIL 1'0"
			4	5	7	S-1	0'-2'	20"	Loose brown moist SAND, some silt, trace gravel 2'0"
	10	13							
			15	16	28	S-2	2'-4'	23"	Compact brown damp SILT, little gravel, little sand, trace clay
5	10	9							S-3: Firm, 2" sand seam from 5'6"-5'8"
			8	9	17	S-3	4'-6'	20"	6'0"
									Boring Terminated at 6'0"
10									
15									
20									
25									
30									

Notes:

1. Dry upon completion.
2. Advanced hole using hollow stem augers.
3. Bore hole backfilled using auger spoils.

N=No. of blows to Drive 2" Spoon 12" with 140 lb. Wt. 30" Ea. Blow Hammer: Safety Size Rod: 2"



Boring Log

Project No.	25.05826	Page	1	of	1	Soil Boring No.	B25-6
Project Name	Dick Van Dyke Treatment Center – Kitchen Expansion, 1330 County Route 132, Ovid, NY						
Client	Colliers Engineering & Design, 280 East Broad Street, Rochester, NY						
Elevation		Weather	Sunny, 50s		Engineer	L. Kosciol	
Date Started	3/10/2025	Completed	3/10/2025		Driller	M. Cheney	
Drilling Company:	Northeast Specialized Drilling				Drilling Equipment:	CME 55 Truck Rig	

Ft.	Blows Per Six Inches				N Value	Sample No.	Depth	Rec	Visual Soil and Rock Classifications
	0"/6"	6"/12"	12"/18"	18"/24"					Remarks
	5	4							CRUSHED STONE 0'3"
			4	4	8	S-1	0'-2'	10"	Loose brown/gray moist SILT, little sand, trace gravel
	11	12							
			15	15	27	S-2	2'-4'	21"	S-2: Compact, little gravel, trace decomposed shale
5	11	12							S-3: Compact, trace decomposed shale, trace clay
			14	16	26	S-3	4'-6'	22"	6'0"
									Boring Terminated at 6'0"
10									
15									
20									
25									
30									

Notes:

1. Dry upon completion.
2. Advanced hole using hollow stem augers.
3. Bore hole backfilled using auger spoils.

N=No. of blows to Drive 2" Spoon 12" with 140 lb. Wt. 30" Ea. Blow Hammer: Safety Size Rod: 2"



**Foundation
Design, P.C.**

Laboratory Test Results

**Dick Van Dyke Treatment Center – Kitchen Expansion
1330 County Route 132, Ovid, NY
25.05826**

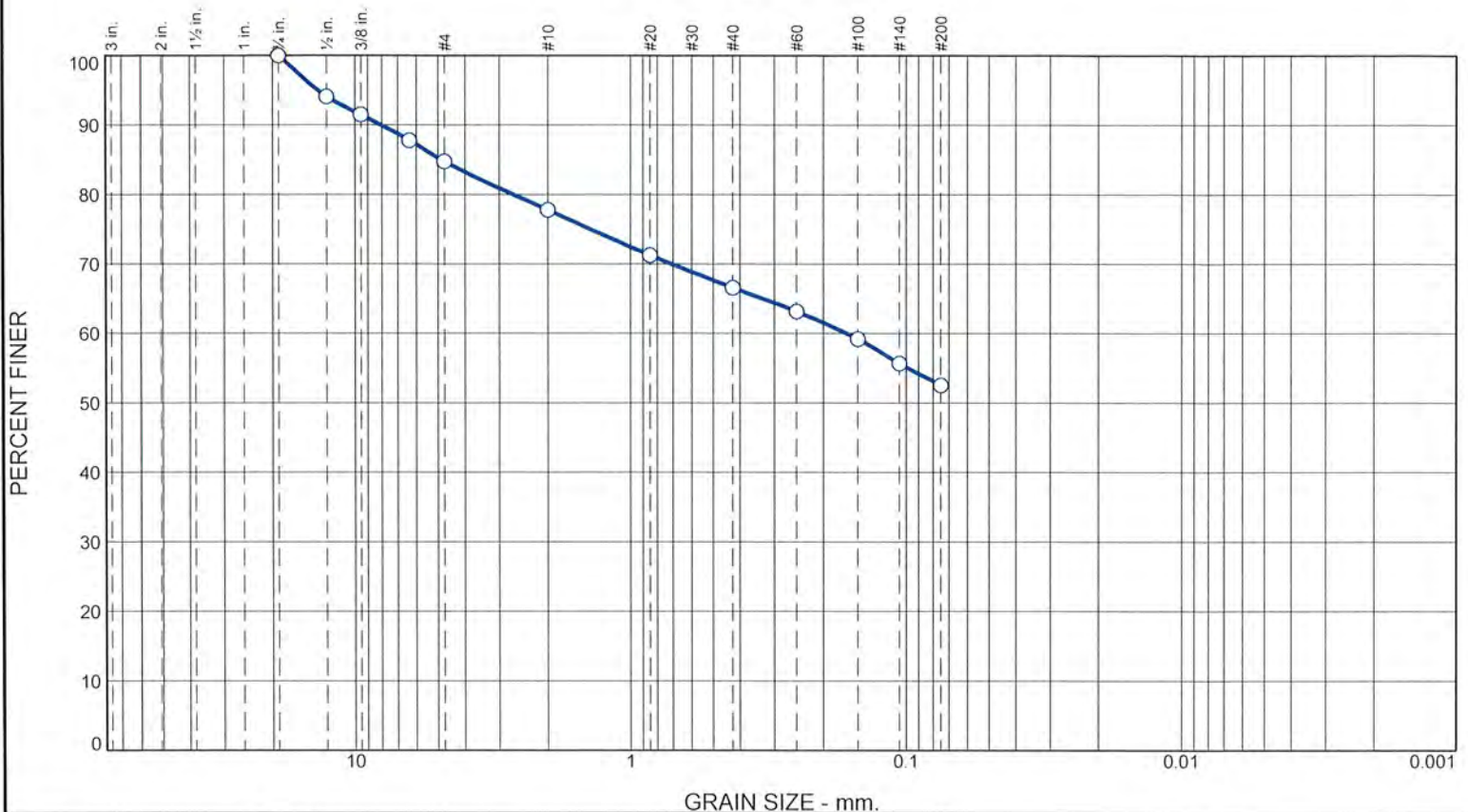
3.17.2025

Moisture Content Test Report (ASTM D-2216)

Moisture Content Test Results					
Boring Number	B25-1	B25-1	B25-2	B25-2	B25-4
Sample Number	S-2	S-3	S-4	S-5	Subbase
Depth	2'-4'	4'-6'	6'-8'	8'-10'	--
Moisture Content (%)	14.3	11.2	9.0	8.5	2.2

Particle Size Distribution Report

ASTM D422 & D1140



% +3"	% Gravel		% Sand			% Fines
	Coarse	Fine	Coarse	Medium	Fine	
0	0	15	7	11	14	53

Test Results (ASTM D422 & D1140)				
Sieve Size or Diam. (mm.)	Finer (%)	Spec.* (%)	Out of Spec. (%)	Pct. of Fines
3/4"	100			
1/2"	94			
3/8"	91			
1/4"	88			
#4	85			
#10	78			
#20	71			
#40	67			
#60	63			
#100	59			
#140	56			
#200	53			

* (no specification provided)

Material Description

ML: Gray brown sandy silt with gravel per ASTM D2488

Atterberg Limits

PL= LL= PI=

Classification

USCS= AASHTO=

Test Remarks

Test performed on 454.96 grams of oven dried sample

Source of Sample: B25-1 Depth: 2'-4'
Sample Number: S-2

Sample Date: 3-10-2025



**Foundation
Design, P.C.**

Client: Colliers Engineering & Design, 280 East Broad Street, Rochester, NY

Project: Dick Van Dyke Treatment Center - Kitchen Expansion
1330 County Route 132, Ovid, NY

Project No: 28.05826

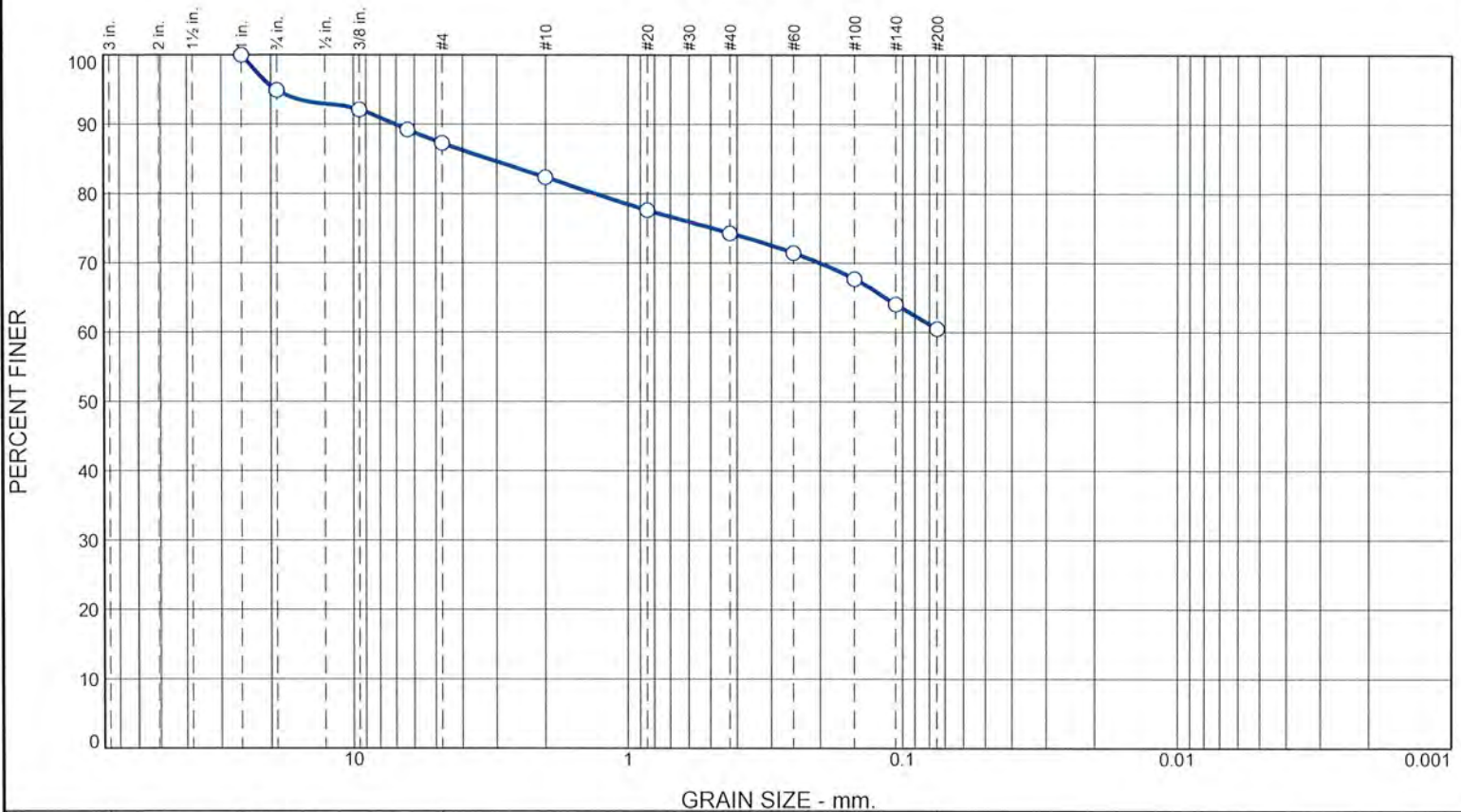
Figure

Tested By: RLS

Checked By: Jay A. Goggin

Particle Size Distribution Report

ASTM D422 & D1140



% +3"	% Gravel		% Sand			% Fines
	Coarse	Fine	Coarse	Medium	Fine	
0	5	8	5	8	14	60

Test Results (ASTM D422 & D1140)				
Sieve Size or Diam. (mm.)	Finer (%)	Spec.* (%)	Out of Spec. (%)	Pct. of Fines
1"	100			
3/4"	95			
3/8"	92			
1/4"	89			
#4	87			
#10	82			
#20	78			
#40	74			
#60	71			
#100	68			
#140	64			
#200	60			

* (no specification provided)

Material Description

ML: Dark brown sandy silt per ASTM D2488

PL= Atterberg Limits LL= PI=

USCS= Classification AASHTO=

Test Remarks

Test performed on 465.47 grams of oven dried sample

Source of Sample: B25-1 Depth: 4'-6'
Sample Number: S-3

Sample Date: 3-10-2025



**Foundation
Design, P.C.**

Client: Colliers Engineering & Design, 280 East Broad Street, Rochester, NY

Project: Dick Van Dyke Treatment Center - Kitchen Expansion
1330 County Route 132, Ovid, NY

Project No: 28.05826

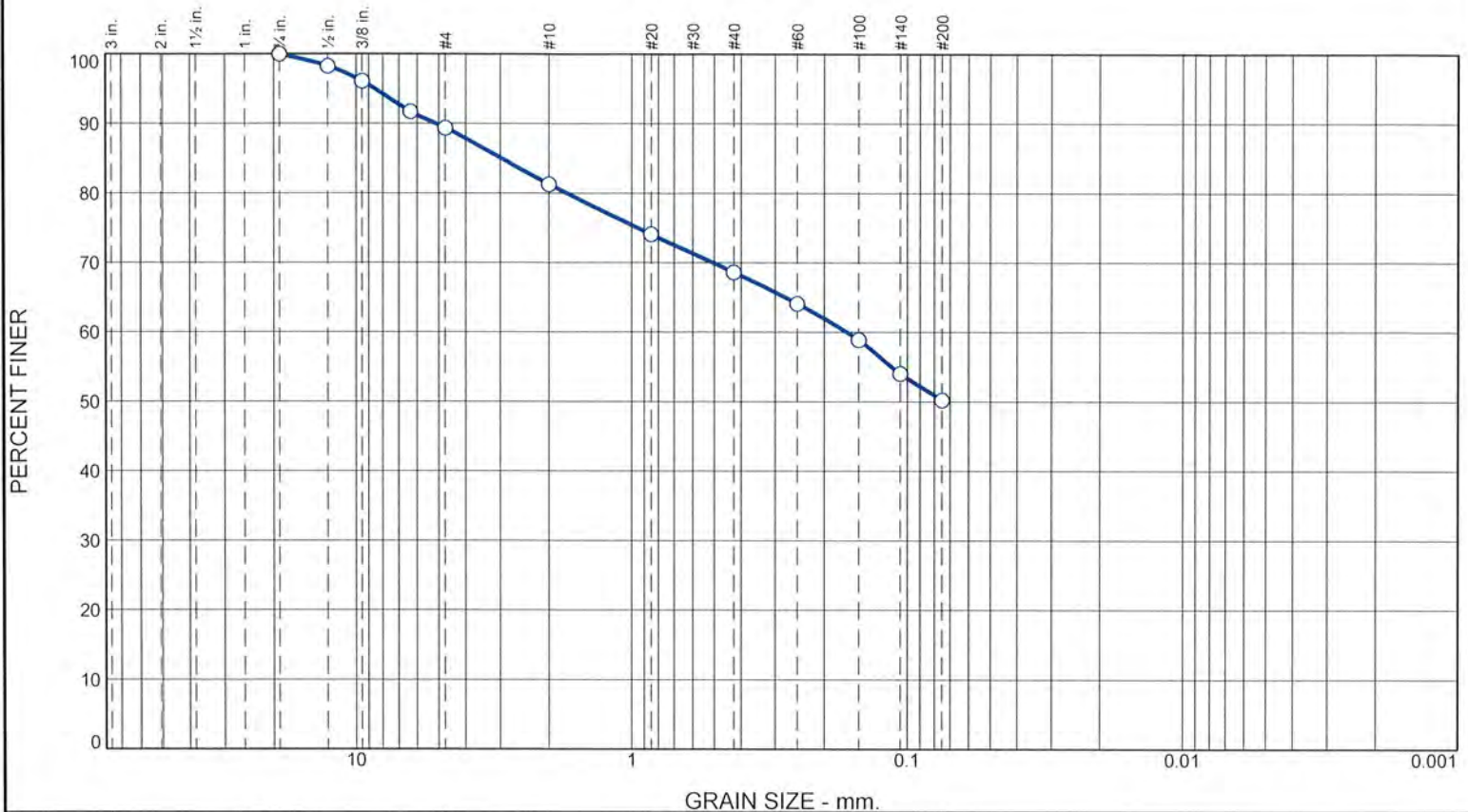
Figure

Tested By: RLS

Checked By: Jay A. Goggin

Particle Size Distribution Report

ASTM D422 & D1140



% +3"	% Gravel		% Sand			% Fines
	Coarse	Fine	Coarse	Medium	Fine	
0	0	11	8	12	19	50

Test Results (ASTM D422 & D1140)				
Sieve Size or Diam. (mm.)	Finer (%)	Spec.* (%)	Out of Spec. (%)	Pct. of Fines
3/4"	100			
1/2"	98			
3/8"	96			
1/4"	92			
#4	89			
#10	81			
#20	74			
#40	69			
#60	64			
#100	59			
#140	54			
#200	50			

* (no specification provided)

Material Description

ML: Brown gray sandy silt per ASTM D2488

PL= Atterberg Limits LL= PI=

USCS= Classification AASHTO=

Test Remarks

Test performed on 708.35 grams of oven dried sample

Source of Sample: B25-2 Depth: 6'-8'
Sample Number: S-4

Sample Date: 3-10-2025



**Foundation
Design, P.C.**

Client: Colliers Engineering & Design, 280 East Broad Street, Rochester, NY

Project: Dick Van Dyke Treatment Center - Kitchen Expansion
1330 County Route 132, Ovid, NY

Project No: 28.05826

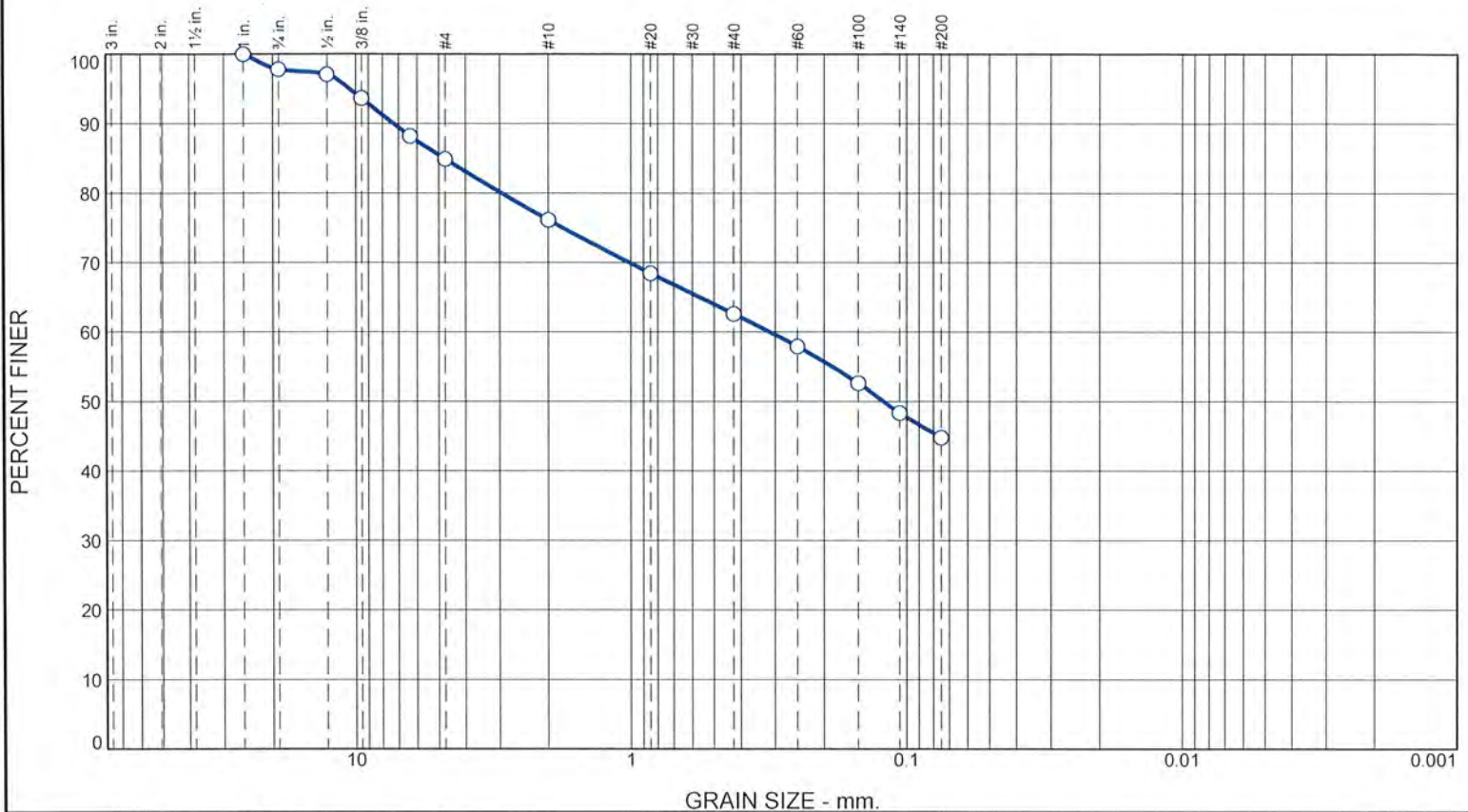
Figure

Tested By: RLS

Checked By: Jay A. Goggin

Particle Size Distribution Report

ASTM D422 & D1140



% +3"	% Gravel		% Sand			% Fines
	Coarse	Fine	Coarse	Medium	Fine	
0	2	13	9	13	18	45

Test Results (ASTM D422 & D1140)				
Sieve Size or Diam. (mm.)	Finer (%)	Spec.* (%)	Out of Spec. (%)	Pct. of Fines
1"	100			
3/4"	98			
1/2"	97			
3/8"	94			
1/4"	88			
#4	85			
#10	76			
#20	68			
#40	63			
#60	58			
#100	53			
#140	48			
#200	45			

* (no specification provided)

Material Description

SM: Brown gray silty sand with gravel per ASTM D2488

PL= Atterberg Limits LL= PI=

USCS= Classification AASHTO=

Test Remarks

Test performed on 469.36 grams of oven dried sample

Source of Sample: B25-2 Depth: 8'-10'
Sample Number: S-5

Sample Date: 3-17-2025



**Foundation
Design, P.C.**

Client: Colliers Engineering & Design, 280 East Broad Street, Rochester, NY

Project: Dick Van Dyke Treatment Center - Kitchen Expansion
1330 County Route 132, Ovid, NY

Project No: 28.05826

Figure

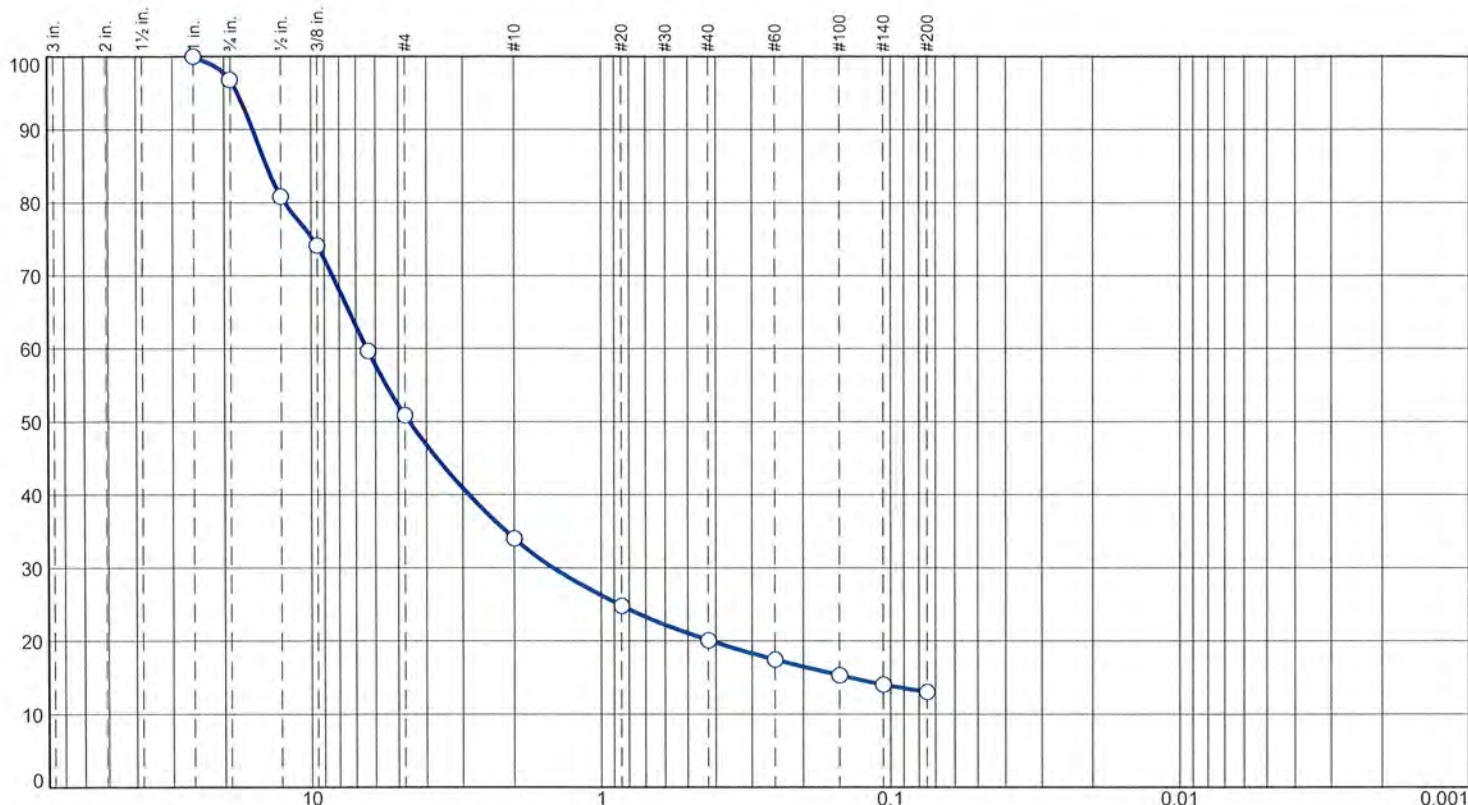
Tested By: RLS

Checked By: Jay A. Goggin

Particle Size Distribution Report

ASTM D422 & D1140

PERCENT FINER



GRAIN SIZE - mm.

% +3"	% Gravel		% Sand			% Fines
	Coarse	Fine	Coarse	Medium	Fine	
0	3	46	17	14	7	13

Test Results (ASTM D422 & D1140)

Sieve Size or Diam. (mm.)	Finer (%)	Spec.* (%)	Out of Spec. (%)	Pct. of Fines
1"	100			
3/4"	97			
1/2"	81			
3/8"	74			
1/4"	60			
#4	51			
#10	34			
#20	25			
#40	20			
#60	17			
#100	15			
#140	14			
#200	13			

* (no specification provided)

Material Description

GM: Brown gray black silty gravel with sand per ASTM 2488 (Crushed stone subbase)

PL= Atterberg Limits LL= PI=

USCS= Classification AASHTO=

Test Remarks

Test performed on 983.34 grams of ove dried sample

Source of Sample: B25-4
Sample Number: Subbase

Depth: Subbase

Sample Date: 3-10-2025



**Foundation
Design, P.C.**

Client: Colliers Engineering & Design, 280 East Broad Street, Rochester, NY

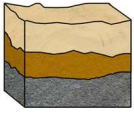
Project: Dick Van Dyke Treatment Center - Kitchen Expansion
1330 County Route 132, Ovid, NY

Project No: 28.05826

Figure

Tested By: RLS

Checked By: Jay A. Goggin



Foundation Design, P.C.

SOIL • BEDROCK • GROUNDWATER

May 16, 2025

Colliers Engineering & Design
280 East Broad Street, Suite 200
Rochester, New York 14604

Attention: Colleen R. Braun, RA, NCARB, AIA
Project Architect

Reference: Dick Van Dyke Treatment Center – Kitchen Expansion
1330 County Route 132, Ovid, New York
Test Pit Addendum No. 1, 25.05826.1

Dear Ms. Braun,

This Addendum outlines our test pit that was performed along the existing foundation to determine its depth and type. To assess the existing foundation, we retained Genesee Construction to perform a single test pit along the east wall where the new addition will be located on May 8, 2025. Genesee Construction provided a John Deere 35P excavator for the exploration work. Genesee Construction removed the plants and landscaping in the test pit area prior to the exploration. We terminated the test pit at a depth of 9.0 feet below grade, at the bottom of the foundation. We located the test pit measuring off of existing site features. Upon completion, the hole was backfilled and Genesee Construction placed the landscaping near to its original layout. No asphalt restoration was required. Our staff logged the subsurface profile and collected representative soil samples. The *Foundation Test Pit Location Plan* and the test pit log are enclosed.

Site Geology/Existing Foundation

The following interpretations of the soil, bedrock, and groundwater conditions are based on the single test pit, our site observations, past experience in the area, and consulting with the design team. See the attached test pit log for soil descriptions at the test location. Variations from the inferred profile are possible. Contact us immediately if variations are found during construction so we may evaluate the impact on our recommendations.

A typical soil profile consists of topsoil over various layers of backfill, then a silty clay. The topsoil was recorded as 4.0 inches at the test pit location. The underlying fill layers consisted of a 1.0 foot layer of washed stone over a waterproof membrane, then a 3.0 inch layer of silty sand. The lower fills consist of reworked silt and clay, likely native soil that was placed as backfill. The soils were noted as wet to saturated and were noted as firm/medium to stiff. We encountered the top of the foundation at a depth of 7.2 feet below grade. The footing is a typical spread footing that bumps out four inches from the wall. The thickness of the footing is 1.8 feet.

Colliers Engineering & Design
May 16, 2025
Page 2

On the north end of the existing building, a crawl space is present. The crawl space floor consists of the native silty clay with an estimated 6.0 inches of clean stone placed on top to provide a working surface. We estimate the crawl space floor is 5.0 feet higher than that of the adjacent full basement finished floor. We believe the native material in the crawl space was left undisturbed as the original plans did not call for a full basement. With the building being nearly 100 years old, this was likely a decision to minimize costs. We do not expect the crawl space to impact the proposed addition and the included full basement. If design plans change and the crawl space is to be removed, contact us so we can reassess our recommendations.

Groundwater was encountered during the test pit at a depth of 4.0 feet below grade. We expect the groundwater is perched above the impermeable silt and clays. Expect groundwater levels to fluctuate with seasonal changes.

Conclusions

We conclude that the existing building was constructed on a spread footing foundation system. We recommend designing the new foundations to bear at the same elevation as the existing foundations, at a depth of 9.0 feet.

We understand that the new addition will have a basement. We suspect that the underpinning of the existing foundation may be required, as the basement floor will likely extend below the top of the existing foundation. Where underpinning is required, we recommend limiting the open areas to a 4-foot width (parallel to the building) and staggering opened areas, leaving at least 8 feet between underpinned excavations. If underpinning is confirmed, we can consult with the structural engineer on the design/method of underpinning when needed.

Submission of this addendum completes our services on this project. Call if you have questions or if you require additional information as you progress your design. Forward a copy of the near final plans and specifications for our review and comment. It has been a pleasure to work with you on this project and we look forward to hearing from you again for the construction phase.

Sincerely,

FOUNDATION DESIGN, P.C.



Ryan J. Radford, P.E.
Vice President
Enc.



Important Information about This Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

The Geoprofessional Business Association (GBA) has prepared this advisory to help you – assumedly a client representative – interpret and apply this geotechnical-engineering report as effectively as possible. In that way, you can benefit from a lowered exposure to problems associated with subsurface conditions at project sites and development of them that, for decades, have been a principal cause of construction delays, cost overruns, claims, and disputes. If you have questions or want more information about any of the issues discussed herein, contact your GBA-member geotechnical engineer. Active engagement in GBA exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project.

Understand the Geotechnical-Engineering Services Provided for this Report

Geotechnical-engineering services typically include the planning, collection, interpretation, and analysis of exploratory data from widely spaced borings and/or test pits. Field data are combined with results from laboratory tests of soil and rock samples obtained from field exploration (if applicable), observations made during site reconnaissance, and historical information to form one or more models of the expected subsurface conditions beneath the site. Local geology and alterations of the site surface and subsurface by previous and proposed construction are also important considerations. Geotechnical engineers apply their engineering training, experience, and judgment to adapt the requirements of the prospective project to the subsurface model(s). Estimates are made of the subsurface conditions that will likely be exposed during construction as well as the expected performance of foundations and other structures being planned and/or affected by construction activities.

The culmination of these geotechnical-engineering services is typically a geotechnical-engineering report providing the data obtained, a discussion of the subsurface model(s), the engineering and geologic engineering assessments and analyses made, and the recommendations developed to satisfy the given requirements of the project. These reports may be titled investigations, explorations, studies, assessments, or evaluations. Regardless of the title used, the geotechnical-engineering report is an engineering interpretation of the subsurface conditions within the context of the project and does not represent a close examination, systematic inquiry, or thorough investigation of all site and subsurface conditions.

Geotechnical-Engineering Services are Performed for Specific Purposes, Persons, and Projects, and At Specific Times

Geotechnical engineers structure their services to meet the specific needs, goals, and risk management preferences of their clients. A geotechnical-engineering study conducted for a given civil engineer

will not likely meet the needs of a civil-works constructor or even a different civil engineer. Because each geotechnical-engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client.

Likewise, geotechnical-engineering services are performed for a specific project and purpose. For example, it is unlikely that a geotechnical-engineering study for a refrigerated warehouse will be the same as one prepared for a parking garage; and a few borings drilled during a preliminary study to evaluate site feasibility will not be adequate to develop geotechnical design recommendations for the project.

Do not rely on this report if your geotechnical engineer prepared it:

- for a different client;
- for a different project or purpose;
- for a different site (that may or may not include all or a portion of the original site); or
- before important events occurred at the site or adjacent to it; e.g., man-made events like construction or environmental remediation, or natural events like floods, droughts, earthquakes, or groundwater fluctuations.

Note, too, the reliability of a geotechnical-engineering report can be affected by the passage of time, because of factors like changed subsurface conditions; new or modified codes, standards, or regulations; or new techniques or tools. *If you are the least bit uncertain about the continued reliability of this report, contact your geotechnical engineer before applying the recommendations in it. A minor amount of additional testing or analysis after the passage of time – if any is required at all – could prevent major problems.*

Read this Report in Full

Costly problems have occurred because those relying on a geotechnical-engineering report did not read the report in its entirety. Do not rely on an executive summary. Do not read selective elements only. *Read and refer to the report in full.*

You Need to Inform Your Geotechnical Engineer About Change

Your geotechnical engineer considered unique, project-specific factors when developing the scope of study behind this report and developing the confirmation-dependent recommendations the report conveys. Typical changes that could erode the reliability of this report include those that affect:

- the site's size or shape;
- the elevation, configuration, location, orientation, function or weight of the proposed structure and the desired performance criteria;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project or site changes – even minor ones – and request an assessment of their impact. *The geotechnical engineer who prepared this report cannot accept*

responsibility or liability for problems that arise because the geotechnical engineer was not informed about developments the engineer otherwise would have considered.

Most of the “Findings” Related in This Report Are Professional Opinions

Before construction begins, geotechnical engineers explore a site’s subsurface using various sampling and testing procedures. *Geotechnical engineers can observe actual subsurface conditions only at those specific locations where sampling and testing is performed.* The data derived from that sampling and testing were reviewed by your geotechnical engineer, who then applied professional judgement to form opinions about subsurface conditions throughout the site. Actual site-wide subsurface conditions may differ – maybe significantly – from those indicated in this report. Confront that risk by retaining your geotechnical engineer to serve on the design team through project completion to obtain informed guidance quickly, whenever needed.

This Report’s Recommendations Are Confirmation-Dependent

The recommendations included in this report – including any options or alternatives – are confirmation-dependent. In other words, they are not final, because the geotechnical engineer who developed them relied heavily on judgement and opinion to do so. Your geotechnical engineer can finalize the recommendations *only after observing actual subsurface conditions* exposed during construction. If through observation your geotechnical engineer confirms that the conditions assumed to exist actually do exist, the recommendations can be relied upon, assuming no other changes have occurred. *The geotechnical engineer who prepared this report cannot assume responsibility or liability for confirmation-dependent recommendations if you fail to retain that engineer to perform construction observation.*

This Report Could Be Misinterpreted

Other design professionals’ misinterpretation of geotechnical-engineering reports has resulted in costly problems. Confront that risk by having your geotechnical engineer serve as a continuing member of the design team, to:

- confer with other design-team members;
- help develop specifications;
- review pertinent elements of other design professionals’ plans and specifications; and
- be available whenever geotechnical-engineering guidance is needed.

You should also confront the risk of constructors misinterpreting this report. Do so by retaining your geotechnical engineer to participate in prebid and preconstruction conferences and to perform construction-phase observations.

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can shift unanticipated-subsurface-conditions liability to constructors by limiting the information they provide for bid preparation. To help prevent the costly, contentious problems this practice has caused, include the complete geotechnical-engineering report, along with any attachments or appendices, with your contract documents, *but be certain to note*

conspicuously that you’ve included the material for information purposes only. To avoid misunderstanding, you may also want to note that “informational purposes” means constructors have no right to rely on the interpretations, opinions, conclusions, or recommendations in the report. Be certain that constructors know they may learn about specific project requirements, including options selected from the report, *only* from the design drawings and specifications. Remind constructors that they may perform their own studies if they want to, and *be sure to allow enough time* to permit them to do so. Only then might you be in a position to give constructors the information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions. Conducting prebid and preconstruction conferences can also be valuable in this respect.

Read Responsibility Provisions Closely

Some client representatives, design professionals, and constructors do not realize that geotechnical engineering is far less exact than other engineering disciplines. This happens in part because soil and rock on project sites are typically heterogeneous and not manufactured materials with well-defined engineering properties like steel and concrete. That lack of understanding has nurtured unrealistic expectations that have resulted in disappointments, delays, cost overruns, claims, and disputes. To confront that risk, geotechnical engineers commonly include explanatory provisions in their reports. Sometimes labeled “limitations,” many of these provisions indicate where geotechnical engineers’ responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely.* Ask questions. Your geotechnical engineer should respond fully and frankly.

Geoenvironmental Concerns Are Not Covered

The personnel, equipment, and techniques used to perform an environmental study – e.g., a “phase-one” or “phase-two” environmental site assessment – differ significantly from those used to perform a geotechnical-engineering study. For that reason, a geotechnical-engineering report does not usually provide environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated subsurface environmental problems have led to project failures.* If you have not obtained your own environmental information about the project site, ask your geotechnical consultant for a recommendation on how to find environmental risk-management guidance.

Obtain Professional Assistance to Deal with Moisture Infiltration and Mold

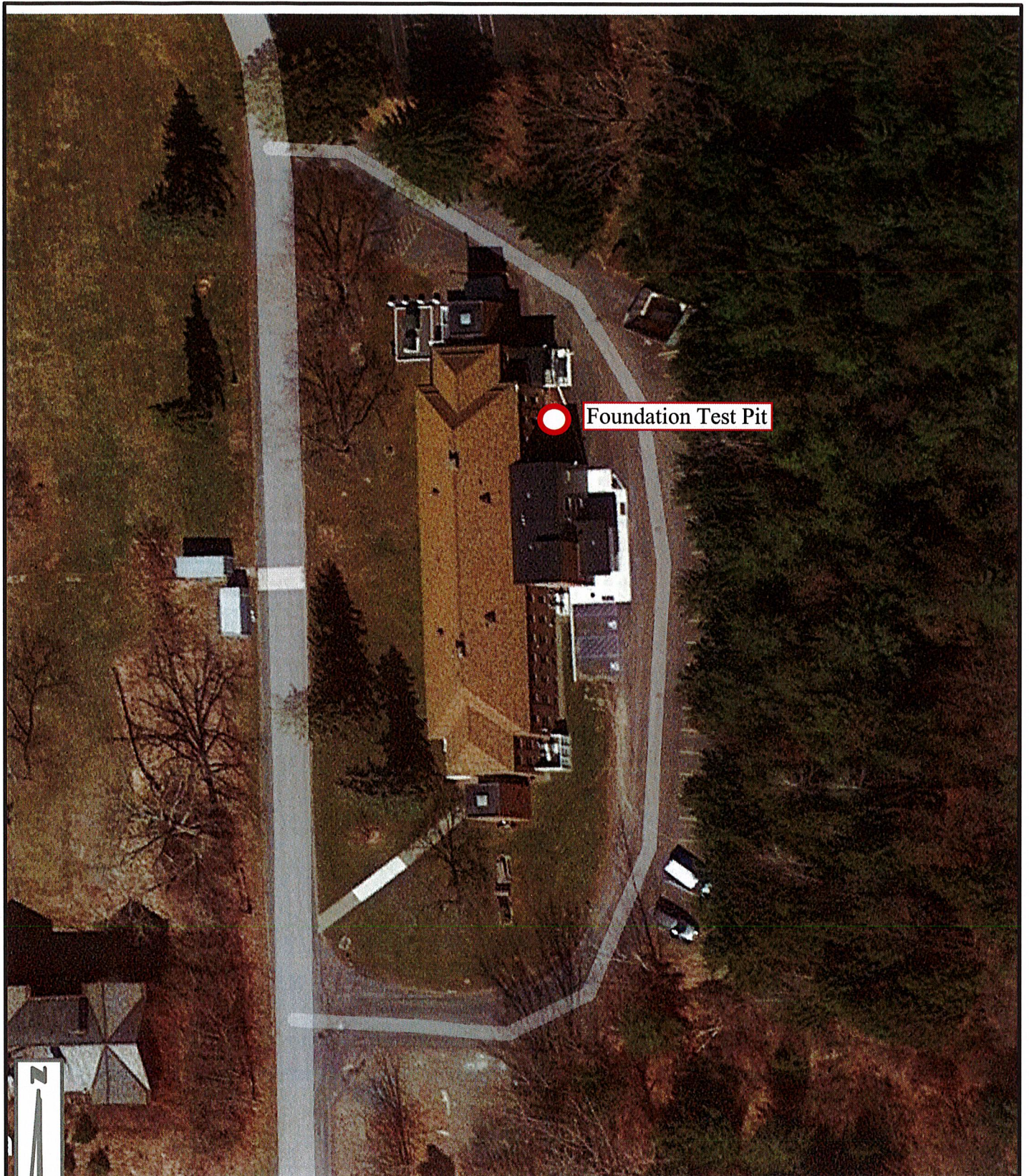
While your geotechnical engineer may have addressed groundwater, water infiltration, or similar issues in this report, the engineer’s services were not designed, conducted, or intended to prevent migration of moisture – including water vapor – from the soil through building slabs and walls and into the building interior, where it can cause mold growth and material-performance deficiencies. Accordingly, *proper implementation of the geotechnical engineer’s recommendations will not of itself be sufficient to prevent moisture infiltration.* Confront the risk of moisture infiltration by including building-envelope or mold specialists on the design team. *Geotechnical engineers are not building-envelope or mold specialists.*



**GEOPROFESSIONAL
BUSINESS
ASSOCIATION**

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**Foundation
Design, P.C.**

46A Sager Dr, Rochester, NY 14607

Dick Van Dyke Treatment Center - Kitchen Expansion

1330 County Road #132, Ovid, New York

Foundation Test Pit Location Plan

Adapted from: Foundation Design, P.C., Google Earth Image

CHECKED BY: RJR

DRAWN BY: LSK

JOB NO.
25.05826

DATE: 5/9/25

SCALE: NTS



SOIL DESCRIPTIONS

COHESIVE SOIL

Very fine grained soil. Plastic soil that can be rolled into a thin thread if moist. Clays and silty clays show cohesion.

DESCRIPTION

Very Soft	Extrude between fingers when squeezed
Soft	Molded by light finger pressure
Medium	Molded by strong finger pressure
Stiff	Indented by thumb with effort
Hard	Indented by thumb nail with difficulty

NON-COHESIVE SOIL

Soil composed of silt, sand and gravel, showing no cohesion or very slight cohesion.

DESCRIPTION

Loose	Minimal effort by excavator
Firm	Moderate effort by excavator
Compact	Significant effort by excavator
Dense	High effort by excavator
Very Dense	Extreme effort by excavator

SOIL COMPOSITION

DESCRIPTION

ESTIMATED PERCENTAGE

and	50
some	30-49
little	11-29
trace	0-10

MOISTURE CONDITIONS

dry, damp, moist, wet, saturated
Groundwater measured in the boring or test pit may not have reached equilibrium

SOIL STRATA

TERM

DESCRIPTION

layer	Soil deposit more than 6" thick
seam	Soil deposit less than 6" thick
parting	Soil deposit less than 1/8" thick
varved	Horizontal uniform layers or seams of soil

GRAIN SIZE

MATERIAL	SIEVE SIZE
Boulder	Larger than 12"
Cobble	3" to 12"
Gravel - coarse	1" to 3"
- medium	3/8" to 1"
- fine	No. 4 to 3/8"
Sand - coarse	No. 10 to No. 4
- medium	No. 40 to No. 10
- fine	No. 200 to No. 40
Silt and Clay	Less than No. 200



Foundation Design, P.C.

SOIL • BEDROCK • GROUNDWATER

Test Pit Log

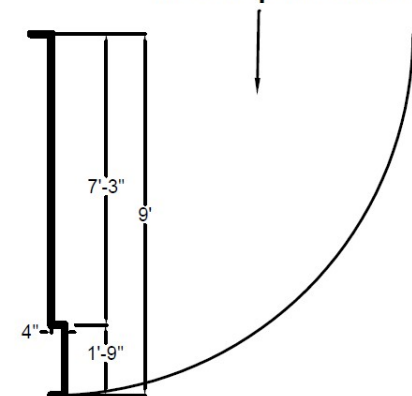
Project No.	5826.0	Page	1	of	1	Test Pit No.	Existing Foundation
Project Name	Dick Van Dyke Treatment Center – Kitchen Expansion, 1330 County Route 132, Ovid, NY						
Client	Colliers Engineering & Design, 280 East Broad Street, Rochester, NY						
Elevation		Weather	Cloudy 45°		Technician	L. Kosciol	
Date Started	5/8/25	Completed	5/8/25		Operator	B. West	
Backhoe Subcontractor	Genessee Construction		Equipment	John Deere 35P			

Depth Below Surface	Sample Number	Depth of Sample	Soil and Rock Classifications	Remarks
2			TOPSOIL	4"
			Loose gray moist CLEAN STONE	1'4"
			Waterproof membrane	1'5"
			Loose brown moist SAND, little silt	1'8"
			FILL: Stiff brown moist CLAY, little silt, trace sand, trace gravel (reworked native)	
4			(Little sand)	
				4'0"
6			FILL: Firm brown saturated SILT, little gravel, little sand	
				4'8"
			FILL: Medium gray wet CLAY, trace cobbles	
8				6'0"
			Medium gray wet CLAY, little silt	
				7'3"
10			Foundation Footer	
				9'0"
12			Test Pit terminated at 9'0"	

Site Pictures

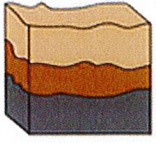


Test pit excavation



Notes:

1. Sides vertical on completion.
2. Groundwater at 4'.
3. Staked location provided by Foundation Design.



**Foundation
Design P.C.**

January 14, 2026

Colliers Engineering & Design
280 East Broad Street, Suite 200
Rochester, New York 14604

Attention: Colleen R. Braun, RA, NCARB, AIA
Project Architect

Reference: Dick Van Dyke Treatment Center – Kitchen Expansion
1330 County Route 132, Ovid, New York
Addendum No. 2 to Geotechnical Evaluation, 25.05826.0

Dear Mrs. Braun:

This letter is intended to serve as an addendum to our previously issued report, 25.05826. Questions arose regarding use of backfill for the various aspects of the project, as well as soil parameters to utilize for design.

Based on the previous data, we make the following recommendations:

For the question about types of backfill use, we would allow the native material (loamy till) to be used for the 3 types of backfill (Basement Foundation wall backfill, Slab on grade foundation wall backfill, and Typical structural fill), but it may be moisture sensitive depending on what time of year the work is performed. See our original report for recommendations on handling onsite soil. The importing of Select Granular fill can be utilized for any type of backfill. This material would typically classify as a SM or GM in USCS (Unified Soil Classification System), but I recommend that we review the material to make sure that it is suitable.

For the design of below grade walls, use the results of the 2017 report for the adjacent addition if the native till (ML) soil is reused. The values used in this report are an angle of 32° , weight of 125 pcf and fluid pressure of 60 pcf. If an imported material is used for backfill, the numbers would be dependent on what material would be used. If a sand (SM), the values would be approximately an angle of 32° , weight of 125 pcf, and fluid pressure of 60 pcf. If a gravel is used (GM), the values would be approximately an angle of 34° , weight of 145 pcf, and fluid pressure of 60 pcf. These values are given as an estimate only and would have to be confirmed upon our review of the material.



Colliers Engineering & Design
January 14, 2026
Page 2

It has been a pleasure to further assist you in this project. Call if we can be of further assistance or if you have questions regarding the results.

Sincerely,

FOUNDATION DESIGN, P.C.

A handwritten signature in blue ink, appearing to read "Ryan J. Radford", written over a horizontal line.

Ryan J. Radford, P.E.
Vice President

